DRAFT FEASIBILITY REPORT FEASIBILITY ANALYSIS OF WATER SUPPLY FOR SMALL PUBLIC WATER SYSTEMS

SOUTH MIDLAND COUNTY WATER SYSTEM, INC. PWS ID# 1650077, CCN# U0181

Prepared for:

THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY





Prepared by:

THE UNIVERSITY OF TEXAS BUREAU OF ECONOMIC GEOLOGY

AND

PARSONS

Preparation of this report was financed by the Texas Commission on Environmental Quality through the Drinking Water State Revolving Fund Small Systems Assistance Program

AUGUST 2005

DRAFT FEASIBILITY REPORT

FEASIBILITY ANALYSIS OF WATER SUPPLY FOR SMALL PUBLIC WATER SYSTEMS

SOUTH MIDLAND COUNTY WATER SYSTEM, INC. PWS ID# 1650077, CCN# U0181

Prepared for:

THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Prepared by:

THE UNIVERSITY OF TEXAS BUREAU OF ECONOMIC GEOLOGY

AND



Preparation of this report was financed by the Texas Commission on Environmental Quality through the Drinking Water State Revolving Fund Small Systems Assistance Program

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW UNDER THE AUTHORITY OF ERIC J. DAWSON, P.E. 79564 ON AUGUST 31, 2005. IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING, OR PERMIT PURPOSES.

AUGUST 2005

EXECUTIVE SUMMARY

INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), were contracted by the Texas Commission on Environmental Quality (TCEQ) to conduct a study to assist with identifying and analyzing alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project was to promote compliance using sound engineering and financial methods and data for PWSs that had recently recorded sample results exceeding maximum contaminant levels (MCL). The primary objectives of this project were to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS for future implementation.

This feasibility report provides an evaluation of water supply alternatives for the South Midland County PWS located in Midland County. Recent sample results from the South Midland County PWS exceeded the MCL for for arsenic of 10 micrograms per liter (μ g/L) that will go into effect on January 23, 2006 (USEPA 2005a; TCEQ 2004a). Therefore, it was likely that the South Midland County PWS would face potential compliance issues under the new arsenic standard. Recent sample results also exceeded the MCL for nitrate of 10 milligrams per liter (mg/L) and the MCL for total dissolved solids (TDS) of 1,000 mg/L (USEPA 2005a; TCEQ 2004a).

Basic system information for the South Midland County PWS is shown in Table ES.1.

Table ES.1 South Midland County PWS Basic System Information

Population served	256
Connections	75
Average daily flow rate	0.024 million gallons per day (mgd)
Water system peak capacity	0.266 mgd
Typical nitrate range	10 to 11 mg/L
Typical arsenic range	12 to 17 μg/L

Typical TDS range	1,500 to 1,900 mg/L
-------------------	---------------------

STUDY METHODS

The methods used for this study were based on a pilot study performed in 2004 and 2005 by TCEQ, BEG, and Parsons. Methods for identifying and analyzing compliance options were developed in the pilot study (a decision tree approach).

The process for developing the feasibility study used the following general steps:

- 1. Gather data from the TCEQ and Texas Water Development Board databases, from TCEQ files, and from information maintained by the PWS:
- 2. Conduct financial, managerial, and technical (FMT) evaluations of the PWS;
- 3. Perform a geologic and hydrogeologic assessment of the study area;
- 4. Develop treatment and non-treatment compliance alternatives which, in general, consist of the following possible options:
 - a. Connecting to neighboring PWSs via new pipeline or by pumping water from a newly installed well or an available surface water supply within the jurisdiction of the neighboring PWS;
 - b. Installing new wells within the vicinity of the PWS into other aquifers with confirmed water quality standards meeting the MCLs:
 - c. Installing a new intake system within the vicinity of the PWS to obtain water from a surface water supply with confirmed water quality standards meeting the MCLs;
 - d. Treating the existing non-compliant water supply by various methods depending on the type of contaminant; and
 - e. Delivering potable water by way of a bottled water program or a treated water dispenser as an interim measure only.
- 5. Assess each of the potential alternatives with respect to economic and non-economic criteria; and
- 6. Prepare a feasibility report and present the results to the PWS.

This basic approach is summarized in Figure ES-1.

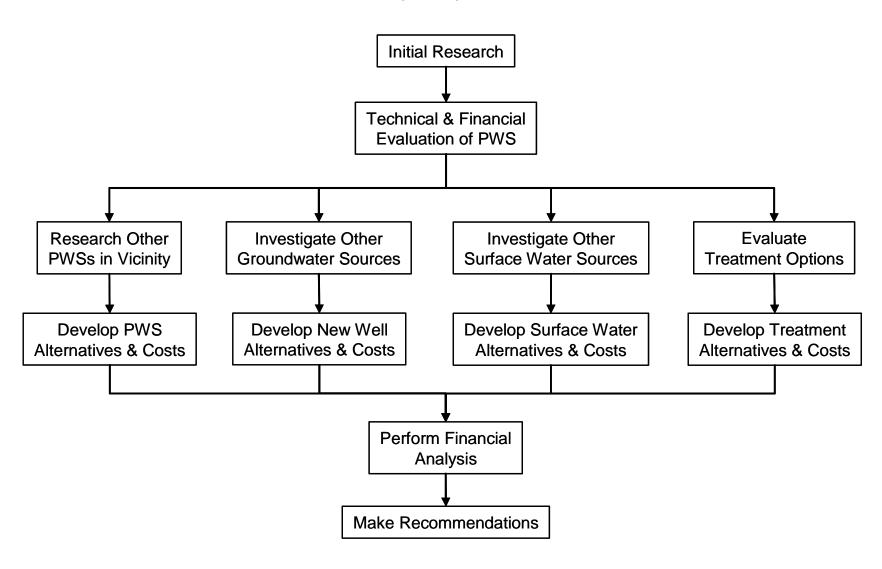
HYDROGEOLOGICAL ANALYSIS

The South Midland County PWS obtains groundwater from the Chicot subunit of the Gulf Coast Aquifer. Nitrate and arsenic are commonly found in area wells at concentrations greater than the MCLs. The arsenic may be naturally occurring, but the nitrate may be the result of agricultural or other human activity. Nitrate and arsenic concentrations can vary significantly over relatively short distances; as a result, there could be good quality groundwater nearby. However, the variability of nitrate and

arsenic concentrations makes it difficult to determine where wells can be located to produce



Figure ES-1 Summary of Project Methods



acceptable water. Additionally, systems with more than one well should characterize the water quality of each well. If one of the wells is found to produce compliant water, as much production as possible should be shifted to that well as a method of achieving compliance. It may also be possible to do down-hole testing on non-compliant wells to determine the source of the contaminants. If the contaminants derive primarily from a single part of the formation, that part could be excluded by modifying the existing well, or avoided altogether by completing a new well.

COMPLIANCE ALTERNATIVES

1 2

The South Midland County PWS is currently under receivership. The only staff for the system is a manager/operator who also owns and operates another small water system in the nearby vicinity. Overall, the system had an inadequate level of FMT capacity. The system had numerous areas that needed improvement to be able to address future compliance issues; however, the system does have positive aspects, including dedicated manager/operator and adequate disinfection throughout the system. Areas of concern for the system included lack of cost-tracking and reconciling operating budget, insufficient revenue collection, lack of emergency funds, insufficient staffing, lack of long-term capital improvements planning, lack of source water protection, and lack of consumer confidence report and public notice.

There are several PWSs within 20 miles of the South Midland County PWS. Many of these nearby systems also have problems with nitrate and arsenic, but there are several with good quality water. In general, feasibility alternatives were developed based on obtaining water from the nearest PWSs, either by directly purchasing water, or by expanding the existing well field. There is a minimum of surface water available in the area, and obtaining a new surface water source is considered through an alternative where treated surface water is obtained from the City of Odessa.

A number of centralized treatment alternatives for nitrate and arsenic removal have been developed and were considered for this report, for example, reverse osmosis and electrodialysis treatments. Point-of-use (POU) and point-of-entry (POE) treatment alternatives were also considered. Temporary solutions such as providing bottled water or providing a centralized dispenser for treated or trucked-in water, were also considered as alternatives.

Developing a new well close to the South Midland County PWS is likely to be the best solution if compliant groundwater can be found. Having a new well close to South Midland County is likely to be one of the lower cost alternatives since the PWS already possesses the technical and managerial expertise needed to implement this option. The cost of new well alternatives quickly increases with pipeline length, making proximity of the alternate source a key concern. A new compliant well or obtaining water from a neighboring compliant PWS has the advantage of providing compliant water to all taps in the system.

Central treatment can be cost-competitive with the alternative of new nearby wells, but would require significant institutional changes to manage and operate. Like obtaining an alternate compliant water source, central treatment would provide compliant water to all water taps.

POU treatment can be cost competitive, but does not supply compliant water to all taps. Additionally, significant efforts would be required for maintenance and monitoring of the POU treatment units.

Providing compliant water through a central dispenser is significantly less expensive than providing bottled water to 100 percent of the population, but a significant effort is required for clients to fill their containers at the central dispenser.

FINANCIAL ANALYSIS

Financial analysis of the South Midland County PWS indicated that current water rates are underfunding operations. Based on estimates provided by the system operator, the current average annual water use by South Midland County residential customers is estimated to be \$299, or approximately 0.7 percent of 2000 median household income (MHI) for Texas, which is \$39,927. Because of the lack of financial data exclusively for the water system, it is difficult to determine exact cash flow needs. Table ES.2 provides a summary of the financial impact of implementing selected compliance alternatives, including the rate increase necessary to meet current operating expenses. The alternatives were selected to highlight results for the best alternatives from each different type or category.

Some of the compliance alternatives offer potential for shared or regional solutions. A group of PWSs could work together to implement alternatives for developing a new groundwater source or expanding an existing source, obtaining compliant water from a large regional provider, or for central treatment. Sharing the cost for implementation of these alternatives could reduce the cost on a per user basis. Additionally, merging PWSs or management of several PWSs by a single entity offers the potential for reduction in administrative costs.

1 2

Table ES.2 Selected Financial Analysis Results

Alternative	Funding Option	Average Annual Water Bill	Percent of MHI
Current	NA	NA \$299	
To meet current expenses	NA	\$175	0.4
Nearby well within	100% Grant	\$299	0.7
approximately 1 mile	Loan/Bond	\$2,756	1.9
Central treatment	100% Grant	\$3,215	8.1
Central treatment	Loan/Bond	\$5,226	13.1
Point-of-use	100% Grant	\$2,165	5.4
roint-or-use	Loan/Bond	\$2,316	5.8
Public dispenser	100% Grant	\$865	2.2
i done dispensei	Loan/Bond	\$900	2.3

1

TABLE OF CONTENTS

2	EXECUTIVE SUMMARY	1
3	Introduction	1
4	Study Methods	2
5	Hydrogeological Analysis	2
6	Compliance Alternatives	4
7	Financial Analysis	5
8	LIST OF TABLES	iv
9	LIST OF FIGURES	v
10	ACRONYMS AND ABBREVIATIONS	vii
11	SECTION 1 INTRODUCTION	1-1
12	1.1 Public Health and Compliance with MCLs	1-2
13	1.2 Methodology	1-5
14	1.3 Regulatory Perspective	1-5
15	1.4 Abatement Options	1-6
16	1.4.1 Existing Public Water Supply Systems	1-6
17	1.4.1.1 Quantity	1-6
18	1.4.1.2 Quality	1-7
19	1.4.2 Potential for New Groundwater Sources	1-7
20	1.4.2.1 Existing Non-Public Supply Wells	1-7
21	1.4.2.2 Develop New Wells	1-9
22	1.4.3 Potential for Surface Water Sources	1-9
23	1.4.3.1 Existing Surface Water Sources	1-9
24	1.4.3.2 New Surface Water Sources	1-10
25	1.4.4 Identification of Treatment Technologies	1-10
26	1.4.4.1 Treatment Technologies for Nitrates	1-10
27	1.4.4.2 Treatment Technologies for Arsenic	1-11
28	1.4.5 Treatment Technologies Description	1-12
29	1.4.5.1 Reverse Osmosis	1-12
30	1.4.5.2 Ion Exchange	1-14
31	1.4.5.3 Electrodialysis Reversal	1-15
32	1.4.5.4 Distillation	1-17
33	1.4.6 Point-of-Entry and Point-of-Use Treatment Systems	
34	1.4.7 Water Delivery or Central Drinking Water Dispensers	1-18

1	SECTION 2	EVALUATION METHODOLOGY	2-1
2	2.1 D	ecision Tree	2-1
3	2.2 D	ata Sources and Data Collection	2-1
4	2.2.1	Data Search	2-1
5	2.2.1.	1 Water Supply Systems	2-1
6	2.2.1.	2 Existing Wells	2-6
7	2.2.1.	3 Surface Water Sources	2-6
8	2.2.1.	4 Groundwater Availability Model	2-6
9	2.2.1.	5 Water Availability Model	2-6
10	2.2.1.	6 Financial Data	2-7
11	2.2.1.	7 Demographic Data	2-7
12	2.2.2	PWS Interviews	2-7
13	2.2.2.	1 PWS Capacity Assessment Process	2-7
14	2.2.2.	2 Interview Process	2-9
15	2.3 A	Iternative Development and Analysis	2-10
16	2.3.1	Existing PWS	2-10
17	2.3.2	New Groundwater Source	2-11
18	2.3.3	New Surface Water Source	2-11
19	2.3.4	Treatment	2-11
20	2.4 C	ost of Service and Funding Analysis	2-12
21	2.4.1	Financial Feasibility	2-12
22	2.4.2	Median Household Income	2-12
23	2.4.3	Annual Average Water Bill	2-13
24	2.4.4	Financial Plan Development	2-13
25	2.4.5	Financial Plan Results	2-14
26	2.4.5.	1 Funding Options	2-14
27	2.4.5.	2 General Assumptions Embodied in Financial Plan Results	2-15
28	2.4.5.	3 Interpretation of Financial Plan Results	2-15
29	2.4.5.	4 Potential Funding Sources	2-16
30	SECTION 3	UNDERSTANDING SOURCES OF CONTAMINANTS	3-1
31		itrate and Arsenic in the Southern High Plains and Edwards Trinity	
32		Plateau) Aquifers	3-1
33	3.2 G	eneral Trends in Nitrate Concentrations	3-2
34	3.3 G	eneral Trends in Arsenic Concentrations	3-8
35	3.4 D	etailed Assessment for South Midland County Water System, Inc.	3-12

1 2	SECTION 4	ANALYSIS OF THE SOUTH MIDLAND COUNTY WATER SYSTEM	4-1
3	4.1 De	escription of Existing System	4-1
4	4.1.1	Existing System	4-1
5	4.1.2	Capacity Assessment for South Midland County Water Systems, Inc	4-3
6	4.1.2.1	General Structure	4-3
7	4.1.2.2	2 General Assessment of Capacity	4-3
8	4.1.2.3	Positive Aspects of Capacity	4-3
9	4.1.2.4	4 Capacity Deficiencies	4-4
10	4.1.2.5	5 Potential Capacity Concerns	4-5
11	4.2 Al	ternative Water Source Development	4-6
12	4.2.1	Identification of Alternative Existing Public Water Supply Sources	4-6
13	4.2.1.1	Colorado River Municipal Water District	4-8
14	4.2.1.2	2 City of Midland	4-9
15	4.2.1.3	Midland International Airport	4-10
16	4.2.1.4	4 Canyon Dam Mobile Home Park	4-10
17	4.2.1.5	5 City of Odessa	4-11
18	4.2.2	Potential for New Groundwater Sources	4-11
19	4.2.2.1	Installing New Compliant Wells	4-11
20	4.2.2.2	Results of Groundwater Availability Modeling	4-12
21	4.2.3	Potential for New Surface Water Sources	4-13
22	4.2.4	Options for Detailed Consideration	4-13
23	4.3 Tr	eatment Options	4-14
24	4.3.1	Centralized Treatment Systems	4-14
25	4.3.2	Point-of-Use Systems	4-14
26	4.3.3	Point-of-Entry Systems	4-14
27	4.4 Bo	ottled Water	4-14
28	4.5 Al	ternative Development and Analysis	4-14
29	4.5.1	Alternative SMC-1: Purchase Treated Water from the City of Midland	l4-15
30	4.5.2	Alternative SMC-2: New Well at Midland International Airport	4-16
31	4.5.3	Alternative SMC-3: New Well at Canyon Dam Mobile Home Park	4-17
32	4.5.4	Alternative SMC-4: Purchase Treated Water from the City of Odessa.	4-17
33	4.5.5	Alternative SMC-5: Central RO Treatment	4-18
34	4.5.6	Alternative SMC-6: Central EDR Treatment	4-19
35	4.5.7	Alternative SMC-7: Point-of-Use Treatment	4-20
36	4.5.8	Alternative SMC-8: Point-of-Entry Treatment	4-21

	:		
1	4.5.9	Alternative SMC-9: New Well at 10 miles	4-22
2	4.5.10	O Alternative SMC-10: New Well at 5 miles	4-23
3	4.5.1	1 Alternative SMC-11: New Well at 1 mile	4-24
4	4.5.1	2 Alternative SMC-12: Public Dispenser for Treated Drinking Water	4-24
5	4.5.1	Alternative SMC-13: 100 Percent Bottled Water Delivery	4-25
6	4.5.1	4 Alternative SMC-14: Public Dispenser for Trucked Drinking Water	4-26
7	4.5.1	5 Summary of Alternatives	4-27
8	4.6	Cost of Service and Funding Analysis	4-30
9	4.6.1	South Midland County Financial Data	4-30
10	4.6.2	Current Financial Condition	4-30
11	4.6	.2.1 Cash Flow Needs	4-30
12	4.6	.2.2 Ratio Analysis	4-30
13	4.6.3	Financial Plan Results	4-31
14	SECTION	5 REFERENCES	5-1
1.5		APPENDICES	
15		APPENDICES	
16	• •	A PWS Interview Form	
17		B Cost Basis	
18		C Compliance Alternative Conceptual Cost Estimates	
19		D Example Financial Model	
20	Appendix 1	E General Geochemistry for Arsenic and Nitrate	
21			
22		LIST OF TABLES	
23	Table ES 1	South Midland County PWS Basic System Information	1
24		Selected Financial Analysis Results	
25		Nitrate-N Concentrations in the South Midland County PWS (TCEQ	
26	14010 011	Database)	3-12
27 28	Table 3.2	Arsenic Concentrations in the South Midland County PWS (TCEQ Database)	3-14
29 30	Table 4.1	Existing Public Water Systems within 20 miles of South Midland County Water System	
31 32	Table 4.2	Public Water Systems within 20 miles of South Midland County Water System Selected for further Evaluation	4-8
33 34	Table 4.3	Summary of Compliance Alternatives for South Midland County Water System	

1	Table 4.4 F	inancial Impact on Households for South Midland County Water System	. 4-32
2			
3		LIST OF FIGURES	
4	Figure ES-1	Summary of Project Methods	3
5	Figure 1.1	South Midland County Water System Location Map	1-3
6	Figure 1.2	South Midland Groundwater Conservation Districts and Planning Groups .	1-4
7	Figure 2.1	Decision Tree – Tree 1 Existing Facility Analysis	2-2
8	Figure 2.2	Decision Tree – Tree 2 Develop Treatment Alternatives	2-3
9	Figure 2.3	Decision Tree – Tree 3 Preliminary Analysis	2-4
10	Figure 2.4	Decision Tree – Tree 4 Financial	2-5
11	Figure 3.1	Public Water Supplies and Major Aquifers in the Study Area	3-1
12 13	Figure 3.2	Detectable Nitrate-N Concentrations in Groundwater (TWDB Database, Analyses from 1937 through 2004)	3-3
14	Figure 3.3	Wells with Nitrate Samples Categorized by Aquifer	3-4
15	Figure 3.4	Distribution of Nitrate-N Concentrations	3-5
16 17	Figure 3.5	Correlation of Nitrate with Chloride, Sulfate, and TDS in the Ogallala Aquifer	3-5
18 19	Figure 3.6	Relationship between Nitrate-N Concentrations and Well Depth in the Ogallala Aquifer	3-6
20 21	Figure 3.7	Relationship Between Nitrate-N Concentrations and Well Depth in the Edwards-Trinity (Plateau) Aquifer	3- <i>6</i>
22 23	Figure 3.8	Spatial Relationship Between Land Cover (NLCD) and Nitrate-N Concentrations	3-7
24	Figure 3.9	Relationship Between Nitrate-N Concentrations and Land Use	3-8
25	Figure 3.10	Spatial Distribution of Arsenic Concentrations (TWDB Database)	3-9
26 27	Figure 3.11	Probabilities of Arsenic Concentrations Exceeding 10 µg/L MCL for Aquifers in the Study Area	3-9
28	Figure 3.12	Relationship Between Arsenic Concentrations and Well Depth	. 3-10
29 30	Figure 3.13	Relationship Between Arsenic and Fluoride, Molybdenum, and Vanadium within the Ogallala Aquifer	
31 32	Figure 3.14	Nitrate-N Concentrations in 5- and 10-km Buffers of South Midland County PWS Wells (TWDB and TCEQ Databases)	. 3-13
33 34	Figure 3.15	Arsenic Concentrations in 5- and 10-km Buffers of South Midland County PWS Wells (TWDB and TCEQ Databases)	
35	Figure 4.1	South Midland Pipeline Alternatives	
36	Figure 4.2	Alternative Costs Summary: South Midland County Water System	4-34

1 ACRONYMS AND ABBREVIATIONS

°F	Degrees Fahrenheit
μg/L	Microgram per liter
AA	Activated Alumina
BAT	Best available technology
BEG	Bureau of Economic Geology
CA	Cellulose acetate
CCN	Certificate of Convenience and Necessity
CFR	Code of Federal Regulations
CO	Correspondence
CRMWD	Colorado River Municipal Water District
EDR	Electrodialysis reversal
FMT	Financial, managerial, and technical
GAM	Groundwater Availability Model
gpm	Gallons per minute
ISD	Independent School District
IX	Ion exchange
MCL	Maximum contaminant level
MF	Microfiltration
mg/L	Milligram per liter
mgd	Million gallons per day
MHI	Median household income
MIWA	Municipal and Industrial Water Authority
MOR	Monthly operating report
NF	Nanofiltration
NMEFC	New Mexico Environmental Financial Center
O&M	Operation and Maintenance
POE	Point-of-entry
POU	Point-of-use
psi	Pounds per square inch
PVC	Polyvinyl chloride
PWS	Public water system
RO	Reverse osmosis
RRA	Red River Authority
SDWA	Safe Drinking Water Act
TCEQ	Texas Commission on Environmental Quality
TDS	Total dissolved solids
TFC	Thin film composite
TSS	Total suspended solids

TWD	В	Texas Water Development Board
USEP	Α	U.S. Environmental Protection Agency
WA	M	Water Availability Model

1 2

SECTION 1 INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), have been contracted by the Texas Commission on Environmental Quality (TCEQ) to assist with identifying and analyzing compliance alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards. A total of 15 PWSs were evaluated in this project and each is addressed in a separate report. The 15 systems evaluated for this project are listed below:

Public Water System	Texas County
City of Eden	Concho
City of Danbury	Brazoria
Rosharon Road Estates Subdivision	Brazoria
Mark V Estates	Brazoria
Rosharon Township	Brazoria
Sandy Meadows Estates Subdivision	Brazoria
Grasslands	Brazoria
City of Mason	Mason
Falling Water	Kerr
Greenwood Independent School District (ISD)	Midland
Country Village Mobile Home Estates	Midland
South Midland County Water Systems	Midland
Warren Road Subdivision Water Supply	Midland
Huber Garden Estates	Ector
Devilla Mobile Home Park	Ector

The overall goal of this project is to promote compliance using sound engineering and financial methods and data for PWSs that have recently had sample results that exceed maximum contaminant levels (MCL). The primary objectives of this project are to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS with regard to future implementation. The feasibility studies identify a range of potential compliance alternatives, and present basic data that can be used for evaluating feasibility. The compliance alternatives addressed include a description of what would be required for implementation, conceptual cost estimates for implementation, and non-cost factors that

1 2

could be used to differentiate between alternatives. The cost estimates are intended for comparing compliance alternatives, and to give a preliminary indication of potential impacts on water rates resulting from implementation.

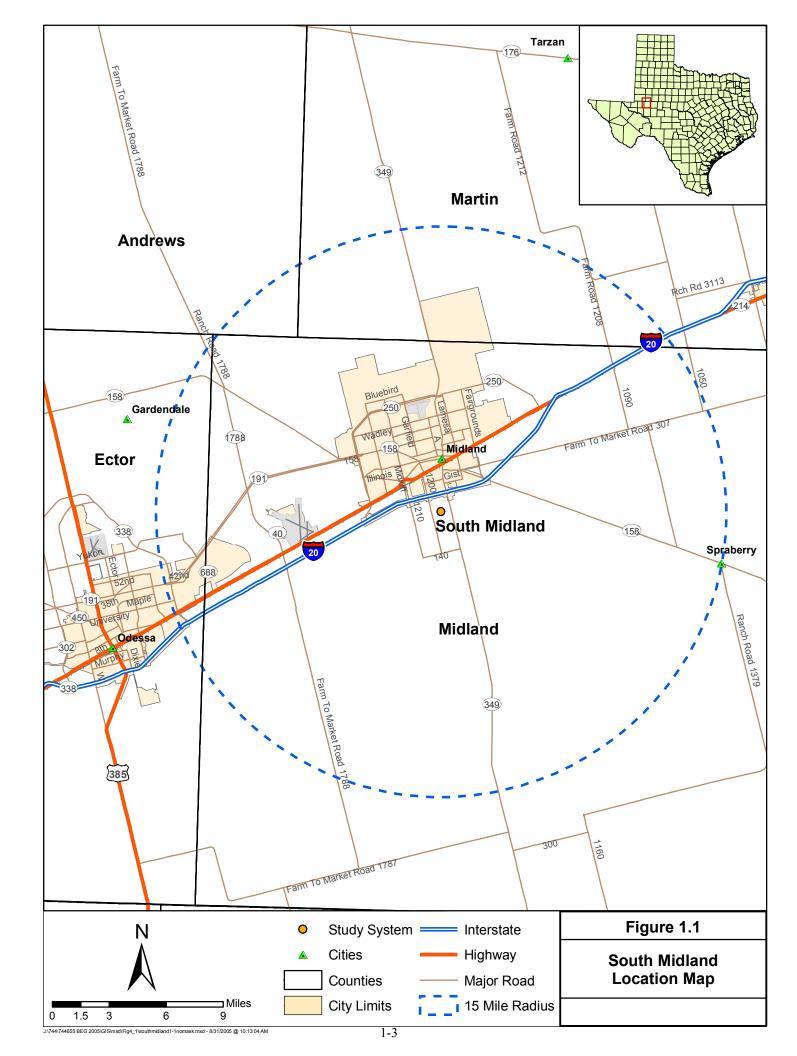
It is anticipated that the PWS will review the compliance alternatives in this report to determine if there are promising alternatives, and then select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation. This report contains a decision tree approach that guided the efforts for this study, and also contains steps to guide a PWS through the subsequent evaluation, selection, and implementation of a compliance alternative.

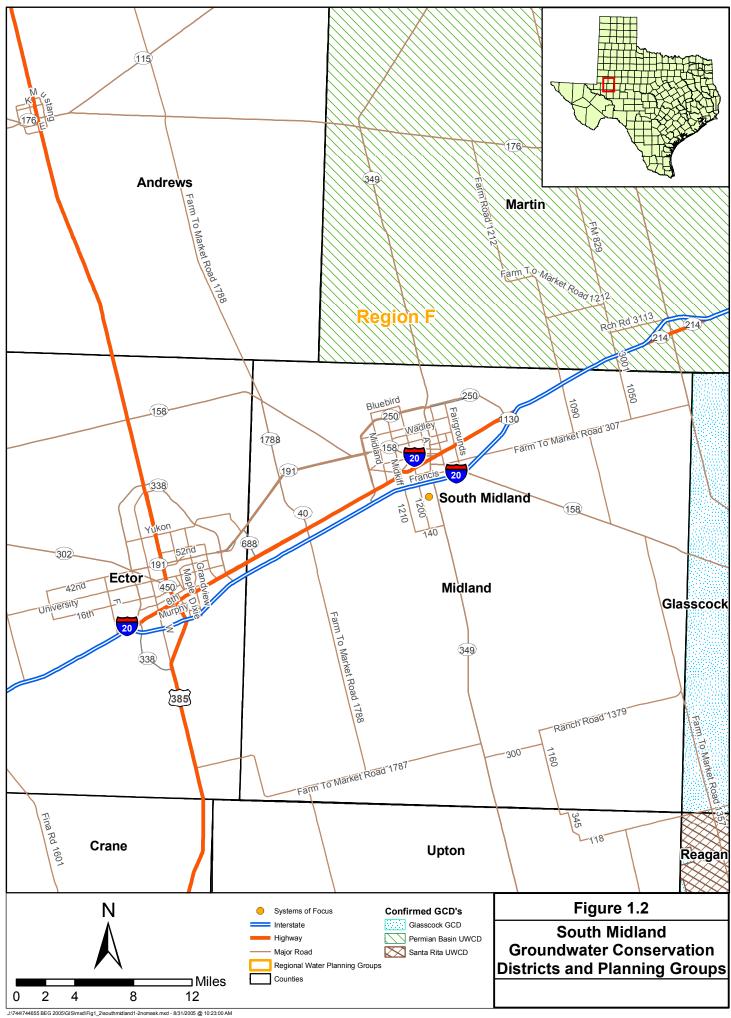
This feasibility report provides an evaluation of water supply compliance options for the South Midland County Water System, PWS ID# 1650077, Certificate of Convenience and Necessity (CCN) # U0181, located in Midland County. Recent sample results from the South Midland County Water System have exceeded the MCL for arsenic of 10 micrograms per liter (µg/L) that will go into effect on January 23, 2006 (USEPA 2005a; TCEQ 2004a). Other recent sample results from the South Midland County Water System also exceeded the MCL for nitrate of 10 milligrams per liter (mg/L) and the MCL for total dissolved solids (TDS) of 1,000 mg/L (USEPA 2005a; TCEQ 2004a). The location of the South Midland County Water System, also referred to as the "study area" in this report, is shown on Figure 1.1. Various water supply and planning jurisdictions are shown on Figure 1.2. These water supply and planning jurisdictions are used in the evaluation of alternate water supplies that may be available in the area.

1.1 PUBLIC HEALTH AND COMPLIANCE WITH MCLS

The goal of this project is to promote compliance for PWSs that supply drinking water exceeding regulatory MCLs. This project only addresses those contaminants and does not address any other violations that may exist for a PWS. As mentioned above, South Midland County Water System had recent sample results exceeding the MCLs for nitrate and arsenic. Health concerns related to drinking water above MCLs for these two chemicals are briefly described below.

In general, contaminant(s) in drinking water above the MCL(s) can have both short-term (acute) and long-term or lifetime (chronic) effects. Short-term effects of nitrate in drinking water above the MCL have caused serious illness and sometimes death. Drinking water health publications conclude that the most susceptible population to adverse nitrate health effects include infants less than 6 months of age; women who are pregnant or nursing; and individuals with enzyme deficiencies or a lack of free hydrochloric acid in the stomach. The serious illness in infants is due to the conversion of nitrate to nitrite by the body, which can interfere with the oxygen-carrying capacity of the child's blood. Symptoms include shortness of breath and blue-baby syndrome. Lifetime exposure to nitrates at levels above the MCL has the potential to cause the following effects: diuresis, increased starchy deposits, and hemorrhaging of the spleen (USEPA 2005a; 2005b).





Potential health effects from long-term ingestion of water with levels of arsenic above the MCL (0.01 mg/L) include non-cancerous effects, such as cardiovascular, pulmonary, immunological, neurological and endocrine effects, and cancerous effects, including skin, bladder, lung, kidney, nasal passage, liver, and prostate cancer (USEPA 2005c).

1.2 METHODOLOGY

1 2

3

4 5

6

7

8 9

10

11

12

13

14

16

17

18

19

23

24

25

2627

28

29

30

31

32

33

34

35

36

The methodology for this project follows that of the pilot study performed in 2004 and 2005 by TCEQ, BEG, and Parsons. The pilot study evaluated water supply alternatives for PWSs that supply drinking water with nitrate concentrations above USEPA and Texas drinking water standards. Three PWSs were evaluated in the pilot study to develop the methodology (*i.e.*, decision tree approach) for analyzing options for provision of compliant drinking water. This project is performed using the decision tree approach developed in the pilot study.

- Other tasks of the feasibility study are as follows:
- Identifying available data sources;
 - Gathering and compiling data;
 - Conducting financial, managerial, and technical (FMT) evaluations of the selected PWSs;
 - Performing a geologic and hydrogeologic assessment of the study area;
- Developing treatment and non-treatment compliance alternatives;
- Assessing potential alternatives with respect to economic and noneconomic criteria;
 - Preparing a feasibility report; and
 - Suggesting refinements to the approach for future studies.

The remainder of Section 1 of this report addresses the regulatory background, and provides a summary of nitrate and arsenic abatement options. Section 2 describes the methodology used to develop and assess compliance alternatives. The groundwater sources of nitrate and arsenic are addressed in Section 3. Findings for the South Midland County Water System, along with compliance alternatives development and evaluation, can be found in Section 4. Section 5 references the sources used in this report.

1.3 REGULATORY PERSPECTIVE

- The Utilities & Districts and Public Drinking Water Sections of the TCEQ Water Supply Division are responsible for implementing the federal Safe Drinking Water Act (SDWA) requirements that include oversight of PWSs and water utilities. These responsibilities include:
- Monitoring public drinking water quality;

- Processing enforcement referrals for MCL violators;
- Tracking and analyzing compliance options for MCL violators;
- Providing FMT assessment and assistance to PWSs;
- Participating in the Drinking Water State Revolving Fund program to assist PWSs in achieving regulatory compliance; and
 - Setting rates for privately owned water utilities.
- 7 This project was conducted to assist in achieving these responsibilities.

1.4 ABATEMENT OPTIONS

When a PWS exceeds a regulatory MCL, the PWS must take action to correct the violation. The MCL exceedances at the South Midland County PWS include nitrates and arsenic. The following subsections explore alternatives considered as potential options for obtaining/providing compliant drinking water.

1.4.1 Existing Public Water Supply Systems

A common approach to achieve compliance is for the PWS to make arrangements with a neighboring PWS for water supply. For this arrangement to work, the PWS from which water is being purchased (supplier PWS) must have water in sufficient quantity and quality, the political will must exist, and it must be economically feasible.

1.4.1.1 Quantity

6

8

13

14

15

16

17

18

19

20

21

22

23

24

25

26

27

28

29

30

31

32

33

34

35

For purposes of this report, quantity refers to water volume, flowrate, and pressure. Before approaching a potential supplier PWS, the non-compliant PWS should determine its water demand on the basis of average day and maximum day. Peak instantaneous demands can be met through proper sizing of storage facilities. Further, the potential for obtaining the appropriate quantity of water to blend to achieve compliance should be considered. The concept of blending involves combining water with low levels of contaminants with non-compliant water in sufficient quantity that the resulting blended water is compliant. The exact blend ratio would depend on the quality of the water a potential supplier PWS can provide, and would likely vary over time. If high quality water is purchased, produced or otherwise obtained, blending can reduce the amount of high quality water required. Implementation of blending will require a control system to ensure the blended water is compliant.

If the supplier PWS does not have sufficient quantity, the non-complaint community could pay for the facilities necessary to increase the quantity to the extent necessary to supply the needs of the non-compliant PWS. Potential improvements might include, but are not limited to:

- Additional wells;
- Developing a new surface water supply;

- Additional or larger-diameter piping;
- Increasing water treatment plant capacity;
- Additional storage tank volume;
 - Reduction of system losses;
 - Higher-pressure pumps; or
 - Upsized, or additional, disinfection equipment.

In addition to the necessary improvements, a transmission pipeline would need to be constructed to tie the two PWSs together. The pipeline must tie-in at a point in the supplier PWS where all the upstream pipes and appurtenances are of sufficient capacity to handle the new demand. In the non-compliant PWS, the pipeline must tie in at a point where no down stream bottlenecks are present. If blending is the selected method of operation, the tie-in point must be at the proper point of the existing non-compliant PWS to ensure that all the water in the system is blended to achieve regulatory compliance.

1.4.1.2 Quality

If a potential supplier PWS obtains its water from the same aquifer (or same portion of the aquifer) as the non-compliant PWS, the quality of water may not be significantly better. However, water quality can vary significantly due to well location, even within the same aquifer. If localized areas with good water quality cannot be identified, the non-compliant PWS would need to find a potential supplier PWS that obtains its water from a different aquifer or from a surface water source. Additionally, a potential supplier PWS may treat non-compliant raw water to an acceptable level.

Surface water sources may offer a potential higher-quality source. Since there are significant treatment requirements, utilization of surface water for drinking water is typically most feasible for larger local or regional authorities or other entities that may provide water to several PWSs. Where PWSs that obtain surface water are neighbors, the non-compliant PWS may need to deal with those systems as well as with the water authorities that supply the surface water.

1.4.2 Potential for New Groundwater Sources

1.4.2.1 Existing Non-Public Supply Wells

Often there are wells not associated with PWSs that are located in the vicinity of the non-compliant PWS. The current use of these wells may be for irrigation, industrial purposes, domestic supply, stock watering, and other purposes. The process for investigating existing wells is as follows:

• Use existing data sources (see below) to identify wells in the areas that have satisfactory quality. The following standards could be used in a rough screening for compliant groundwater:

o Nitrate (measured as nitrogen) concentrations less than 8 mg/L (below 1 2 the MCL of 10 mg/L); 3 o Arsenic concentrations less than 0.008 mg/L (below the MCL of 0.01 mg/L); and 4 5 o TDS concentrations less than 1,000 mg/L. 6 Review the recorded well information to eliminate those wells that appear to be unsuitable for the application. Often, the "Remarks" column in the 7 8 Texas Water Development Board (TWDB) hard-copy database provides 9 helpful information. Wells eliminated from consideration generally include domestic and stock wells, dug wells, test holes, observation wells, 10 seeps and springs, destroyed wells, wells used by other communities, etc. 11 12 Identify wells of sufficient size which have been used for industrial or irrigation purposes. Often the TWDB database will include well yields, 13 which may indicate the likelihood that a particular well is a satisfactory 14 15 source. 16 At this point in the process, the local groundwater control district (if one 17 exists) should be contacted to obtain information about pumping restrictions. Also, preliminary cost estimates should be made to establish 18 the feasibility of pursuing further well development options. 19 20 If particular wells appear to be acceptable, the owner(s) should be 21 contacted to ascertain their willingness to work with the PWS. Once the 22 owner agrees to participate in the program, questions should be asked 23 about the wells. Many owners have more than one well, and would 24 probably be the best source of information regarding the latest test dates, 25 who tested the water, flowrates, and other well characteristics. 26 After collecting as much information as possible from cooperative owners, 27 the PWS would then narrow the selection of wells and sample and analyze 28 them for quality. Wells with good quality would then be potential 29 candidates for test pumping. In some cases, a particular well may need to be refurbished before test pumping. 30 Information obtained from test 31 pumping would then be used in combination with information about the 32 general characteristics of the aquifer to determine whether a well at this 33 location would be suitable as a supply source. 34 It is recommended that new wells be installed instead of using existing wells to ensure the well characteristics are known and the well meets 35 36 construction standards. 37 Permit(s) would then be obtained from the groundwater control district or 38 other regulatory authority, and an agreement with the owner (purchase or

lease, access easements, etc.) would then be negotiated.

39

1.4.2.2 Develop New Wells

If no existing wells are available for development, the PWS or group of PWSs has an option of developing new wells. Records of existing wells, along with other hydrogeologic information and modern geophysical techniques, should be used to identify potential locations for new wells. In some areas, the TWDB's Groundwater Availability Model (GAM) may be applied to indicate potential sources. Once a general area has been identified, land owners and regulatory agencies should be contacted to determine an exact location for a new well or well field. Pump tests and water quality tests would be required to determine if a new well will produce an adequate quantity of good quality water. Permits from the local groundwater control district or other regulatory authority could also be required for a new well.

1.4.3 Potential for Surface Water Sources

Water rights law dominates the acquisition of water from surface water sources. For a PWS, 100 percent availability of water is required, except where a back-up source is available. For PWSs with an existing water source, although it may be non-compliant because of elevated concentrations of one or more parameters, water rights may not need to be 100 percent available.

1.4.3.1 Existing Surface Water Sources

"Existing surface water sources" of water refers to municipal water authorities and cities that obtain water from surface water sources. The process of obtaining water from such a source is generally less time consuming and less costly than the process of developing a new source; therefore, it should be a primary course of investigation. An existing source would be limited by its water rights, the safe yield of a reservoir or river, or by its water treatment or water conveyance capability. The source must be able to meet the current demand and honor contracts with communities it currently supplies. In many cases, the contract amounts reflect projected future water demand based on population or industrial growth.

A non-compliant PWS would look for a source with sufficient spare capacity. Where no such capacity exists, the non-compliant PWS could offer to fund the improvements necessary to obtain the capacity. This approach would work only where the safe yield could be increased (perhaps by enlarging a reservoir) or where treatment capacity could be increased. In some instances water rights, where they are available, could possibly be purchased.

In addition to securing the water supply from an existing source, the non-compliant PWS would need to arrange for transmission of the water to the PWS. In some cases, that could require negotiations with, contracts with, and payments to an intermediate PWS (an intermediate PWS is one where the infrastructure is used to transmit water from a "supplier" PWS to a "supplied" PWS, but does not provide any additional treatment to the supplied water). The non-compliant PWS could be faced with having to fund

improvements to the intermediate PWS in addition to constructing its own necessary transmission facilities.

1.4.3.2 New Surface Water Sources

3

4

5

6

7

8

9

10

11

12

13

14

1516

17

18

19

20

21

22

23

24

25

2627

28 29

30

Communication with the TCEQ and relevant planning groups from the beginning is essential in the process of obtaining a new surface water source. Preliminary assessment of the potential for acquiring new rights may be based on surface water availability maps located on the TWDB website. Where water rights appear to be available, the following activities need to occur:

- Discussions with TCEQ to indicate the likelihood of obtaining those rights. The TCEQ may use the Water Availability Model (WAM) to assist in the determination.
- Discussions with land owners to indicate potential treatment plant locations.
- Coordination with US Army Corps of Engineers and local river authorities.
- Preliminary engineering design to determine the feasibility, costs, and environmental issues of a new intake, treatment plant, and conveyance system.

Should these discussions indicate that a new surface water source is the best option, the community would proceed with more intensive planning (initially obtaining funding), permitting, land acquisition, and detailed designs.

1.4.4 Identification of Treatment Technologies

Various treatment technologies were also investigated as compliance alternatives for treatment of nitrate and arsenic to regulatory levels (*i.e.*, MCLs). Numerous options have been identified by the USEPA as best available technologies (BAT) for non-compliant constituents. Identification and descriptions of the various BATs are provided in the following sections. Several other treatment options are also described but were not further considered in the feasibility study (*e.g.*, because of lack of commercial applications or other limitations).

1.4.4.1 Treatment Technologies for Nitrates

- The MCL for nitrate (as nitrogen) was set at 10 mg/L by the USEPA on January 30, 1992, as part of the Phase II Rules, and became effective on July 30, 1992 (USEPA 1992). This MCL applies to all community water systems, regardless of size.
- BATs identified by USEPA for removal of nitrates include:
- o Reverse Osmosis (RO);
- o Ion Exchange (IX); and

o Electrodialysis Reversal (EDR).

1.4.4.2 Treatment Technologies for Arsenic

In January 2001, the USEPA published a final rule in the Federal Register that established an MCL for arsenic of 0.01 mg/L (USEPA 2001). The regulation applies to all community water systems and non-transient, non-community water systems, regardless of size.

The new arsenic MCL of 0.01 mg/L becomes effective January 23, 2006, at which time the running average annual arsenic level must be at or below 0.01 mg/L at each entry point to the distribution system, although point-of-use (POU) treatment can be instituted in place of centralized treatment. All surface water systems must complete initial monitoring for the new arsenic MCL or have a state-approved waiver by December 31, 2006. All groundwater systems must complete initial monitoring or have a state-approved waiver by December 31, 2007.

The following BATs were identified in the final rule for achieving compliance with the arsenic MCL:

1

2

3

4

5

6

7

8

9

10

11

12 13

14

15

- 18 o EDR;
- o Activated Alumina (AA);
- 0 Oxidation/Filtration;
- o Enhanced Coagulation/Filtration; and
- o Enhanced Lime Softening.

In addition, the following technologies are listed in the final rule as Small System Compliance Technologies:

- o RO (centralized and POU);
- 26 o IX;
- 27 o EDR:
- o AA (centralized and POU);
- 29 o Oxidation/Filtration;
- Coagulation/Filtration, Enhanced Coagulation/Filtration, and
 Coagulation-Assisted Microfiltration; and
- o Lime Softening and Enhanced Lime Softening.

1.4.5 Treatment Technologies Description

Reverse osmosis, IX, and EDR are identified by USEPA as BATs for removal of nitrates. These three treatment technologies are also applicable to arsenic, and are the only three technologies common to both nitrate and arsenic treatment. RO and IX are also viable options for point-of-entry (POE) and POU systems. A description of these technologies follows.

1.4.5.1 Reverse Osmosis

1

2

3

4

5

7

8 9

10

11

12

13

14

15

16

17 18

19

20

21

22

2324

25

26

27

28

29

30

31

32

33

34

35

36

37

38

39

40

41

Process. RO is a physical process in which contaminants are removed by applying pressure on the feed water to force it through a semi-permeable membrane. RO membranes reject ions based on size and electrical charge. The raw water is typically called feed; the product water is called permeate; and the concentrated reject is called concentrate. Common RO membrane materials include asymmetric cellulose acetate (CA) or polyamide thin film composite (TFC). The TFC membrane operates at much lower pressure and can achieve higher salt rejection than the CA membranes but is less chlorine resistant. Common membrane construction includes spiral wound or hollow fine fiber. Each material and construction method has specific benefits and limitations depending on the raw water characteristics and pre-treatment. Spiral wound has been the dominant configuration in common RO systems. A newer, lower pressure type membrane which is similar in operation to RO, is nanofiltration (NF) which has higher rejection for divalent ions than mono-valent ions. NF is sometimes used instead of RO for treating water with high hardness and sulfate concentrations. installation includes a high pressure feed pump; parallel first and second stage membrane elements (in pressure vessels); and valves and piping for feed, permeate, and concentrate streams. Factors influencing membrane selection are cost, recovery, rejection, raw water characteristics, and pre-treatment. Factors influencing performance are raw water characteristics, pressure, temperature, and regular monitoring and maintenance. Depending on the membrane type and operating pressure, RO is capable of removing 95 percent of nitrate and arsenic while NF has a lower nitrate and arsenic rejection efficiency. The treatment process is relatively insensitive to pH. Water recovery is 60-80 percent, depending on raw water characteristics. The concentrate volume for disposal can be significant. The conventional RO treatment train for well water uses anti-scalant addition, cartridge filtration, RO membranes, chlorine disinfection, and clearwell storage.

<u>Pre-treatment</u>. RO requires careful review of raw water characteristics, and pre-treatment needs to prevent membranes from fouling, scaling, or other membrane degradation. Removal or sequestering of suspended solids is necessary to prevent colloidal and bio-fouling, and removal of sparingly soluble constituents such as calcium, magnesium, silica, sulfate, barium, *etc.*, may be required to prevent scaling. Pretreatment can include media filters to remove suspended particles; IX softening to remove hardness; antiscalant feed; temperature and pH adjustment to maintain efficiency; acid to prevent scaling and membrane damage; activated carbon or bisulfite to remove chlorine

(post-disinfection may be required); and cartridge filters to remove any remaining suspended particles to protect membranes from upsets.

Maintenance. Rejection percentages must be monitored to ensure contaminant removal below MCLs. Regular monitoring of membrane performance is necessary to determine fouling, scaling, or other membrane degradation. Use of monitoring equipment to track membrane performance is recommended. Acidic or caustic solutions are regularly flushed through the system at high volume/low pressure with a cleaning agent to remove fouling and scaling. The system is flushed and returned to service. RO stages are cleaned sequentially. Frequency of membrane replacement is dependent on raw water characteristics, pre-treatment, and maintenance.

<u>Waste Disposal</u>. Pre-treatment waste streams, concentrate flows, and spent filters and membrane elements all require approved disposal methods. Disposal of the significant volume of the concentrate stream is a problem for many utilities.

Advantages (RO)

1 2

3

4

5

6 7

8

9

10

11

12

13

14

15

16

17

18

19

20

21

22

23

24

28

29

30

31

32

33

34

35

- o Produces the highest water quality.
- Can effectively treat a wide range of dissolved salts and minerals, turbidity, health and aesthetic contaminants, and certain organics.
 Some highly-maintained units are capable of treating biological contaminants.
- Low pressure less than 100 pounds per square inch (psi), compact, self-contained, single membrane units are available for small installations.

Disadvantages (RO)

- o Relatively expensive to install and operate.
- 25 o Frequent membrane monitoring and maintenance; pressure,
 26 temperature, and pH requirements to meet membrane tolerances.
 27 Membranes can be chemically sensitive.
 - o Additional water usage depending on rejection rate.
 - o Concentrated disposal.

A concern with RO for treatment of inorganics is that if the full stream is treated, then most of the alkalinity and hardness would also be removed. In that event, post-treatment may be necessary to avoid corrosion problems. If feasible, a way to avoid this issue is to treat a slip stream of raw water and blend the slip stream back with the raw water rather than treat the full stream. The amount of water rejected is also an issue with RO. Discharge concentrate can be between 10 and 50 percent of the influent flow.

1.4.5.2 Ion Exchange

1

2

3

5 6

7

8

9

10 11

12 13

14

15 16

17

18

19

20

21

22

23

24

25

26

27

28

29

30

31 32

33

34

35 36

37

38

Process. In solution, salts separate into positively charged cations and negatively charged anions. Ion exchange is a reversible chemical process in which ions from an insoluble, permanent, solid resin bed are exchanged for ions in water. The process relies on the fact that certain ions are preferentially adsorbed on the IX resin. Operation begins with a fully recharged cation or anion resin bed, having enough positively or negatively charged ions to carry out the cation or anion exchange. Usually a polymer resin bed is composed of millions of spherical beads about the size of medium sand grains. As water passes through the resin bed, the positively or negatively charged ions are released into the water, being substituted or replaced with the contaminant ions in the water (ion exchange). When the resin becomes exhausted of positively or negatively charged ions, the bed must be regenerated by passing a strong, usually sodium chloride, solution over the resin bed, displacing the contaminant ions with sodium ions for cation exchange and chloride ions for anion exchange. Many different types of resins can be used to reduce dissolved contaminant concentrations. The IX treatment train for groundwater typically includes cation or anion resin beds, chlorine disinfection, and clearwell storage. Treatment trains for surface water may also include raw water pumps, debris screens, and gravity filters for pre-treatment. Additional treatment or management of the concentrate and the removed solids will be necessary prior to disposal. For nitrate and arsenic removal, a strong base anion exchange resin in the chloride form can remove 99 percent of the nitrate and arsenic. Sulfate is a strong competing anion for nitrate and arsenic adsorption by IX. Regeneration is accomplished with sodium chloride.

<u>Pre-treatment</u>. There are pretreatment requirements pH, organics, turbidity, and other raw water characteristics. Pre-treatment may be required to reduce excessive amounts of total suspended solids (TSS), iron, and manganese, which could plug the resin bed, and typically includes media or carbon filtration. Pre-treatment may also be required to remove sulfate that can interfere with nitrate and arsenic removal.

<u>Maintenance</u>. The IX resin requires regular on-site regeneration, the frequency of which depends on raw water characteristics, the contaminant concentration, and the size and number of IX vessels. Many systems have undersized the IX vessels only to realize higher than necessary operating costs. Preparation of the sodium chloride solution is required. If used, filter replacement and backwashing would be required.

<u>Waste Disposal</u>. Approval from local authorities is usually required for disposal of concentrate from the regeneration cycle (highly concentrated salt solution); occasional solid waste (in the form of broken resin beads) which is backwashed during regeneration; and if used, spent filters and backwash wastewater.

Advantages (IX)

- Acid addition, degasification, and repressurization are not required.
- o Ease of operation; highly reliable.
- o Lower initial cost; resins will not wear out with regular regeneration.

- o Effective; widely used. 1 2
 - o Suitable for small and large installations.
 - o A variety of specific resins are available for removing specific contaminants.

Disadvantages (IX)

3

4

5

6

8

10

11

12 13

14 15

16

17

18

19

20

21

22

23

24

25

26

27

28

29

30

31

32

33

34

35

36

37

38

39

40

- o Requires salt storage; regular regeneration.
- 7 o Concentrate disposal.
 - o Usually not feasible with high levels of TDS.
- 9 o Resins are sensitive to the presence of competing ions.

In considering application of IX for inorganics removal, it is important to understand what the effect of competing ions would be, and to what extent the brine can be recycled. Similar to AA, IX exhibits a selectivity sequence, which refers to an order in which ions are preferred. Barium, lead, and copper are highly preferred cations. Sulfate competes with both nitrate and arsenic, but more aggressively with arsenic in anion exchange. Source waters with TDS levels above 500 mg/L and sulfate levels above 120 mg/L are not amenable to IX treatment. Spent regenerant is produced during IX bed regeneration, and this spent regenerant may have high concentrations of sorbed contaminants which can be expensive to treat and/or dispose. Research has been conducted to minimize this effect; recent research on arsenic removal shows that the brine can be reused as many as 25 times.

1.4.5.3 Electrodialysis Reversal

EDR is an electrochemical process in which ions migrate through Process. ion-selective semi-permeable membranes as a result of their attraction to two electrically charged electrodes. A typical EDR system includes a membrane stack with a number of cell pairs, each consisting of a cation transfer membrane, a demineralized flow spacer, an anion transfer membrane, and a concentrate flow spacer. Electrode compartments are at opposite ends of the stack. The influent feed water (chemically treated to prevent precipitation) and the concentrated reject flow in parallel across the membranes and through the demineralized and concentrate flow spacers, respectively. The electrodes are continually flushed to reduce fouling or scaling. Careful consideration of flush feed water is required. Typically, the membranes are cation or anion exchange resins cast in sheet form; the spacers are high density polyethylene; and the electrodes are inert metal. EDR stacks are tank-contained and often staged. Membrane selection is based on review of raw water characteristics. A single-stage EDR system usually removes 40-50 percent of nitrate, arsenic, and TDS. Additional stages are required to achieve higher removal efficiency if necessary. EDR uses the technique of regularly reversing the polarity of the electrodes, thereby freeing accumulated ions on the membrane surface. This process requires additional plumbing and electrical controls, but it increases membrane life, may require less added chemicals, and eases cleaning. The conventional EDR treatment train typically includes EDR membranes, chlorine disinfection, and clearwell storage.

Treatment of surface water may also require pre-treatment steps such as raw water pumps, debris screens, rapid mix with addition of an anti-scalant, slow mix flocculator, sedimentation basin or clarifier, and gravity filters. Microfiltration (MF) could be used in place of flocculation, sedimentation, and filtration. Additional treatment or management of the concentrate and the removed solids would be necessary prior to disposal.

<u>Pre-treatment.</u> There are pretreatment requirements for pH, organics, turbidity, and other raw water characteristics. EDR typically requires chemical feed to prevent scaling, acid addition for pH adjustment, and a cartridge filter for prefiltration.

Maintenance. EDR membranes are durable, can tolerate a pH range from 1 to 10, and temperatures to 115 degrees Fahrenheit (°F) for cleaning. They can be removed from the unit and scrubbed. Solids can be washed off by turning the power off and letting water circulate through the stack. Electrode washes flush out byproducts of electrode reaction. The byproducts are hydrogen, formed in the cathode space, and oxygen and chlorine gas, formed in the anode space. If the chlorine is not removed, toxic chlorine gas may form. Depending on raw water characteristics, the membranes would require regular maintenance or replacement. EDR requires reversing the polarity. Flushing at high volume/low pressure continuously is required to clean electrodes. If used, pretreatment filter replacement and backwashing would be required. The EDR stack must be disassembled, mechanically cleaned, and reassembled at regular intervals.

<u>Waste Disposal</u>. Highly concentrated reject flows, electrode cleaning flows, and spent membranes require approved disposal methods. Pre-treatment processes and spent materials also require approved disposal methods.

Advantages (EDR)

6 7

8

9

10

11

12

13

14

15

16

17

18

19

20

21

22

23

24

28

29

30

31

32

33

34

35

- o EDR can operate with minimal fouling or scaling, or chemical addition.
- o Low pressure requirements; typically quieter than RO.
- o Long membrane life expectancy; EDR extends membrane life and reduces maintenance.
 - o More flexible than RO in tailoring treated water quality requirements.

Disadvantages (EDR)

- o Not suitable for high levels of iron, manganese, and hydrogen sulfide.
- o High energy usage at higher TDS water.

EDR can be quite expensive to run because of the energy it uses. However, because it is generally automated and allows for part-time operation, it may be an appropriate technology for small systems. It can be used to simultaneously reduce nitrate, TDS, and arsenic.

1.4.5.4 Distillation

Distillation heats water until it turns to steam. The steam travels through a condenser coil where it is cooled and returned to liquid. The nitrate and arsenic remain in the boiler section. Distillation is energy-intensive in relation to the other processes, but not well suited for production of drinking water for the centralized-treatment, POU, or POE applications.

Owing to the lack of commercial applications for this technology, it will be eliminated from further consideration.

1.4.6 Point-of-Entry and Point-of-Use Treatment Systems

Point-of-entry and POU treatment systems can be used to provide compliant drinking water. For nitrate and arsenic removal, these systems typically use small RO treatment units installed "under the sink" in the case of POU, and where water enters a residence or building in the case of POE. It should be noted that the POU treatment units would need to be more complex than units typically found in commercial retail outlets in order to meet regulatory requirements, making purchase and installation more expensive. POE and POU treatment units would be purchased and owned by the PWS. These solutions are decentralized in nature, and require utility personnel to enter into houses or at least onto private property for installation, maintenance, and testing. Due to the large number of treatment units that would be employed and would be largely out of the control of the PWS, it is very difficult to ensure 100 percent compliance. Prior to selection of a POE or POU program for implementation, consultation with TCEQ would be required to address measurement and determination of the level of compliance.

The SDWA [§1412(b)(4)(E)(ii)] regulates the design, management, and operation of POU and POE treatment units used to achieve compliance with an MCL. These restrictions include:

- POU and POE treatment units must be owned, controlled, and maintained by the water system, although the utility may hire a contractor to ensure proper operation and maintenance (O&M) and compliance with MCLs. The water system must retain unit ownership and oversight of unit installation, maintenance and sampling; the utility ultimately is the responsible party when it comes to regulatory compliance. The water system staff need not perform all installation, maintenance, or management functions, as these tasks may be contracted to a third party, but the final responsibility for quality and quantity of the water supplied to the community resides with the water system, and the utility must monitor all contractors closely. Responsibility for the O&M of POU or POE devices installed for SDWA compliance may not be delegated to homeowners.
- POU and POE units must have mechanical warning systems to automatically notify customers of operational problems. Each POU or POE treatment device must be equipped with a warning device (*e.g.*, alarm, light) that will alert users when their unit is no longer adequately treating their water. As an alternative,

1 2

units may be equipped with an automatic shut-off mechanism to meet this requirement.

• If the American National Standards Institute has issued product standards for a specific type of POU or POE treatment unit, only those units that have been independently certified according to those standards may be used as part of a compliance strategy.

With regard to using POE and POU devices for SDWA compliance, the following observations were made (Raucher, *et al.* 2004):

- If POU devices are used as an SDWA compliance strategy, certain consumer behavioral changes will be necessary (*e.g.*, encouraging people to drink water only from certain treated taps) to ensure comprehensive consumer health protection.
- Although not explicitly prohibited in SDWA, USEPA indicates that POU treatment devices should not be used to treat for radon or for most volatile organic contaminants to achieve compliance, because POU devices do not provide 100 percent protection against inhalation or contact exposure to those contaminants at untreated taps (*e.g.*, shower heads).
- Liability PWSs considering unconventional treatment options (POU, POE, or bottled water) must address liability issues. These could be meeting the drinking water standards, property entry and ensuing liabilities, and damage arising from improper installation or improper function of the POU and POE devices.

1.4.7 Water Delivery or Central Drinking Water Dispensers

Current USEPA regulations 40 Code of Federal Regulations (CFR) 141.101 prohibit the use of bottled water to achieve compliance with an MCL, except on a temporary basis. State regulations do not directly address the use of bottled water. Use of bottled water at a non-compliant PWS would be on a temporary basis. Every 3 years, the PWSs that employ interim measures are required to present the TCEQ with estimates of costs for piping compliant water to their systems. As long as the projected costs remain prohibitively high, the bottled water interim measure is extended. Until USEPA amends the noted regulation, the TCEQ is unable to accept water delivery or central drinking water dispensers as compliance solutions.

Central provision of compliant drinking water would consist of having one or more dispensers of compliant water where customers could come to fill containers with drinking water. The centralized water source could be from small to medium sized treatment units or could be compliant water delivered to the central point by truck.

Water delivery is an interim measure for providing compliant water. As an interim measure for a small impacted population, providing delivered drinking water may be cost

effective. If the susceptible population is large, the cost of water delivery would increase significantly.

Water delivery programs require consumer participation to a varying degree. Ideally, consumers would have to do no more than they currently do for a piped-water delivery system. Least desirable are those systems that require maximum effort on the part of the customer (*e.g.*, customer has to travel to get the water, transport the water, and physically handle the bottles). Such a system may appear to be lowest-cost to the utility; however, should a consumer experience ill effects from contaminated water and take legal action, the ultimate cost could increase significantly.

The ideal system would:

- Completely identify the susceptible population. If bottled water is only provided to customers who are part of the susceptible population, the utility should have an active means of identifying the susceptible population. Problems with illiteracy, language fluency, fear of legal authority, desire for privacy, and apathy may be reasons that some members of the susceptible population do not become known to the utility, and do not take part in the water delivery program.
- Maintain customer privacy by eliminating the need for utility personnel to enter the home.
- Have buffer capacity (e.g., two bottles in service, so that when one is empty, the other is being used over a time period sufficient to allow the utility to change out the empty bottle).
- Provide for regularly scheduled delivery so the customer would not have to notify the utility when the supply is low.
- Use utility personnel and equipment to handle water containers, without requiring customers to lift or handle bottles with water in them.
- Be sanitary (*e.g.*, where an outside connection is made, contaminants from the environment must be eliminated).
- Be vandal-resistant.
- Avoid heating the water due to exterior temperatures and solar radiation.
- Avoid freezing the water.

SECTION 2 EVALUATION METHODOLOGY

2.1 DECISION TREE

The decision tree is a flow chart for conducting feasibility studies for a non-compliant PWS. The decision tree is shown in Figures 2.1 through 2.4. The tree guides the user through a series of phases in the design process. Figure 2.1 shows Tree 1, which outlines the process for defining the existing system parameters, followed by optimizing the existing treatment system operation. If optimizing the existing system does not correct the deficiency, the tree leads to six alternative preliminary branches for investigation. The groundwater branch leads through investigating existing wells to developing a new well field. The treatment alternatives address centralized and on-site treatment. The objective of this phase is to develop conceptual designs and cost estimates for the six types of alternatives. The work done for this report follows through Tree 1 and Tree 2, as well as a preliminary pass through Tree 4.

Tree 3, which begins at the conclusion of the work for this report, starts with a comparison of the conceptual designs, selecting the two or three alternatives that appear to be most promising, and eliminating those alternatives which are obviously infeasible. It is envisaged that a process similar to this would be used by the study PWS to refine the list of viable alternatives. The selected alternatives are then subjected to intensive investigation, and highlighted by an investigation into the socio-political aspects of implementation. Designs are further refined and compared, resulting in the selection of a preferred alternative. The steps for assessing the financial and economic aspects of the alternatives (one of the steps in Tree 3) are given in Tree 4 in Figure 2.4.

2.2 DATA SOURCES AND DATA COLLECTION

2.2.1 Data Search

2.2.1.1 Water Supply Systems

- The TCEQ maintains a set of files on public water systems, utilities, and districts at its headquarters in Austin, Texas. The files are organized under two identifiers: a PWS identification number and a Certificate of Convenience and Necessity (CNN) number. The PWS identification number is used to retrieve four types of files:
- CO Correspondence,
- CA Chemical analysis,
- MOR Monthly operating reports (quality/quantity), and
- FMT Financial, managerial and technical issues.

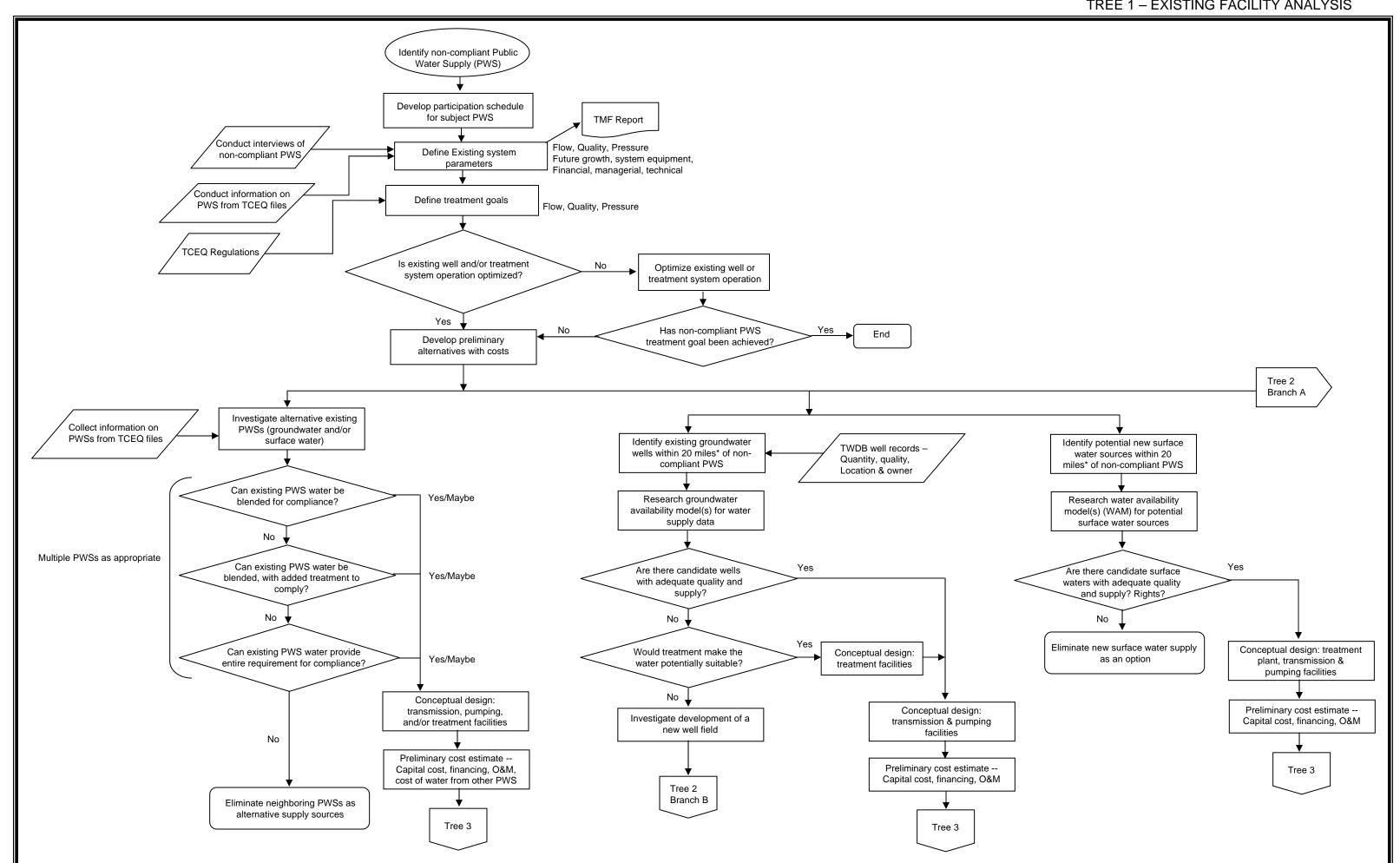
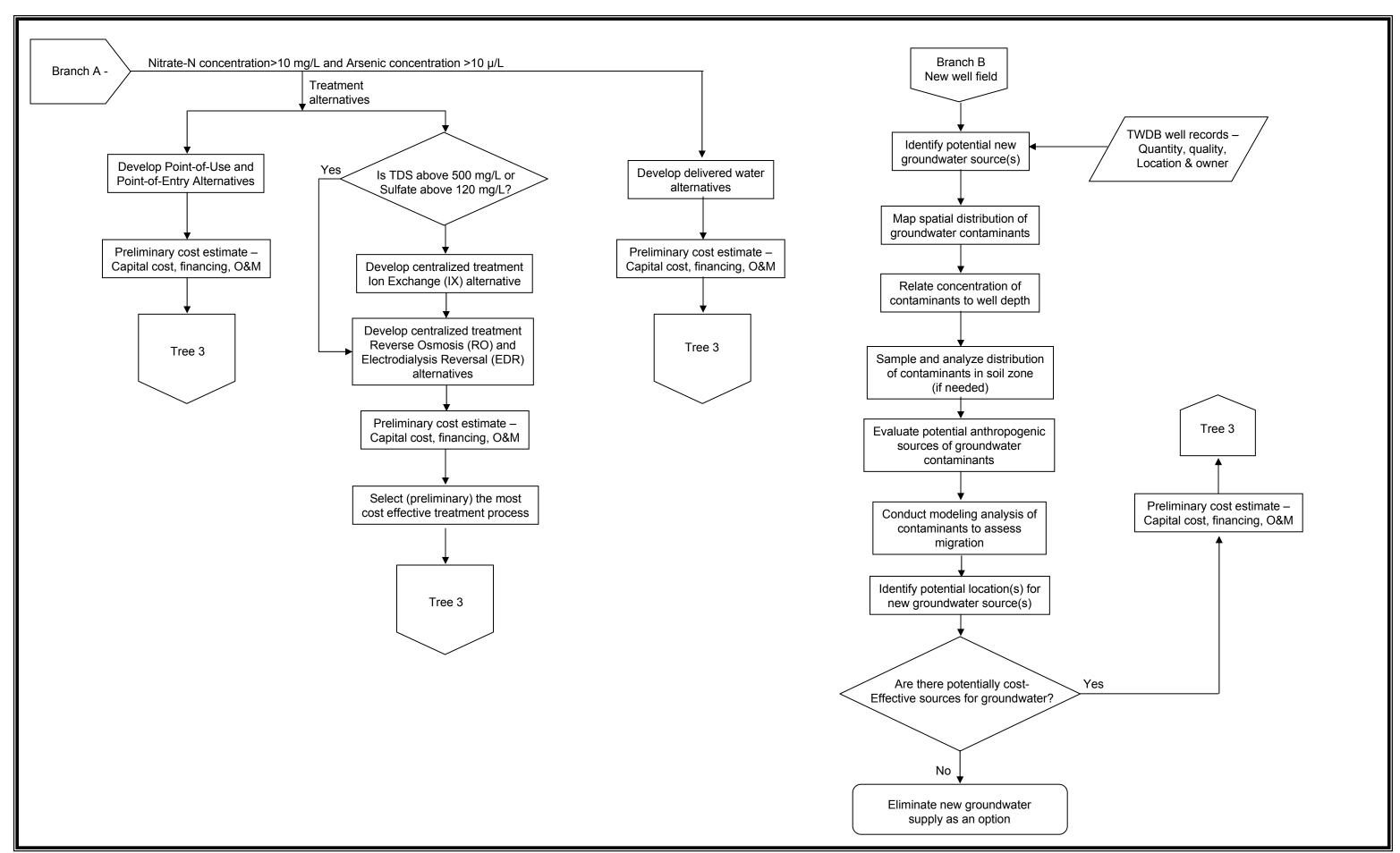


Figure 2.2 TREE 2 – DEVELOP TREATMENT ALTERNATAIVES



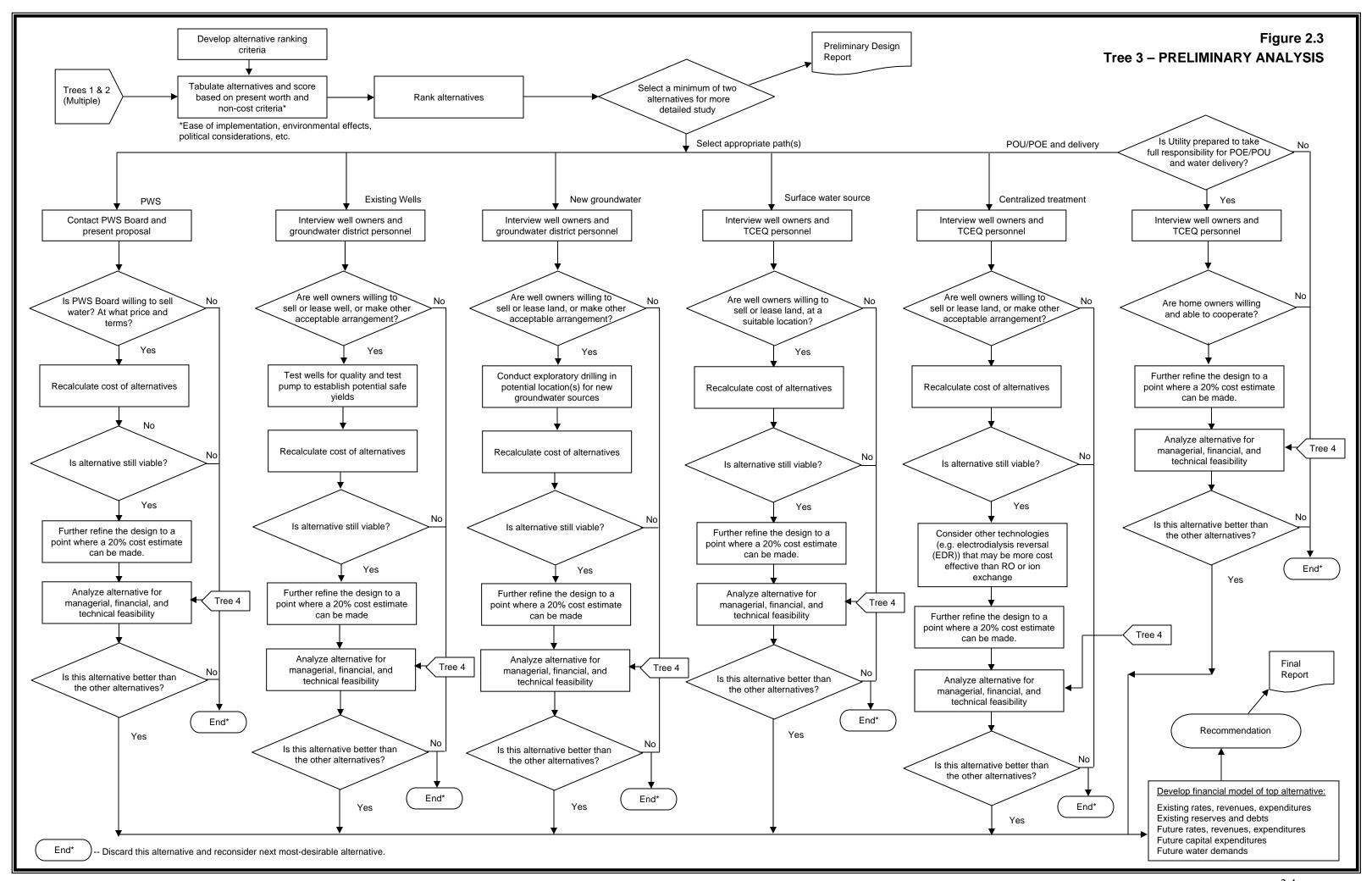
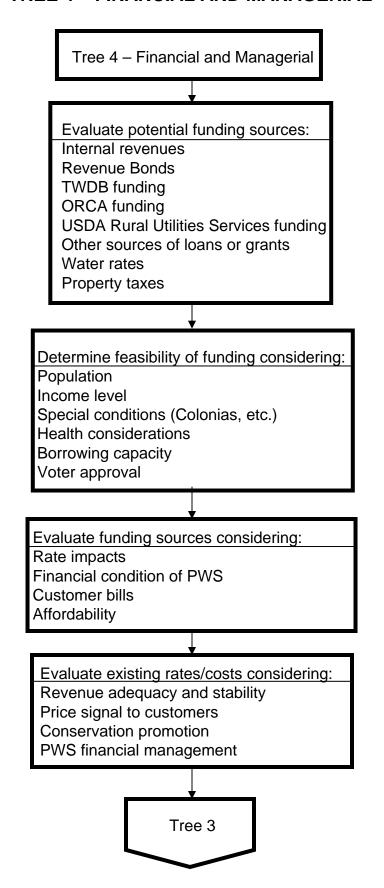


Figure 2.4
TREE 4 – FINANCIAL AND MANAGERIAL



- The CCN files generally contain a copy of the system's Certificate of Convenience and Necessity, along with maps and other technical data.
- These files were reviewed for the PWS and surrounding systems.
- The following websites were consulted to identify the water supply systems in the study area:
 - Texas Commission on Environmental Quality
 <u>www.tnrcc.state.tx.us/iwud/pws/index.cfm</u>. Under "Advanced Search",
 type in the name(s) of the County(ies) in the study area to get a listing of
 the public water supply systems.
 - USEPA Safe Drinking Water Information System www.epa.gov/safewater/data/getdata.html.

Groundwater Control Districts were identified on the TWDB web site, which has a series of maps covering various groundwater and surface water subjects. One of those maps shows groundwater control districts in the State of Texas.

2.2.1.2 Existing Wells

6

7

8

9

10

11

12

13

14

15

16

17 18

19

20

21

22

23

24

25

27

32

33

34

35

The TWDB maintains a groundwater database available at www.twdb.state.tx.us that has two tables with helpful information. The "Well Data Table" provides a physical description of the well, owner, location in terms of latitude and longitude, current use, and for some wells, items such as flowrate, and nature of the surrounding formation. The "Water Quality Table" provides information on the aquifer and the various chemical concentrations in the water. For this study, it was assumed that the nitrate concentration given in this database was the concentration of nitrate, with a molecular weight of 62. To convert to the same basis used for the MCL (Nitrate-N), the value given in the TWDB database was divided by 4.5.

2.2.1.3 Surface Water Sources

Regional planning documents were consulted for lists of surface water sources.

2.2.1.4 Groundwater Availability Model

GAMs, developed by the TWDB, are planning tools and should be consulted as part of a search for new or supplementary water sources. The GAMs for the Ogallala and Edwards-Trinity Plateau aquifers were investigated as a potential tool for identifying available and suitable groundwater resources.

2.2.1.5 Water Availability Model

The WAM is a computer-based simulation predicting the amount of water that would be in a river or stream under a specified set of conditions. WAMs are used to determine whether water would be available for a newly requested water right or amendment. If water is available, these models estimate how often the applicant could count on water under various conditions (*e.g.*, whether water would be available only 1 month out of the year, half the year, or all year, and whether that water would be available in a repeat of the drought of record).

WAMs provide information that assist TCEQ staff in determining whether to recommend the granting or denial of an application.

2.2.1.6 Financial Data

7

20

21

22

2324

25

26

27

28

29 30

31

32

33

34

- 8 Financial data were collected through a site visit. Data sought included:
- Annual Budget
- Audited Financial Statements
- o Balance Sheet
- o Income & Expense Statement
- o Cash Flow Statement
- o Debt Schedule
- Water Rate Structure
- Water Use Data
- o Production
- 18 o Billing
- o Customer Counts

2.2.1.7 Demographic Data

Basic demographic data were collected from the 2000 Census to establish incomes and eligibility for potential low cost funding for capital improvements. Median household income (MHI) and number of families below poverty level were the primary data points of significance. If available, MHI for the customers of the PWS should be used. In addition, unemployment data were collected from current U.S. Bureau of Labor Statistics. These data were collected for the following levels: national, state, and county.

2.2.2 PWS Interviews

2.2.2.1 PWS Capacity Assessment Process

A capacity assessment is the industry standard term for an evaluation of a water system's financial, managerial, and technical capacity to effectively deliver safe drinking water to its customers now and in the future at a reasonable cost, and to achieve, maintain and plan for compliance with applicable regulations. The assessment process involves interviews with staff and management who have a responsibility in the operations and the management of the system.

Financial, managerial, and technical capacity are individual yet highly interrelated components of a system's capacity. A system cannot sustain capacity without maintaining adequate capability in all three components.

Financial capacity is a water system's ability to acquire and manage sufficient financial resources to allow the system to achieve and maintain compliance with the Safe Drinking Water Act (SDWA) requirements. Financial capacity refers to the financial resources of the water system, including but not limited to revenue sufficiency, credit worthiness, and fiscal controls.

Managerial capacity is the ability of a water system to conduct its affairs so that the system is able to achieve and maintain compliance with SDWA regulations. Managerial capacity refers to the management structure of the water system, including but not limited to ownership accountability, staffing and organization, and effective relationships to customers and regulatory agencies.

Technical capacity is the physical and operational ability of a water system to achieve and maintain compliance with SDWA regulations. It refers to the physical infrastructure of the water system, including the adequacy of the source water, treatment, storage and distribution infrastructure. It also refers to the ability of system personnel to effectively operate and maintain the system and to otherwise implement essential technical knowledge.

Many aspects of water system operations involve more than one component of capacity. Infrastructure replacement or improvement, for example, requires financial resources, management planning and oversight, and technical knowledge. A deficiency in any one area could disrupt the entire effort. A system that is able to meet both its immediate and long-term challenges demonstrates that it has sufficient financial, managerial, and technical capacity.

Assessment of the FMT capacity of the PWS was based on an approach developed by the New Mexico Environmental Finance Center (NMEFC), which is consistent with the TCEQ FMT assessment process. This methodology was developed from work the NMEFC did while assisting USEPA Region 6 in developing and piloting groundwater comprehensive performance evaluations. The NMEFC developed a standard list of questions that could be asked of water system personnel. The list was then tailored slightly to have two sets of questions – one for managerial and financial personnel, and one for operations personnel (the questions are included in Appendix A). Each person with a role in the FMT capacity of the system was asked the applicable standard set of questions individually. The interviewees were not given the questions in advance and were not told the answers others provided. Also, most of the questions are open ended type questions so they were not asked in a fashion to indicate what would be the "right" or "wrong" answer. The interviews lasted between 45 minutes to 75 minutes depending on the individual's role in the system and the length of the individual's answers.

In addition to the interview process, visual observations of the physical components of the system were made. A technical information form was created to capture this

information. This form is also contained in Appendix A. This information was considered supplemental to the interviews because it served as a check on information provided in the interviews. For example, if an interviewee stated he or she had an excellent preventative maintenance schedule and the visit to the facility indicated a significant amount of deterioration (more than would be expected for the age of the facility) then the preventative maintenance program could be further investigated or the assessor could decide that the preventative maintenance program was inadequate.

Following interviews and observations of the facility, answers that all personnel provided were compared and contrasted to provide a clearer picture of the true operations at the water system. The intent was to go beyond simply asking the question, "Do you have a budget?" to actually finding out if the budget was developed and being used appropriately. For example, if a water system manager was asked the question, "Do you have a budget?" he or she may say, "yes" and the capacity assessor would be left with the impression that the system is doing well in this area. However, if several different people are asked about the budget in more detail, the assessor may find that although a budget is present, operations personnel do not have input into the budget, the budget is not used by the financial personnel, the budget is not updated regularly, or the budget is not used in setting or evaluating rates. With this approach, the inadequacy of the budget would be discovered and the capacity deficiency in this area would be noted.

Following the comparison of answers, the next step was to determine which items noted as a potential deficiency truly had a negative effect on the system's operations. If a system had what appeared to be a deficiency, but this deficiency was not creating a problem in terms of the operations or management of the system, it was not considered critical and may not have needed to be addressed as a high priority. As an example, the assessment may have revealed an insufficient number of staff members to operate the facility. However, it may also have been revealed that the system was able to work around that problem by receiving assistance from a neighboring system, so no severe problems resulted from the number of staff members. Although staffing may not be ideal, the system does not need to focus on this particular issue. The system needs to focus on items that are truly affecting operations. As an example of this type of deficiency, a system may lack a reserve account which can then lead the system to delay much-needed maintenance or repair on its storage tank. In this case, the system needs to address the reserve account issue so that proper maintenance can be completed.

The intent was to develop a list of capacity deficiencies with the greatest impact on the system's overall capacity. Those were the most critical items to address through follow-up technical assistance or by the system itself.

2.2.2.2 Interview Process

PWS personnel were interviewed by the project team, and each was interviewed separately. Interview forms were completed during each interview.

2.3 ALTERNATIVE DEVELOPMENT AND ANALYSIS

The initial objective for developing alternatives to address compliance issues is to identify a comprehensive range of possible options that can be evaluated to determine which are the most promising for implementation. Once the possible alternatives are identified, they must be defined in sufficient detail so a conceptual cost estimate (capital and O&M costs) can be developed. These conceptual cost estimates are used to compare the affordability of compliance alternatives, and to give a preliminary indication of rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. The basis for the unit costs used for the compliance alternative cost estimates is summarized in Appendix B. Other non-economic factors for the alternatives, such as reliability and ease of implementation, are also addressed.

2.3.1 Existing PWS

The neighboring PWSs were identified, and the extents of their systems were investigated. PWSs farther than 20 miles from the non-compliant PWS were generally not considered because the length of pipelines required would make the alternative cost prohibitive. The quality of water provided was also investigated. For neighboring PWSs with compliant water, options for water purchase and/or expansion of existing well fields were considered. The neighboring PWSs with non-compliant water were considered as possible partners in sharing the cost for obtaining compliant water either through treatment or developing an alternate source.

The neighboring PWSs were investigated to get an idea of the water sources in use and the quantity of water that might be available for sale. They were contacted to identify key locations in their systems where a connection might be made to obtain water, and to explore on a preliminary basis their willingness to partner or sell water. Then, the major system components that would be required to provide compliant water were identified. The major system components included treatment units, wells, storage tanks, pump stations, and pipelines.

Once the major components were identified, a preliminary design was developed to identify sizing requirements and routings. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.2 New Groundwater Source

It was not possible in the scope of this study to determine conclusively whether new wells could be installed to provide compliant drinking water. In order to evaluate potential new groundwater source alternatives, three test cases were developed based on distance from the PWS intake point. The test cases were based on distances of 10 miles, 5 miles, and 1 mile. It was assumed that a pipeline would be required for all three test cases, and a storage tank and pump station would be required for the 10-mile and 5-mile alternatives. It was also assumed that new wells would be installed, and that their depths would be similar to the depths of the existing wells, or other existing drinking water wells in the area.

A preliminary design was developed to identify sizing requirements for the required system components. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change (*i.e.*, from current expenditures) in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.3 New Surface Water Source

New surface water sources were investigated. Availability of adequate quality water was investigated for the main rivers in the study area, as well as the major reservoirs. TCEQ WAMs were inspected, and the WAM was run, where appropriate.

2.3.4 Treatment

Treatment technologies considered potentially applicable to both nitrate and arsenic removal are RO, IX, and EDR since they are proven technologies with numerous successful installations. However, all systems with elevated nitrate and arsenic also have TDS levels higher than 1,000 mg/L and thus, IX is not economically feasible. RO treatment is considered for central treatment alternatives, as well as POU and POE alternatives. EDR treatment is considered for central treatment alternatives only. Both RO and EDR treatment produce a liquid waste: a reject stream from RO treatment and a concentrate stream from EDR treatment. As a result, the treated volume of water is less than the volume of raw water that enters the treatment system. The amount of raw water used increases to produce the same amount of treated water if RO or EDR treatment is implemented. The treatment units were sized based on flow rates, and capital and annual O&M cost estimates were made based on the size of the treatment equipment required. Neighboring non-compliant PWSs were identified to look for opportunities where the costs and benefits of central treatment could be shared between systems.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

6 2.4 COST OF SERVICE AND FUNDING ANALYSIS

The primary purpose of the cost of service and funding analysis was to determine the financial impact of implementing compliance alternatives, primarily by examining the required rate increases, and analyzing the fraction of household income that water bills consume. The current financial situation was also reviewed to determine what rate increases were necessary for the PWS to achieve or maintain financial viability.

2.4.1 Financial Feasibility

1 2

A key financial metric is comparison of the average annual household water bill for a PWS customer to the MHI for the area. MHI data from the 2000 Census were used at the most detailed level available for the community. Typically, county level data are used for small rural water utilities due to small population sizes. Annual water bills were determined for existing base conditions and included consideration of additional rate increases needed under current conditions. Annual water bills were also calculated after adding incremental capital and operating costs for each of the alternatives to determine feasibility under several potential funding sources.

Additionally, the use of standard ratios provided insight into the financial condition of any business. Three ratios are particularly significant for water utilities:

- Current Ratio = current assets divided by current liabilities provides insight into the ability to meet short-term payments. For a healthy utility, the value should be greater than 1.0.
- Debt to Net Worth Ratio = total debt divided by net worth shows to what degree assets of the company have been funded through borrowing. A lower ratio indicates a healthier condition.
- Operating Ratio = total operating revenues divided by total operating expenses show the degree to which revenues cover ongoing expenses. The value is greater than 1.0 if the utility is covering its expenses.

2.4.2 Median Household Income

The 2000 Census was used as the basis for MHI. In addition to consideration of affordability, MHI may also be an important factor for sources of funds for capital programs needed to resolve water quality issues. Many grant and loan programs are available to lower income rural areas, based on comparisons of local income to statewide incomes. In the 2000 Census, MHI for the State of Texas was \$39,927, compared to the

- 1 U.S. level of \$41,994. For service areas with a sparse population base, county data may
- 2 be the most reliable and, for many rural areas, correspond to census tract data.

3 2.4.3 Annual Average Water Bill

The annual average household water bill was calculated for existing conditions and for future conditions incorporating the alternative solutions. Average residential consumption was estimated and applied to the existing rate structure to estimate the annual water bill. The estimates were generated from a long-term financial planning model that detailed annual revenue, expenditure and cash reserve requirements over a 30-year period.

2.4.4 Financial Plan Development

The financial planning model used available data to establish base conditions under which the system operates. The model included, as available:

- Accounts and consumption data
- Water tariff structure
- Beginning available cash balance
- Sources of receipts:

- o Customer billings
- o Membership fees
- o Capital Funding receipts from:
- 20 ❖ Grants
- 21 Proceeds from borrowing
- Operating expenditures:
- O Water purchases
- o Utilities
- 25 o Administrative costs
- o Salaries
- Capital expenditures
- Debt service:
- o Existing principal and interest payments
- o Future principal and interest necessary to fund viable operations
- Net cash flow
- Restricted or desired cash balances:

Small Public Water Systems – South Midland County Evaluation Methodology 1 o Working capital reserve (based on 1-4 months of operating expenses) 2 o Replacement reserves to provide funding for planned and unplanned repairs and replacements 3 4 From the model, changes in water rates were determined for existing conditions and for implementing the compliance alternatives. 5 2.4.5 **Financial Plan Results** 6 7 Results from the financial planning model were summarized in two ways: 8 percentage of household income and by total water rate increase necessary to implement 9 the alternatives and maintain financial viability. 2.4.5.1 Funding Options 10 11 Results, summarized in Table 4.4, show the following according to alternative and 12 funding source: • Percentage of the median annual household income that the average 13 14 annual residential water bill represents. The first year in which a water rate increase will be required. 15 16 The total increase in water rates required, compared to current rates. 17 Water rates resulting from the incremental capital costs of the alternative solutions were examined under a number of funding options. The first alternative examined was 18 always funded from existing reserves plus future rate increases. Several funding options 19 20 were analyzed to frame a range of possible outcomes. 21 Grant funds for 100 percent of required capital. In this case, the PWS was 22 only responsible for the associated O&M costs. 23 Grant funds for 75 percent of required capital, with the balance treated as if revenue bond funded. 24 25 • Grant funds for 50 percent of required capital, with the balance treated as 26 if revenue bond funded. 27 State revolving fund loan at the most favorable available rates and terms 28 applicable to the communities. 29 • If local MHI > 75 percent of state MHI, standard terms, currently at 30 3.8 percent interest for non-rated entities. Additionally:

o If local MHI = 70-75 percent of state MHI, 1 percent interest rate on

o If local MHI = 60-70 percent of state MHI, 0 percent interest rate on

loan.

loan.

31

32

33

- 1 o If local MHI = 50-60 percent of state MHI, 0 percent interest and 15 percent forgiveness of principal.
 - o If local MHI less than 50 percent of state MHI, 0 percent interest and 35 percent forgiveness of principal.
 - Terms of revenue bonds assumed to be 25-year term at 6.0 percent interest rate.

2.4.5.2 General Assumptions Embodied in Financial Plan Results

The basis used to project future financial performance for the financial plan model included:

- No account growth (either positive or negative).
 - No change in estimate of uncollectible revenues over time.
 - Average consumption per account unchanged over time.
 - No change in unaccounted for water as percentage of total (more efficient water use would lower total water requirements and costs).
 - No inflation included in the analyses (although the model has provisions to add escalation of O&M costs, doing so would mix water rate impacts from inflation with the impacts from the alternatives being examined).
 - Minimum working capital fund established for each district, based on specified months of O&M expenditures.
 - O&M for alternatives begins 1 year after capital implementation.
 - Balance of capital expenditures not funded from primary grant program is funded through debt (bond equivalent).
 - Cash balance drives rate increases, unless provision chosen to override where current net cash flow is positive.

2.4.5.3 Interpretation of Financial Plan Results

Results from the financial plan model, as presented in Table 4.4, show the percentage of MHI represented by the annual water bill that resulted from any rate increases necessary to maintain financial viability over time. In some cases, this may require rate increases even without implementing a compliance alternative (the no action alternative). The table shows any increases such as these separately. The results table shows the total increase in rates necessary, including both the no-action alternative increase and any increase required for the alternative. For example, if the no action alternative required a 10 percent increase in rates and the results table shows a rate increase of 25 percent, then the impact from the alternative was an increase in water rates of 15 percent. Likewise, the percentage of household income in the table reflects the total impact from all rate increases.

2.4.5.4 Potential Funding Sources

1

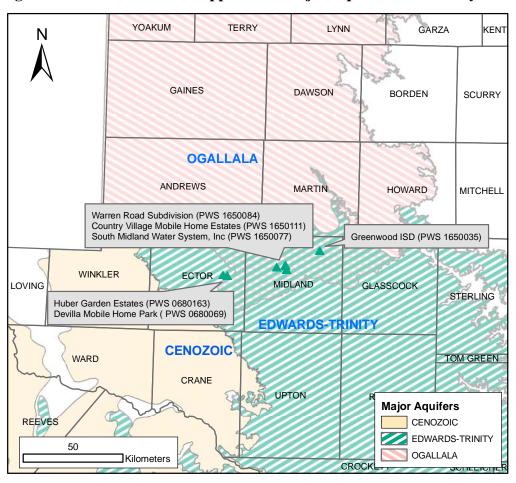
- A number of potential funding sources exists for rural utilities. Both state and federal agencies offer grant and loan programs to assist rural communities in meeting their infrastructure needs.
- 5 Within Texas, the following state agencies offer financial assistance if needed:
- Texas Water Development Board,
- Office of Rural Community Affairs, and
- Texas Department of Health (Texas Small Towns Environment Program).
- 9 Small rural communities can also get assistance from the federal government. The primary agencies providing aid are:
 - United States Department of Agriculture, Rural Utilities Service, and
- United States Housing and Urban Development.

SECTION 3 UNDERSTANDING SOURCES OF CONTAMINANTS

3.1 NITRATE AND ARSENIC IN THE SOUTHERN HIGH PLAINS AND EDWARDS TRINITY (PLATEAU) AQUIFERS

The major aquifers in the vicinity of the evaluated public water systems include the Ogallala aquifer (Miocene–Pliocene age), the Edwards Trinity (Plateau) aquifer (Cretaceous age), and the Cenozoic Pecos Alluvium (CPA) aquifer (Tertiary and Quaternary age) (Ashworth and Hopkins 1995). Figure 3.1 shows assessed public water supplies and major aquifers in the study area.

Figure 3.1 Public Water Supplies and Major Aquifers in the Study Area



The Ogallala Formation consists of coarse sandstone and conglomerates of late Tertiary (Miocene-Pliocene) age (Nativ 1988). The sediments consist of coarse fluvial clastics that were deposited in paleovalleys in a mid-Tertiary erosional surface with eolian sands in intervening upland areas. The Ogallala Formation is ~ 30m thick in the south (Ector-Midland Counties). The top of the Ogallala Formation is marked by a resistant calcite layer termed the "caprock" caliche.

17

11

12

13

1

2

3

4

5

6

7

8 9

2

3

4 5

6 7

8

9

10

11 12

13

14

15

16

17

18

19 20

21

22

23

24

25

26

27

28

29

30

31

The Edwards Trinity (Plateau) aquifer underlies the Ogallala aquifer in Andrews, Martin, Ector, Midland, and Glasscock Counties and crops out south of this region. This aquifer consists predominantly of the Trinity Group (Early Cretaceous age) and includes the Antlers Sandstone in Ector and Midland Counties, which is overlain by the Washita and Fredericksburg Divisions in Glasscock County (Barker and Ardis 1996). Antlers Sandstone consists of basal gravels overlain by fluvial-deltaic sands deposited on a pre-Cretaceous unconformity developed on Paleozoic and earlier Mesozoic rocks. The basal gravels are thicker in paleovalleys. The overlying Washita and Fredericksburg Divisions are carbonate dominated with interbedded sandstones. The Lower Cretaceous formations were karstified before deposition of the Upper Cretaceous formations. These units are divided into several formations with complicated terminology: Formation, Comanche Peak Limestone, and Edwards Limestone transitioning laterally in name to Fort Terrett Formation (base) and Fort Lancaster Formation in some places, and Segovia Formation in other places. The most prolific producing unit is the Fort Terrett When overlain by the Ogallala Formation, both formations are Formation. hydrologically connected and form the High Plains aquifer. However, in some areas only the Cretaceous unit is saturated, and the Ogallala sediments are in the unsaturated zone.

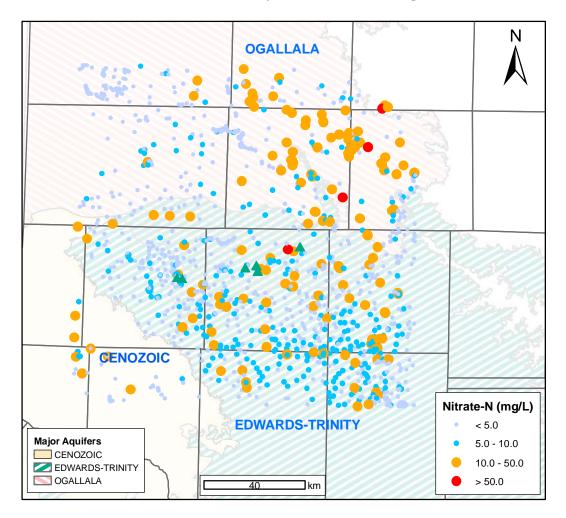
The CPA aquifer consists of up to 1,500 feet of alluvial fill and occupies two separate basins: the Pecos Trough to the west, and the Monument Draw Trough in the east (E. Ector, Winkler, Ward, Crane, and Pecos Counties). These troughs formed as a result of dissolution of underlying evaporites (rock salt, anhydrite, gypsum) in the Permian units. Groundwater occurs under unconfined (water table) or semiconfined conditions. The alluvium consists of unconsolidated or poorly cemented clay, sand, gravel, and caliche (White 1971). North of the Pecos River the alluvium is overlain by windblown sand deposited in dunes. The sand dunes are up to 250 feet thick.

3.2 GENERAL TRENDS IN NITRATE CONCENTRATIONS

The geochemistry of nitrate is described in Appendix E. Nitrate trends in the vicinity of the assessed PWSs were examined to assess spatial trends, as well as correlations with other water quality parameters. Nitrate measurements are from the TWDB database. Figure 3.2 shows spatial distribution of nitrate concentrations from the TWDB database.

1 2

Figure 3.2 Detectable Nitrate-N Concentrations in Groundwater (TWDB Database, Analyses from 1937 through 2004)



4

5

6 7

8

9

10

From the TWDB database, 1,410 measurements were extracted, representing the most recent nitrate measurements taken at a specific well (if more than one sample existed for 1 day the average for the day was calculated). Samples were limited to an area delimited by the following coordinates: bottom left corner -102.84E, 31.46N and upper right corner -101.41E, 32.66N. Coordinates are in decimal degrees, and the datum is North American Datum 1983 (NAD 1983). Figure 3.3 shows wells with nitrate samples categorized by aquifers.

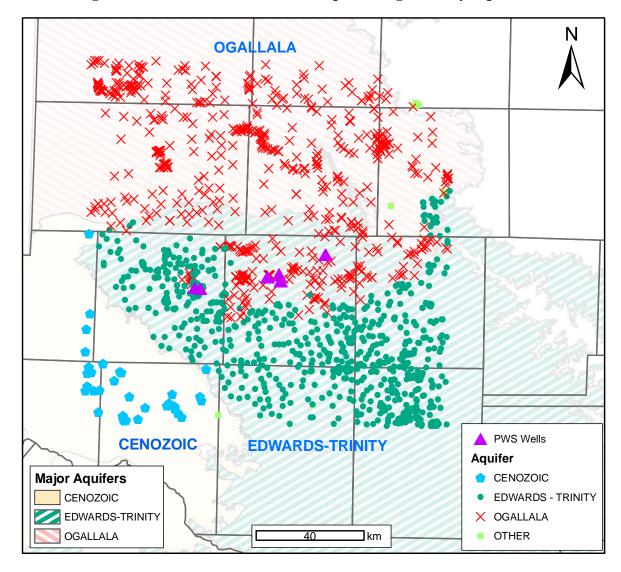
2

3

5 6

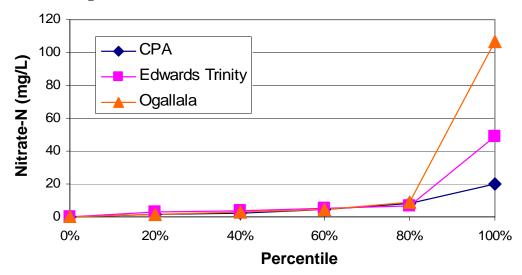
7 8

Figure 3.3 Wells with Nitrate Samples Categorized by Aquifer



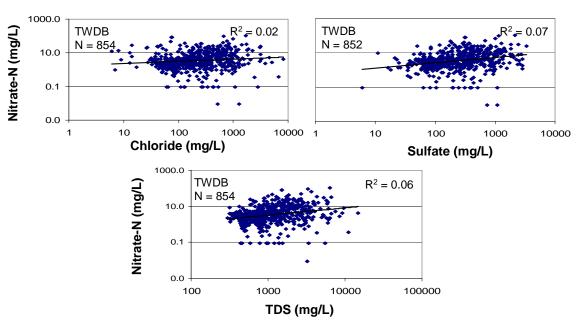
The above map (Figure 3.3) shows 1,410 wells that have nitrate measurements from the TWDB database: 774 are in the Edwards Trinity (Plateau) aquifer, 584 in the Ogallala aquifer, 43 in the CPA aquifer, and 9 in other aquifers. The distribution of nitrate-N concentrations within the three aquifers (CPA, Edwards Trinity (Plateau), and Ogallala) is similar (Figure 3.4). The similarity in nitrate-N levels among the aquifers suggests the source of nitrate is not a particular geologic unit but probably anthropogenic in origin.

Figure 3.4 Distribution of Nitrate-N Concentrations



Nitrate-N is not strongly related to general water quality parameters (sulfate, chloride, and TDS) in the Ogallala aquifer (Figure 3.5). Similar results were found for the Edwards-Trinity (Plateau) aquifer where the coefficient of determination or R-squared (R²) is less than 0.1 (i.e., little to no correlation), strengthening the conclusion that nitrate-N sources are anthropogenic rather than geologic in origin.

Figure 3.5 Correlation of Nitrate with Chloride, Sulfate, and TDS in the Ogallala Aquifer



Note: N represents the number of wells in the analysis. The most recent measurement is shown for each well (when there is more than one sample in 1 day the average concentration is calculated; only seven wells had more than one sample for the most recent day).

10 11

1

2

3

4

5

6 7

8

5

6

7

8

9

10

11 12

13

14

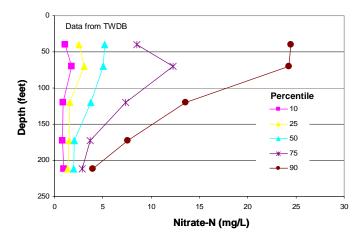
15

16

17

Nitrate-N concentrations are compared with well depth to assess stratification in nitrate concentrations in the Ogallala aquifer (Figure 3.6) and Edwards-Trinity (Plateau) aquifer (Figure 3.7).

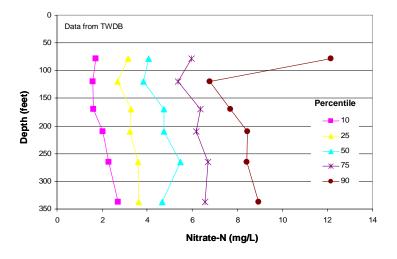
Figure 3.6 Relationship between Nitrate-N Concentrations and Well Depth in the Ogallala Aquifer



Depth interval (feet)	Min. depth (feet)	Max. depth (feet)	Median depth (feet)	Num. of wells
< 50	20	49	40	31
50-100	50	99	70	150
100- 150	100	148	120	158
150- 200	150	197	173	126
> 200	200	306	212	49

For Figure 3.6, wells are divided into depth bins, and for each bin the nitrate-N concentration is shown with respect to the median depth. The table on the right summarizes depth values for each bin and gives the number of wells in the analysis for that depth range. The analysis shows that within the Ogallala aquifer, highest nitrate-N concentrations are found in shallower wells (depth < 100 feet), and nitrate-N concentrations generally decrease with depth, particularly the 75th and 90th percentile values.

Figure 3.7 Relationship Between Nitrate-N Concentrations and Well Depth in the Edwards-Trinity (Plateau) Aquifer



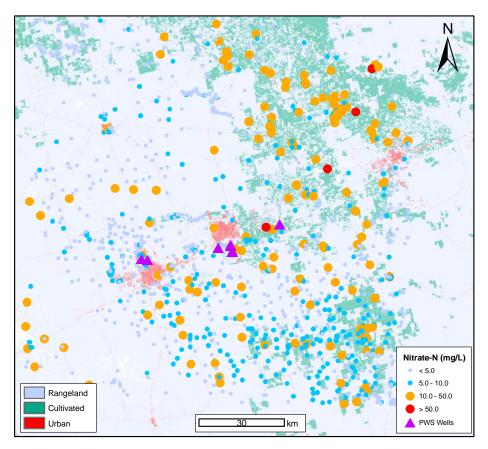
Depth range (feet) < 100	Min. depth (feet) 37	Max. depth (feet) 99	Median depth (feet)	Num. of wells
< 100	31	99	19	7.7
100- 150	100	149	120	170
150- 200	150	197	170	143
200- 250	200	248	211	106
250- 300	250	297	265	72
> 300	300	495	337	116

Figure 3.7 shows the relationship between nitrate-N concentrations and depth within the Edwards-Trinity (Plateau) aquifer. Wells are divided into depth bins, and for each bin, nitrate-N concentrations are shown with respect to median depth. The table on the

right summarizes the depth values for each bin and gives the number of wells in the analysis for that depth range. The analysis shows that within the Edwards-Trinity (Plateau) aquifer, nitrate-N concentrations generally show no systematic variation with depth. In general, concentrations remain constant with depth, although some relationship is seen within the 90^{th} percentile, where the shallower wells (< 100 feet) have higher concentrations.

Nitrate-N concentrations from the TWDB database were compared with land use from the National Land Cover Dataset (NLCD 1992). Land-use datasets are categorized into three groups (rangeland, cultivated, and urban) and compared with nitrate-N concentrations within the study area. Figure 3.8 shows the spatial distribution of nitrate-N and land use; high concentrations of nitrate-N are generally found in cultivated areas. Figure 3.9 shows the correlation between land-use types and nitrate-N concentrations.

Figure 3.8 Spatial Relationship Between Land Cover (NLCD) and Nitrate-N Concentrations



Note: Nitrate concentrations are from the TWDB database, and the most recent nitrate measurement is shown for each well.

1 2

3 4

5

6

7

8

9

10

11

1213

Figure 3.9 Relationship Between Nitrate-N Concentrations and Land Use

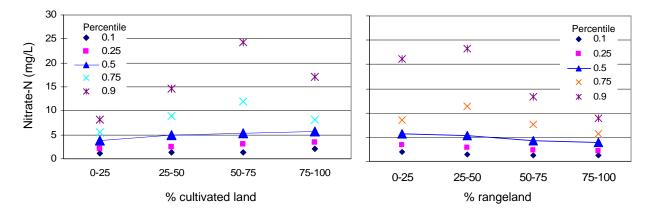


Figure 3.9 shows nitrate-N concentrations in groundwater in relation to land use within a 1-km radius of well locations. Land use was obtained from the NLCD and was categorized into the following land-use types: rangeland (NLCD codes 51, 71, 41, 42, and 43), cultivated (NLCD codes 81, 82, 83, and 61), and urban (NLCD codes 21, 22, 23, and 85). The complementary analysis accounts for more than 90 percent of the land use related to over 95 percent of the wells. Nitrate-N concentrations are from the TWDB database, and the most recent measurement is used for each well. Nitrate-N concentrations generally increase with percentage of cultivated land (left plot) and decrease with percentage of rangeland (right plot). The two plots are generally complementary with increases in nitrate-N with cultivation and decreases in nitrate-N with rangeland. The greatest increases in nitrate-N with cultivation occur in the upper 75th and 90th percentiles. Population means of the land-use groups (percentage bins) are statistically different (P < 1e⁻⁹) for both land-use categories.

3.3 GENERAL TRENDS IN ARSENIC CONCENTRATIONS

The geochemistry of arsenic is described in Appendix E. Arsenic trends in the vicinity of the analyzed PWSs were examined to assess spatial trends, as well as correlations with other water quality parameters. Arsenic measurements were obtained from the TWDB database and from a subset of the National Geochemical Database, also known as the NURE (National Uranium Resource Evaluation) database. Figure 3.10 shows spatial distribution of arsenic concentrations from the TWDB database, and Figure 3.11 shows percentages of wells in each aquifer that exceed the MCL of arsenic of $10\,\mu\text{g/L}$.

Figure 3.10 Spatial Distribution of Arsenic Concentrations (TWDB Database)

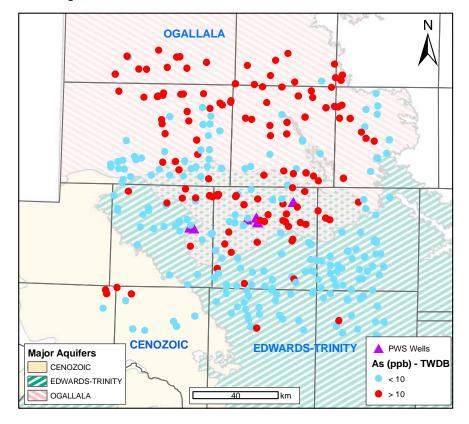
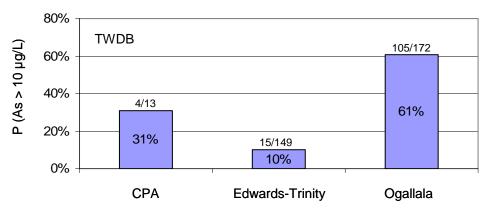


Figure 3.11 Probabilities of Arsenic Concentrations Exceeding 10 μ g/L MCL for Aquifers in the Study Area



Data in Figures 3.10 and 3.11 are from the TWDB database. The most recent arsenic measurement was used for each well. The Ogallala aquifer has a percentage of wells with arsenic concentrations >10 μ g/L which is higher than the other aquifers (Figuew 3.11). Within the Ogallala aquifer, 61 percent of the wells had arsenic concentrations >10 μ g/L, in comparison with the CPA (31%) and Edwards-Trinity (Plateau) (10%) aquifers. A closer review of the spatial distribution of wells in the Edwards-Trinity (Plateau) with high arsenic concentrations reveals that almost all wells with high arsenic concentrations are within the boundary of the Ogallala aquifer (only

5

6

7

8 9

10

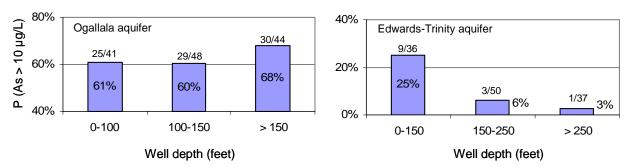
2

4

seven wells with high arsenic are outside the aquifer boundary, and three of those seven are within 5 km of the boundary). It is possible these wells are screened within the Ogallala aquifer or screened across the Edwards-Trinity (Plateau) and Ogallala aquifers together. This assumption cannot be verified because only one well of the seven has a secondary aquifer (Dockum) designated in the TWDB database.

To assess relationships between elevated arsenic concentrations and specific stratigraphic units, arsenic concentrations were compared with well depth for the Ogallala and Edwards-Trinity (Plateau) aquifers separately (Figure 3.12). Within the Ogallala aquifer, arsenic concentrations were not strongly correlated with well depth. Within the Edwards-Trinity (Plateau) aquifers the shallower wells (<150 feet) have higher probabilities of arsenic concentrations exceeding $10\,\mu\text{g/L}$. The shallower wells are closer to the Ogallala Formation (which overlies the Edwards-Trinity Plateau), and these wells may be screened within the Ogallala Formation or across both the Edwards-Trinity (Plateau) and Ogallala Formations. This restriction of high arsenic levels to shallow wells in the Edwards Trinity (Plateau) aquifer strengthens the assumption that the source of contamination for wells within the Edwards-Trinity (Plateau) aquifers is actually from the Ogallala aquifer.

Figure 3.12 Relationship Between Arsenic Concentrations and Well Depth

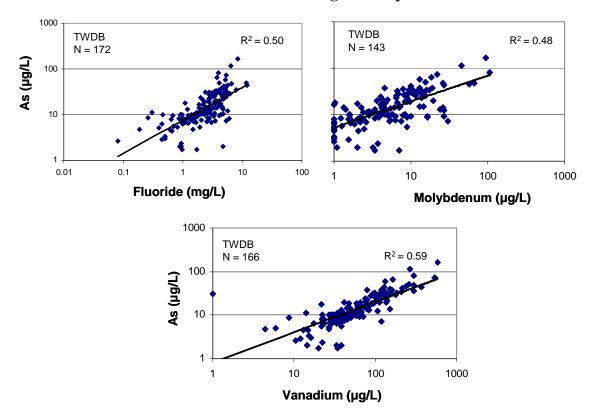


Data are from the TWDB database, and the most recent arsenic measurement was used for analysis for each well. Numbers above each column represent numbers of arsenic measurements that are >10 μ g/L and total number of analyses in the bin. For example, 25/41 represents 24 samples >10 μ g/L out of 41 analyses at a well depth between 0 and 100 feet.

Relationships between arsenic and pH, SO_4 , fluoride, chloride, TDS, vanadium, and molybdenum were evaluated using data from the TWDB database. Data from the NURE database were used to evaluate the relationship between arsenic concentrations and dissolved oxygen concentrations. Strong coefficients of determination or R-squared values ($R^2 > 0.48$) were found between arsenic and fluoride, arsenic and vanadium, and arsenic and molybdenum within the Ogallala aquifer (Figure 3.13). Arsenic and vanadium were also correlated within the Edwards-Trinity (Plateau), but other parameters were not highly correlated with arsenic within the Edwards-Trinity (Plateau) aquifer.

1 2

Figure 3.13 Relationship Between Arsenic and Fluoride, Molybdenum, and Vanadium within the Ogallala Aquifer



Data are from the TWDB database, and the most recent arsenic sample was used in the analysis for each well. Fluoride, molybdenum, and vanadium concentrations were measured the same day as those of the most recent arsenic measurements. A total of nine arsenic measurements within the database were below the detection limit of $10 \,\mu\text{g/L}$, and two samples are below the detection limit of $2 \,\mu\text{g/L}$. These samples are plotted as equal to detection limits (10 and 2, respectively). Vanadium samples have a detection limit of $1 \,\mu\text{g/L}$ and are plotted as equal to the detection limit. Molybdenum concentrations in the TWDB database have detection limits of 50, 20, 4, 2, and $1 \,\mu\text{g/L}$. Values below detection limits of 50 and 20 were excluded from analysis, and remaining values were plotted as equal to detection limits.

Within the NURE database, only 25 wells were sampled in the study area. Dissolved oxygen in the 25 samples ranged between 6.7 and 14.3 mg/L. No aquifer designation is within the NURE database, but 21 of the 25 wells are within the Ogallala aquifer boundary, and the other four are proximal to it (>15 km). Depths for these wells range from 6 to 70 feet, also suggesting they are in the shallow Ogallala aquifer. Dissolved oxygen values show that groundwater is oxidizing and that arsenic should be present as arsenate and may have been mobilized under high pH (see Appendix E).

Generally high correlations between arsenic and fluoride, molybdenum, and vanadium (Figure 3.13) and dissolved oxygen concentrations from the NURE database

suggest natural sources of elevated arsenic within the Ogallala aquifer. Within the Edwards-Trinity (Plateau) aquifer, correlations are not as strong, and it is more likely the source of arsenic is from the Ogallala aquifer overlying the Edwards-Trinity (Plateau)

4 aquifer.

5

6

7

8

9 10

11

12

13

14

15

16

3.4 DETAILED ASSESSMENT FOR SOUTH MIDLAND COUNTY WATER SYSTEM, INC.

Two active wells are in this water supply system: G1650077A, G1650077B. There are also three inactive wells in this water supply system: G1650077C, G1650077D, and G1650077E. The depths of the wells range between 80 and 100 feet, and they are all within the Ogallala aquifer. Well G1650077A is the only one with screen depth information, and it is screened between 60 and 80 feet. All five wells are related to one entry point in the water supply system, making it difficult to trace contaminants to a specific well. Table 3.1 summarizes nitrate-N concentrations measured at the South Midland County Water System PWS.

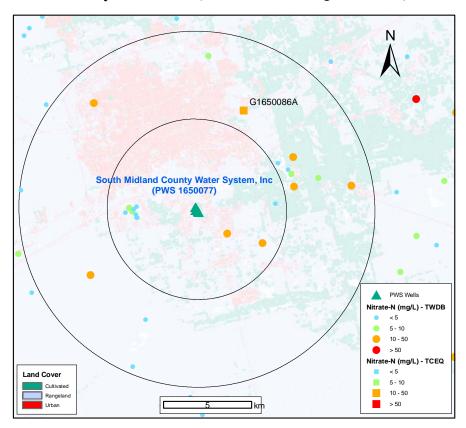
Table 3.1 Nitrate-N Concentrations in the South Midland County PWS (TCEQ Database)

Date	Nitrate-N (mg/L)	Source
3/18/1997	10.24	TCEQ
6/25/1997	12.02	TCEQ
9/18/1997	11.39	TCEQ
12/15/1997	11.09	TCEQ
2/25/1998	11.01	TCEQ
2/25/1998	9.98	TCEQ
4/20/1998	10.14	TCEQ
8/11/1998	10.26	TCEQ
9/30/1999	< 0.01	TCEQ
11/1/1999	10.45	TCEQ
3/9/2000	11	TCEQ
6/22/2000	10.15	TCEQ
7/26/2000	10.86	TCEQ
2/27/2001	10.72	TCEQ
6/25/2001	11.49	TCEQ
6/24/2002	9.82	TCEQ
8/29/2002	11.06	TCEQ
11/25/2002	10.71	TCEQ
9/22/2004	10.3	TCEQ
11/10/2004	10.6	TCEQ

Groundwater nitrate and arsenic concentrations can have a high degree of spatial variability. Because of this variability, an investigation of the existing wells should be conducted to determine whether one, several or all five wells produce non-compliant water. If one well is found to produce compliant water, as much production as possible should be shifted to the compliant well. Also, if one well is found to produce compliant water, the wells should be compared in terms of depths and well logs to try and identify differences that could be responsible for the elevated concentration of nitrates or arsenic in the other well. Then if blending of water from the existing wells does not produce a sufficient quantity of compliant water, it may be possible to install a new well similar to the existing compliant well that also would provide compliant water.

Twenty nitrate samples were collected at the PWS between 1997 and 2004. All the samples except for one, which is below detection limit (0.01 mg/L), are above the nitrate-N MCL (10 mg/L). Figure 3.14 shows the spatial distribution of nitrate-N concentrations within 5- and 10-km buffers of the PWS wells.

Figure 3.14 Nitrate-N Concentrations in 5- and 10-km Buffers of South Midland County PWS Wells (TWDB and TCEQ Databases)



Data are from the TCEQ and TWDB databases. Maximum nitrate-N concentration is shown for each well. Two types of samples were included in the analysis from the TCEQ database: raw samples that can be related to a single well and entry-point samples taken from a single entry point, which can be related to a single well.

3

4

5

6

7

8

9

10

11 12

13

14

15

16

17

Only one well has nitrate-N concentrations from the TCEQ database within the buffers (G1650086A), and it has high nitrate-N concentrations (>10 mg/L). Seven wells from the TWDB database within the buffers of the PWS wells have high (>10 mg/L) nitrate-N concentrations.

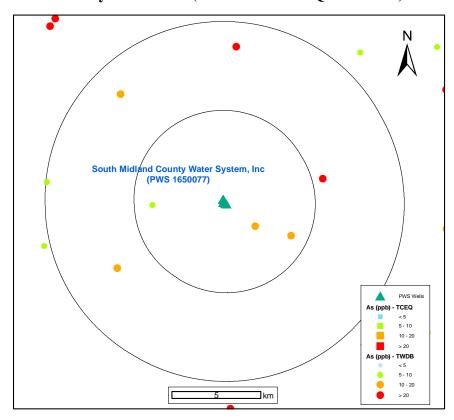
Table 3.2 summarizes arsenic concentrations measured at the South Midland County Water System PWS.

Table 3.2 Arsenic Concentrations in the South Midland County PWS (TCEQ Database)

Date	As (μg/L)	Source
2/25/1998	17	TCEQ
2/27/2001	12.1	TCEQ

Two arsenic measurements for this PWS are in the TCEQ database between 1998 and 2001. The measurements are above the arsenic MCL ($10\,\mu\text{g/L}$). Figure 3.15 shows the spatial distribution of arsenic concentrations within 5- and 10-km buffers of the PWS wells.

Figure 3.15 Arsenic Concentrations in 5- and 10-km Buffers of South Midland County PWS Wells (TWDB and TCEQ Databases)



Data are from the TWDB and TCEQ databases (no wells have arsenic samples from the TCEQ database within the buffers). Maximum arsenic concentration is shown for

- 1 each well. Six of the TWDB wells within the buffers have arsenic exceeding the arsenic
- 2 MCL.

SECTION 4 2 ANALYSIS OF THE SOUTH MIDLAND COUNTY WATER SYSTEM

4.1 DESCRIPTION OF EXISTING SYSTEM

4 4.1.1 Existing System

3

5

6

7

8

9

10

11

12

13 14

15

16

17

18

19

20

21

22

23

24

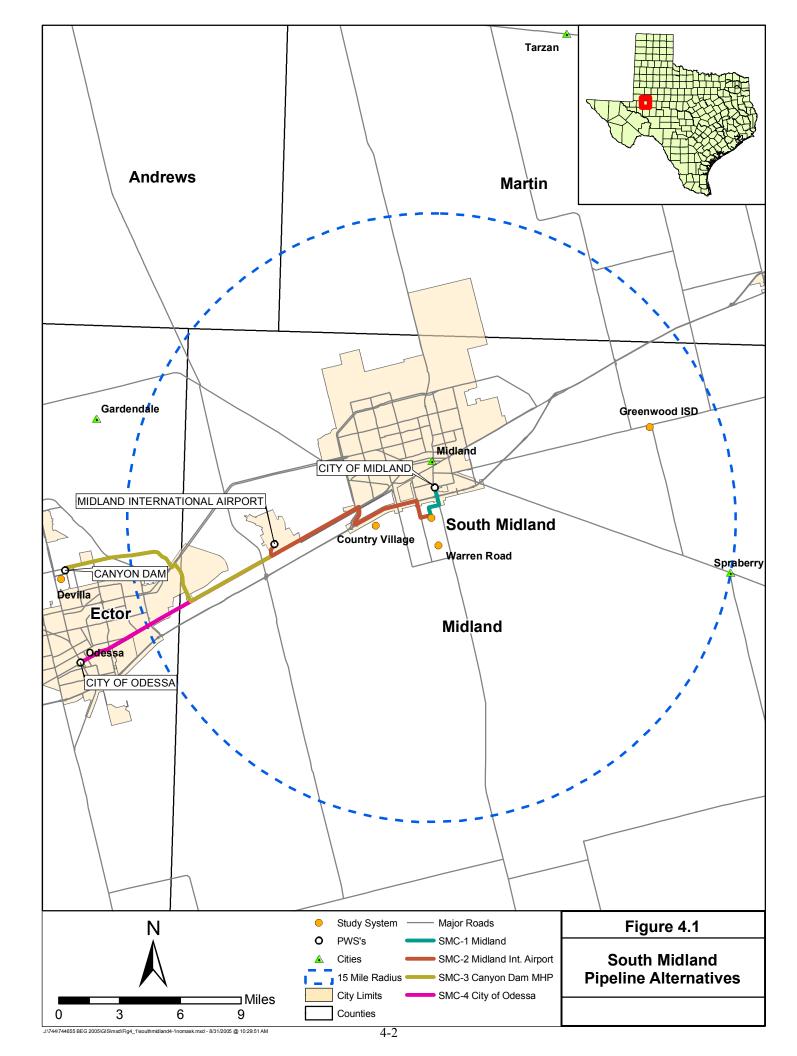
25

The location of the South Midland County Water System is shown on Figure 4.1. The system provides drinking water to the South Midland County Mobile Home Park and is privately owned and operated. There are 75 metered connections in the system.

The water sources for this PWS are two active wells, both of which are completed in the Ogallala Formation (Code 1210GLL). The wells are approximately 80 and 100 feet deep, and are rated at 80 and 30 gallons per minute (gpm), respectively. The water is chlorinated with hypochlorite at each wellhead, before being transferred to a 21,000-gallon storage tank, which then feeds booster pumps that pump into the distribution system. Nitrate concentration of the combined flow from the two wells has recently been in the 10 to 11 mg/L range, and the arsenic concentration averaged around $14 \,\mu g/L$, with TDS concentrations around $1,700 \, mg/L$.

The treatment employed is not appropriate or effective for removal of arsenic or nitrate, so optimization is not expected to be effective in increasing removal of either of these contaminants. There is, however, a potential opportunity for system optimization to reduce arsenic concentration. The system has more than one well, and since arsenic concentrations can vary significantly between wells, arsenic concentrations should be determined for each well. If one or more wells happens to produce water with acceptable arsenic levels, as much production as possible should be shifted to that well. It may also be possible to identify arsenic-producing strata through comparison of well logs or through sampling of water produced by various strata intercepted by the well screen.

- Basic system information is as follows:
- Population served: 256
- Connections: 75
- Average daily flow: 0.024 million gallons per day (mgd)
- Total production capacity: 0.266 mgd
- Typical nitrate range: 10 to 11 mg/L
- Typical arsenic range: 12 to 17 μg/L
- Typical TDS range: 1,500 to 1,900 mg/L



4.1.2 Capacity Assessment for South Midland County Water Systems, Inc.

The project team conducted a capacity assessment of the South Midland County Water System to evaluate the system's FMT capabilities. The evaluation process involved interviews with staff and management who have a responsibility in either the operations or management of the system. The questions were designed to be open ended to provide a better assessment of overall capacity. In general, the technical aspects of capacity are discussed elsewhere in this report. This section focuses on the managerial and financial components of capacity.

The capacity assessment is separated into four categories: general assessment of capacity, positive aspects of capacity, capacity deficiencies, and capacity concerns. The general assessment of capacity describes the overall impression of technical, managerial, and financial capability of the water system. The positive aspects of capacity describe those factors that the system is doing well. These factors should provide opportunities for the system to build upon in order to improve capacity deficiencies. The capacity deficiencies noted are those aspects that are creating a particular problem for the system. Primarily, these problems are related to the system's ability to meet current or future compliance, ensure proper revenue to pay the expenses of running the system, and to ensure the proper operation of the system. The last category is titled capacity concerns. These are items that in general are not causing significant problems for the system at this time. However, the system may want to address them before they do have the opportunity to cause problems.

Raymond Gonzales, the Owner and Manager/Operator, was interviewed for the South Midland County Water System PWS. The interview was conducted in person.

4.1.2.1 General Structure

South Midland County Water System, Inc. is currently under receivership. The only staff for the system is a manager/operator who also owns and operates another small water system in the nearby vicinity. The manager/operator is a certified operator who handles all the O&M as well as meter reading, billing, and collections. The water system has 75 connections, and serves a population of about 256 people. Most of the system is metered and is supplied by groundwater.

4.1.2.2 General Assessment of Capacity

This system has an inadequate level of capacity. Even though the manager/operator has only been involved since November 2004, there are numerous areas that need improvement, especially regarding managerial and financial capabilities.

4.1.2.3 Positive Aspects of Capacity

In assessing a system's overall capacity, it is important to look at all aspects – positive and negative. It is important for systems to understand what those positive characteristics are so they can be continued or strengthened. These positive aspects assist

the system in addressing capacity deficiencies or concerns. The factors that were particularly important for South Midland County are listed below.

- **Dedicated Manager/Operator** There appears to be a long history of problems associated with the management of this system. While there are numerous issues to be resolved, the current manager/operator is providing adequate water service to the residents. In addition, he has taken time to personally repair leaks in individual homes. While this is not his primary responsibility, it does show his commitment to residents. All residents have the phone number of the manager/operator, who is on call 24 hours a day.
- **Disinfection Residual** For maximum public health protection, it is important to have continuous disinfection of the system. The effectiveness of the disinfection process is determined by measuring free chlorine residual. The owner recognizes the importance of maintaining adequate disinfection throughout the system, and his goal is to maintain a free chlorine residual of 0.6 mg/L at the pumphouse and 0.4 mg/L in the distribution system.

4.1.2.4 Capacity Deficiencies

The following capacity deficiencies were noted in conducting the assessment.

- Lack of Cost-Tracking and Reconciling Operating Budget South Midland County does have some type of operating budget that was submitted to TCEQ and the manager/operator files some type of monthly financial reports. However, there does not appear to be any type of process for tracking expenses or reconciling the operating budget with the revenues generated. While the manager/operator has not been involved with the system for very long, it is crucial to have a process for tracking expenses and revenues since the system is in receivership. In addition, this information is critical to preparing an operating budget for the next year as well as determining if the rate schedule is sufficient to cover the cost of providing water service.
- Insufficient Revenue Collection The manager/operator is responsible for all meter reading, billing, and collections. There is a rate schedule that includes fees for hook-up and reconnections, and a policy for disconnection for non-payment. However, there have been occasions when meters were not read on a monthly basis and bills were not sent out. There has not been enough revenue generated to replace a chlorine pump, repair or replace a master meter, or at times, pay the manager/operator's salary. The collection rate is currently estimated to be 50 percent, and the manager/operator has spent time going door-to-door to collect bills. With such a low collection rate it is not possible to determine if the rate schedule is generating enough revenue to cover the cost of providing water service.

- No Reserve Account It appears there is no reserve account for emergencies or future capital expenditures. Again, until sufficient revenue is collected to pay for daily O&M, there is no way to fund a reserve account.
 - Lack of Long-term Capital Improvements Planning There is no long-term or capital improvements planning. Needs are assessed on a day-to-day basis. The lack of planning negatively impacts the system's ability to look long term and develop a budget and associated rate structure to provide for the system's long-term needs.
 - **Insufficient Staffing** Currently there is only one person to manage and operate the system. At this time the manager/operator is providing an adequate level of service, but at least some part-time help for meter reading and sending out bills is needed. It is difficult to determine if revenues generated by the current rate schedule will allow for hiring additional staff since the collection rate is at 50 percent.
 - Lack of Source Water Protection There is no source water protection program for the system. The wells are located in a cultivated field with no protection from runoff or vandals. Source water protection is critical to prevent contamination of the wells.
 - Lack of Consumer Confidence Report and Public Notice Public notices do not appear to have been issued quarterly for nitrates. In addition, a consumer confidence report was not issued to the residents in 2005.

4.1.2.5 Potential Capacity Concerns

The following items were concerns regarding capacity but there are no particular operational, managerial, or financial problems that can be attributed to these items. The system should focus on the deficiencies noted above in the capacity deficiency section. Addressing the items listed below will help in further improving FMT capabilities.

- Preventive Maintenance Program There is no preventive maintenance program. The manager/operator makes repairs on a reactive basis instead of a proactive one. He does carry a small inventory of spare parts in his vehicle. In addition, there is no scheduled maintenance such as line flushing or valve exercising. Without regular schedules of valve exercising, there can be no sure way of identifying those valves that need replacement prior to failure in an emergency.
- Written Procedures There are no written procedures for operational staff. At this time, the manager/operator is capable of operating the system without written procedures. However, if additional staff is hired or if someone else takes over the system, the lack of written procedures may cause problems.

- Mapping The manager/operator does not have any maps available to determine the location of distribution lines. As-built maps are a useful tool that can assist operations staff in identifying areas of line breaks, water quality complaints, and pressure concerns.
 - Cross Connection Control There is no cross-connection control program. To protect public health, it is important to educate consumers about the hazards of cross-connections in the water system, and implement a program to identify and correct any known cross connections.
 - Unaccounted for Water The system does not have any plan to measure unaccounted for water, although it is estimated to be over 10 percent. Any reduction in water loss would reduce the amount of water that must be pumped and/or treated depending on the compliance alternative implemented.
 - **Emergency Planning** The system does not have a written emergency plan, nor does it have emergency equipment such as a back-up generator.

4.2 ALTERNATIVE WATER SOURCE DEVELOPMENT

4.2.1 Identification of Alternative Existing Public Water Supply Sources

Using data drawn from the TCEQ drinking water and TWDB groundwater well databases, PWSs surrounding the South Midland County Water System were reviewed with regard to their reported drinking water quality and production capacity. PWSs that appeared to have water supplies with water quality issues were ruled out from consideration as alternative sources, while those without identified water quality issues were investigated further. If it was determined that these PWSs had excess supply capacity and might be willing to sell the excess, or might be a suitable location for a new groundwater well, the system was taken forward for further consideration.

- Table 4.1 is a list of the existing PWSs within approximately 20 miles of South Midland County Water System. Twenty miles was selected as the radius for the evaluation owing to the large number of PWSs in proximity to the South Midland County Water System.
- Based on the initial screening summarized in Table 4.1, four alternatives were selected for further evaluation. These are summarized in Table 4.2.

Table 4.1 Existing Public Water Systems within 20 miles of South Midland County Water System

System Name	Distance. from South Midland Co. Water Syst.	Comments/ Other Issues
Johns Mobile Home Park	0.5 miles	Small system with WQ issues: arsenic, TDS, nitrate
Warren Road Subdivision	1.5 miles	Small system with WQ issues: arsenic, TDS, nitrate
Twin Oaks Mobile Home Park	2 miles	Small system with WQ issues: arsenic, TDS, nitrate; marginal exceedances: selenium
City of Midland	2 miles	Large system (>1 mgd) that uses both surface water and groundwater. No current violations. Evaluate further .
Country Village Mobile Home Estates	3 miles	Small system with WQ issues: arsenic, TDS, nitrate
Westgate Mobile Home Park	3 miles	Small system with water quality issues: trichloroethylene (TCE) and methyl t-butyl ether (MTBE) have been detected
Valley View Mobile Home Park	5 miles	Small system with WQ issues: arsenic, TDS, nitrate, gross alpha; marginal exceedances: selenium
Airline Mobile Home Park LTD	5.5 miles	Small system with WQ issues: TDS, gross alpha; marginal exceedances: arsenic
Spring Meadow Mobile Home Park	6 miles	Small system with WQ issues: arsenic, TDS; marginal exceedances: nitrate
Pecan Grove Mobile Home Park	7.5 miles	Small system with WQ issues: TDS; marginal exceedances: nitrate
Water Runners Inc.	8 miles	Small system; current use requires extensive treatment to address WQ issues
Midland International Airport	8 miles	Large system (>1 mgd) with marginal arsenic exceedances. Evaluate further .
Pecan Acres Homeowners Association	9 miles	Small system with WQ issues: arsenic, TDS; marginal exceedances: gross alpha
Greenwood ISD	11.5 miles	Small system with WQ issues: arsenic, TDS, nitrate, selenium; marginal exceedances: fluoride
Greenwood Ventures Inc.	12 miles	Small system with WQ issues: arsenic, TDS, nitrate, gross alpha; marginal exceedances: fluoride, selenium
Greenwood Water System	12 miles	Small system with WQ issues: arsenic, fluoride; marginal exceedances: TDS
Odessa Country Club	13 miles	Small system with WQ issues: TDS, nitrate
Centriflo Pump & Machine Co.	17 miles	Small system with WQ issues: TDS, nitrate; marginal exceedances: arsenic
Double H Mobile Home Park	18 miles	Small system with marginal arsenic exceedances
Canyon Dam Mobile Home Park	18 miles	Small system without identified WQ issues. May be possible location for new well. Evaluate further .
Northgate Mobile Home Park 1	18.5 miles	Small system with WQ issues: nitrate, gross alpha, TDS; marginal exceedances: sulfate
DeVilla Mobile Home Park	18.5 miles	Small system with WQ issues: TDS, nitrate, sulfate, gross alpha; marginal exceedances: combined uranium
Depot Water Store	18.5 miles	Small system with WQ issues: TDS
Gardendale County Water Inc.	18.5 miles	Small system with marginal nitrate, TDS exceedances
City of Odessa	19 miles	Large system (>1 mgd) that uses both surface water and groundwater. No current violations. Evaluate further .
Orchard Water Supply	19 miles	Small system with WQ issues: arsenic; marginal exceedances: fluoride, TDS
Sunset Country Club	19.5 miles	Small system with marginal arsenic, TDS exceedances
Huber Garden Estates	20 miles	Small system with WQ issues: arsenic, nitrate, gross alpha, TDS, sulfate

Table 4.2 Public Water Systems within 20 miles of South Midland County Water System Selected for further Evaluation

System Name	Pop	Conn	Total Production (mgd)	Ave Daily Usage (mgd)	Approx. Dist. from South Midland Co. Water System	Comments/ Other Issues
South Midland County Water System	256	75	0.266	0.024	n/a	n/a
City of Midland	98,045	35,494	64.644	23.040	2 miles	Large system (>1 mgd) that uses both surface water and groundwater. No current violations. Evaluate further.
Midland International Airport	1,000	56	1.880	0.327	8 miles	Large system (>1 mgd) with marginal arsenic exceedances. Evaluate further .
Canyon Dam Mobile Home Park	50	19	0.144	nd	18 miles	Small system without identified WQ issues. May be possible location for new well. Evaluate further .
City of Odessa	101,719	41,588	80.2	19.583	19 miles	Large system (>1 mgd) that uses both surface water and groundwater. No current violations. Evaluate further.

4.2.1.1 Colorado River Municipal Water District

The Colorado River Municipal Water District (CRMWD) supplies water to both the Cities of Midland and Odessa and, while it would not supply water directly to the South Midland County Water System, a brief description is included here because of its role in supplying water to these two cities. The CRMWD was authorized in 1949 by the 51st Legislature of the State of Texas for the purpose of providing water to the District's Member cities of Odessa, Big Spring, and Snyder. The CRMWD also has contracts to provide specified quantities of water to the cities of Midland, San Angelo, Stanton, Robert Lee, Grandfalls, Pyote, and Abilene (through the West Central Texas Municipal Water District).

The CRMWD owns and operates three major surface water supplies on the Colorado River in west Texas. These are Lake J. B. Thomas, the E. V. Spence Reservoir, and the O. H. Ivie Reservoir. Together, the full combined capacity of these reservoirs is 1.272 million acre-feet. Additionally, CRMWD operates five well fields for water supply. Three of these fields were developed by the Member Cities prior to 1949. The fourth field, located in Martin County, began delivering water in 1952. The fifth field, located in Ward County southwest of Monahans, can supply up to 28 mgd. CRMWD primarily uses these well fields to supplement surface water deliveries during the summer months.

4.2.1.2 City of Midland

1

2

3

5

6

7

8

9

10

11

12

13 14

15

16

17

18

19

20

21

22

23

24

25

26

27

28

29

30

31

32

33

34

35

36

37

38

39

40

41

42

The City of Midland is located approximately 2 miles northwest of South Midland County Water System. The City of Midland purchases approximately 75 to 80 percent of its water from the CRMWD through a 1966 contract. This purchased water comprises mainly untreated surface water from several reservoirs including Lake J.B. Thomas, Lake E.V. Spence, and Lake O.H. Ivie, though the CRMWD may also supplement the supply with groundwater during the high demand summer months. The City of Midland gets the other 20 to 25 percent of its water from various City-owned well fields, which provide lower quality water. Midland is classified as a member city of CRMWD and is allowed to use alternate water supplies, unlike Odessa whose water can only be provided by CRMWD.

As part of Midland's primary water sources, raw water from CRMWD is delivered to one of three reservoirs. Two of the three reservoirs are owned by CRMWD and include a 15 million gallon reservoir located at the water purification plant and the 100 million gallon Terminal Reservoir located on FM 1788, approximately 2 miles south of Highway 191. The Terminal Reservoir is shared by both Midland and Odessa. The third reservoir, Lake Peggy Sue, is owned by Midland and is located approximately 2 miles west of the City's water treatment plant. In addition to the surface water provided by CRMWD, under a 1995 agreement, Midland owns 16.54 percent of Lake Ivie, located approximately 170 miles southwest of Midland. Each day, 15 million gallons from Lake Ivie and 16 million gallons from CRMWD reservoirs are delivered via pipeline from Ballinger to San Angelo, and then to one of the three reservoirs around Midland.

In addition to CRMWD surface water, the City owns or leases water rights in three well fields. The McMillen well field was in operation from the early 1950s until it was depleted in the mid 1960s. It was used as a reserve water supply but is no longer used following detection of perchlorate in water samples from the well field. The Paul Davis well field, located 30 miles north of Midland, was developed in the late 1950s and is used during peak periods to offset the demand exceeding the 31 mgd provided by the surface water from CRMWD reservoirs. The well field can sustain a pumping rate of 18 to 19 mgd, but normally averages 10 mgd annually. The well field currently consists of two 2.5 million gallon tanks that receive groundwater from 29 wells. These wells are installed between 150 and 200 feet deep in the Ogallala aquifer (Code 121OGLL). Since arsenic, fluoride, perchlorate, and radionuclides were reported, both in samples from individual wells and in batch samples from the well field, the City of Midland carefully monitors the blending of surface water from CRMWD and the groundwater from the Paul Davis Well Field to maintain a potable water supply that does not exceed the MCLs for these constituents. The third well field is the T-Bar Ranch, located in western Winkler County approximately 70 miles west of Midland. This well field is still being developed and will be brought online as the Paul Davis well field is depleted.

The City of Midland operates two treatment plants to treat surface water supplied by CRMWD and provides water to a service population of approximately 100,000. The City

has a total of approximately 35,500 connections, about 32,000 of which are metered. The major users of water in Midland include college, parks, and schools which use the water for irrigation. The current monthly rates per connection are a \$12 base charge for the first 2,000 gallons and \$2.75 for each additional 1,000 gallons.

In the fall of 2003, the Midland City Council decided that water can only be provided to areas annexed by the City of Midland. Consequently, while the City of Midland does have sufficient excess drinking water capacity, any location to receive water from the City would have to agree to be annexed. To be annexed, a commission representing the town to be annexed must submit a petition signed by at least 50 percent of the community residents wanting to be annexed. This commission would then appoint a Public Improvement District to build a water line from a Midland supply line to the community. In the past, Midland has financed the Public Improvement District through the sale of bonds. The community would be subject to the same rates as the other residences in Midland.

4.2.1.3 Midland International Airport

Midland International Airport is located approximately 8 miles west of South Midland County Water System. The Midland International Airport is supplied by 10 groundwater wells which are completed in the Antler Sands aquifer (Code 218ALRS), range in depth from 85 to 130 feet, and are rated from 61 to 203 gpm. These wells are maintained and operated by the City of Midland Utility Department. Water from the wells is chlorinated and piped to an elevated 500,000-gallon storage tank before entering the airport's distribution system. The system is capable of producing up to 1.5 mgd, and average daily consumption is approximately 0.5 mgd.

A Midland consulting firm, Arcadis, is currently evaluating the ability for the Midland International Airport well field to continue meeting the demands of the airport. Data for this report were collected during the summer of 2005, and the evaluation report will be completed in the fall of 2005.

Currently the operators of the PWS do not consider there to be sufficient excess capacity to provide water to offsite facilities or areas. However, based on available water quality data, the location may be a suitable point for a new groundwater well.

4.2.1.4 Canyon Dam Mobile Home Park

Canyon Dam Mobile Home Park is located approximately 18 miles to the west of South Midland County Water System. Canyon Dam Mobile Home Park has a population of 50 and is served by 19 connections. It has two wells, both rated 40 gpm (0.058 mgd), both of which are about 150 feet deep. The owners are currently making plans to install a third well. The water system has a maximum rated capacity of 0.144 mgd. The water is disinfected using hypochlorite prior to distribution. The estimated average and maximum daily demand is 0.007 mgd and 0.026 mgd, respectively.

This system does not currently have sufficient capacity to supply water to another system; however, based on the available water quality data, the location may be a suitable point for a new groundwater well.

4.2.1.5 City of Odessa

The intake point for the City of Odessa is located approximately 19 miles west of South Midland County Water System. The City of Odessa is one of three original members of CRMWD and, by contract, may only obtain its water supply through them. The water supplied to the City of Odessa originates in a network of three reservoirs (Lake Ivie, Lake Spence, and Lake Thomas), but this water may be supplemented with groundwater during the high-demand summer months. The untreated water from the reservoirs is pumped from Ballinger, Texas to San Angelo, Texas via a 60-inch pipeline and then through a 53-inch pipeline from San Angelo northwest to Odessa, which is 1,400 feet higher in elevation than San Angelo. Groundwater is pumped from a well field in Ward County.

The raw water is delivered to a treatment facility, where it is filtered and chlorinated, and then stored in a 4.3 million gallon concrete storage tank prior to distribution to the City of Odessa. In addition to the water delivered via the CRMWD pipeline, a relatively small amount of water (less than 10 percent) is also delivered by a second pipeline from the Ward County well field located approximately 60 miles west of Odessa. This water is pH-adjusted and chlorinated prior to being pumped to the 4.3 million gallon storage tank.

In 2004, approximately 6.7 billion gallons of water were delivered to Odessa from San Angelo via the CRMWD pipeline, and 4.5 percent or 0.31 billion gallons originated from the Ward County well field. Average usage by the City of Odessa ranges from 12 to 15 mgd in the winter to 35 to 36 mgd in the summer. The City of Odessa provides water to a population of approximately 108,000 and has a total of approximately 42,000 connections. The current customer rate per connection for potable water is \$2.50 per 1,000 gallons.

The City of Odessa does have an excess capacity of treated water and may be willing to sell water to other PWSs. A community wanting to purchase treated water from the City of Odessa must submit a formal request to the City for review by the five-member City Council. The community does not have to be annexed in order to receive treated water via pipeline, but it would have to fund the cost of the connecting pipeline.

4.2.2 Potential for New Groundwater Sources

4.2.2.1 Installing New Compliant Wells

Developing new wells or well fields is recommended, provided good quality groundwater available in sufficient quantity can be identified. Since a number of water systems in the area also have problems with nitrate and arsenic, it should be possible to

share in the cost and effort of identifying compliant groundwater and constructing well fields.

Since the PWS is already familiar with well operation, installation of a new well in the vicinity of the system intake point is likely to be an attractive option, provided compliant groundwater can be found. As a result, existing wells identified with good water quality should be investigated. Re-sampling and test pumping would be required to verify and determine the quality and quantity of water at those wells.

The use of existing wells should probably be limited to use as indicators of groundwater quality and availability. If a new groundwater source is to be developed, it is recommended that a new well or wells be installed instead of using existing wells. This will ensure the well characteristics are known and the well construction meets standards for drinking water wells.

4.2.2.2 Results of Groundwater Availability Modeling

Regional groundwater withdrawal in the vicinity of the South Midland County Water System is extensive and likely to remain near current levels over the next decades. In northern Midland County, where the study area is located, two aquifers are potential groundwater sources for public supplies: the Ogallala aquifer, and the downdip of the Edwards-Trinity Plateau aquifer.

Supply wells for the South Midland County Water System and its vicinity withdraw groundwater primarily from the Southern Ogallala aquifer. The aquifer outcrop extends over most of the Texas panhandle and into eastern New Mexico, reaching northern and central Midland County. The 2002 Texas Water Plan anticipates 24 percent depletion in the Ogallala supply over the next decades, from 5,000,097 acre-feet per year estimated in 2000 to 3,785,409 acre-feet per year in 2050. Nearly 95 percent of the groundwater pumped is used for irrigated agriculture.

A GAM for the Ogallala aquifer was recently developed by the TWDB (Blandford, et al. 2003). Modeling was performed to simulate historical conditions and to develop long-term groundwater projections. Predictive simulations using the GAM model indicated that, if estimated future withdrawals are realized, aquifer water levels could decline to a point at which significant regions currently practicing irrigated agriculture could be essentially dewatered by 2050 (Blandford, et al. 2003). The model predicted the most critical conditions for Cochran, Hockley, Lubbock, Yoakum, Terry, and Gaines Counties where the simulated drawdown could exceed 100 feet. For northern Midland County, the simulated drawdown by the year 2050 would be more moderate, within the 0 to 25-feet range (Blandford, et al. 2003). The Ogallala aquifer GAM was not run for the South Midland County Water System. Water use by the system would represent a minor addition to regional withdrawal conditions, making potential changes in aquifer levels beyond the spatial resolution of the regional GAM model.

In northern Midland County, the downdip of the Edwards-Trinity Plateau aquifer underlies the Ogallala aquifer. A GAM for the Edwards-Trinity Plateau aquifer was

- 1 published by the TWDB in September 2004 (Anaya and Jones 2004). GAM data for the
- 2 aquifer indicate that total withdrawal in Midland County had a steady decline in recent
- 3 years, from a peak annual use of 21,127 acre-feet in 1995 to 13,484 acre-feet in 2000.
- 4 This reduced water withdrawal from the Edwards-Trinity Plateau aquifer in Midland
- 5 County is expected to remain nearly constant over the simulation period ending in the
- 6 year 2050 (Anaya and Jones 2004).

8

9 10

11

12

13

14

15

16

17

18 19

20 21

22

23

24

25

26

27

28

29

30

31

32

33

34

35

36

37

4.2.3 Potential for New Surface Water Sources

There is a minimum potential for development of new surface water sources for the South Midland County Water System, as indicated by limited water availability over the entire river basin, and within the site vicinity.

The South Midland County Water System is located in the upper reach of the Colorado River Basin where current surface water availability is expected to steadily decrease as a result of the increased water demand. The TWDB's 2002 Water Plan anticipates an 11 percent reduction in surface water availability in the Colorado River Basin over the next 50 years, from 879,400 acre-feet per year in 2002 to 783,641 acre-feet per year in 2050.

The vicinity of the South Midland County Water System has a minimum availability of surface water for new uses as indicated by the TCEQ's availability maps for the Colorado Basin. In the site vicinity, and over the entire Midland County, unappropriated flows for new uses are available at most 50 percent of the time. This supply is inadequate as the TCEQ requires a 100 percent supply availability for a municipal water supply.

4.2.4 Options for Detailed Consideration

The initial review of alternative sources of water results in the following four options for more-detailed consideration:

- 1. City of Midland. Obtain treated CRMWD water through the City of Midland system. A pipeline and pump station would be constructed to transfer the water to the South Midland County Water System storage tank (Alternative SMC-1).
- 2. Midland International Airport. A new well would be installed in the vicinity of the wells at Midland International Airport. A pipeline and pump station would be constructed to transfer the water to the South Midland County Water System storage tanks (Alternative SMC-2).
- 3. Canyon Dam Mobile Home Park. A new well would be installed in the vicinity of the wells at the Canyon Dam Mobile Home Park. A pipeline and pump station would be constructed to transfer the water to the South Midland County Water System storage tank (Alternative SMC-3).

4. City of Odessa. Obtain treated CRMWD water through the City of Odessa system. A pipeline and pump station would be constructed to transfer the water to the South Midland County Water System storage tank (Alternative SMC-4).

In addition to the location-specific alternatives above, three hypothetical alternatives are considered in which new wells would be installed 10-, 5-, and 1-miles from the South Midland County PWS. Under each of these alternatives, it is assumed that a source of compliant water can be located and then a new well would be completed and a pipeline would be constructed to transfer the compliant water to South Midland County. These alternatives are SMC-9, SMC-10, and SMC-11.

4.3 TREATMENT OPTIONS

12 4.3.1 Centralized Treatment Systems

- 13 Centralized treatment of the well field water is identified as a potential alternative for
- 14 South Midland County Water System. RO and EDR are potential applicable processes.
- RO and EDR can reduce nitrate, TDS, and arsenic to produce compliant water. The
- 16 central RO treatment alternative is Alternative SMC-5, and the central EDR treatment
- 17 alternative is Alternative SMC-6.

18 **4.3.2** Point-of-Use Systems

- 19 POU treatment using RO technology is valid for nitrate and arsenic removal. The
- 20 POU treatment alternative is SMC-7.

21 **4.3.3** Point-of-Entry Systems

- POE treatment using RO technology is valid for nitrate and arsenic removal. The
- 23 POE treatment alternative is SMC-8.

24 **4.4 BOTTLED WATER**

- Providing bottled water is considered an interim measure to be used until a
- 26 compliance alternative is implemented. Even though the community is small and people
- 27 know each other; it would be reasonable to require a quarterly communication advising
- customers of the need to take advantage of a bottled water program. An alternative to
- 29 providing bottled water is to provide a central, publicly accessible dispenser for treated
- 30 drinking water. Alternatives addressing bottled water are SMC-12, SMC-13, and
- 31 SMC-14.

1

2

3

4

5

6 7

8

9

10

11

32 4.5 ALTERNATIVE DEVELOPMENT AND ANALYSIS

- A number of potential alternatives for compliance with the MCL for nitrate and
- 34 arsenic have been identified. Each of the potential alternatives is described in the
- 35 following subsections. It should be noted that the cost information given is the capital
- 36 cost and change in O&M costs associated with implementing the particular alternative.

- 1 Appendix C contains cost estimates for the compliance alternatives. These compliance
- 2 alternatives represent a range of possibilities, and a number of them are likely not
- 3 feasible. However, all have been presented to provide a complete picture of the range of
- 4 alternatives considered. It is anticipated that a PWS will be able to use the information
- 5 contained herein to select the most attractive alternative(s) for more detailed evaluation
- 6 and possible subsequent implementation.

4.5.1 Alternative SMC-1: Purchase Treated Water from the City of Midland

This alternative involves purchasing treated water from the City of Midland, which will be used to supply the South Midland County Water System. The City of Midland currently has sufficient excess capacity for this alternative to be feasible, although current City policy only allows drinking water to be provided to areas annexed by the City. For purposes of this report, in order to allow direct and straightforward comparison with other alternatives, this alternative assumes that water would be purchased from the City. Also, it is assumed that the South Midland County Water System would obtain all its water from the City of Midland.

This alternative would require constructing a pipeline from the City of Midland water main to the existing storage tank for the South Midland County Water System. A pump station would also be required to overcome pipe friction and elevation differences between Midland and the South Midland County Water System. The required pipeline would be approximately 2 miles long and would be constructed of 4-inch polyvinyl chloride (PVC) pipe.

The pump station would include two pumps, including one standby, and would be housed in a building. A storage tank would also be constructed for the pumps to draw from. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the South Midland County Water System, since the incremental cost would be relatively small, and would provide operational flexibility.

This alternative involves regionalization by definition, since the South Midland County Water System would be obtaining drinking water from an existing larger supplier. Also, other PWSs near South Midland County Water System are in need of compliant drinking water and could share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the purchase price for the treated water minus the cost related to current operation of the South Midland County Water System wells, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$552,500, and the alternatives' estimated annual O&M cost is \$18,400.

The reliability of adequate amounts of compliant water under this alternative should be good. City of Midland provides treated surface water on a large scale, facilitating adequate O&M resources. From South Midland County Water System's perspective, this

- alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood. If the decision is made to perform blending, then the operational complexity would increase.
 - The feasibility of this alternative is dependent on an agreement being reached with the City of Midland to purchase treated drinking water.

4.5.2 Alternative SMC-2: New Well at Midland International Airport

This alternative consists of drilling a new well in the Midland International Airport area that would replace South Midland County Water System's wells. Records indicate nitrate levels in a range of 4 to 6 mg/L in the Midland International Airport wells, which is not low enough to provide a high confidence level that blending would be possible. As a result, for this alternative, it is assumed that South Midland County Water System would obtain all its water from the new well.

This alternative would require drilling a new well and installing a well pump, small ground storage tank, a pump station with two transfer pumps, and a pipeline to the South Midland County Water System. One of the two pumps in the pump station is for backup in case the other pump fails. The pipeline, approximately 11 miles long, would primarily follow I-20W and Business Route I-20W, and would be a 4-inch PVC line that discharges to the existing storage tank in South Midland County Water System.

This alternative presents a limited regional solution, since other PWSs in the area also need compliant water. Some regionalization could be accomplished by sharing the cost of drilling the well and possibly constructing the pipeline and pump station with other non-compliant PWSs in the area.

The estimated capital cost for this alternative includes constructing a new well and small ground storage tank, a pump station with two transfer pumps, and a pipeline to the South Midland County Water System. The estimated O&M cost for this alternative are related to taking the existing well field out of service, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$2.69 million, and the estimated annual O&M cost for this alternative is \$33,000.

The reliability of adequate amounts of compliant water under this alternative should be good. South Midland County Water System has a well field with adequate capacity. From the South Midland County Water System' perspective, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pumps stations is well understood, and South Midland County Water System currently operates pumps.

The feasibility of this alternative is dependent on finding a suitable well site.

4.5.3 Alternative SMC-3: New Well at Canyon Dam Mobile Home Park

This alternative consists of drilling a new well in the Canyon Dam Mobile Home Park area that would replace South Midland County Water System's wells. Records indicate nitrate levels around 5 mg/L in the Canyon Dam Mobile Home Park wells, which is not low enough to provide a high confidence level that blending will be possible. As a result, for this alternative, it is assumed that South Midland County Water System would obtain all its water from the new well.

This alternative would require drilling a new well and installing a well pump, small ground storage tank, a pump station with two transfer pumps, and a pipeline to the South Midland County Water System. One of the two pumps in the pump station is for backup in case the other pump fails. The 4-inch PVC pipeline would primarily follow I-20W and Business Route I-20W, would be approximately 23 miles long, and would discharge to the existing storage tank in South Midland County Water System.

This alternative presents a limited regional solution, since other PWSs in the area also need compliant water. Some regionalization could be accomplished by sharing the cost of drilling the well and possibly constructing the pipeline and pump station with other non-compliant PWSs in the area.

The estimated capital cost for this alternative includes constructing a new well and small ground storage tank, a pump station with two transfer pumps, and a pipeline to the South Midland County Water System. The estimated O&M cost for this alternative includes additional costs related to taking the existing well field out of service, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$5.39 million, and the estimated annual O&M cost for this alternative is \$58,200.

The reliability of adequate amounts of compliant water under this alternative should be good. South Midland County Water System has a well field with adequate capacity. From South Midland County Water System's perspective, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pumps stations is well understood, and South Midland County Water System currently operates pumps.

The feasibility of this alternative is dependent on finding a suitable well site.

4.5.4 Alternative SMC-4: Purchase Treated Water from the City of Odessa

This alternative involves purchasing treated surface water from the City of Odessa, which will be used to supply the South Midland County Water System. The City of Odessa currently has sufficient excess capacity for this alternative to be feasible and has indicated it would be amenable to negotiating an agreement to supply water to PWSs in the area. Records indicate the City of Odessa water has low levels of nitrate (less than 1 mg/L) and arsenic (less than 0.004 mg/L), which is low enough to make blending a

realistic consideration. However, for this alternative, it is assumed the South Midland County Water System would obtain all its water from the City of Odessa.

This alternative would require constructing a pipeline from the City of Odessa water main to the existing storage tank for the South Midland County Water System. A pump station would also be required to overcome pipe friction and the elevation differences between Odessa and South Midland County Water System. The 4-inch PVC pipeline would primarily follow I-20W and Business Route I-20W and would be approximately 21 miles long.

The pump station would include two pumps, including one standby, and would be housed in a building. A tank would also be constructed for the pumps to draw from. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the South Midland County Water System, since the incremental cost would be relatively small, and it would provide operational flexibility.

This alternative involves regionalization by definition, since South Midland County Water System would be obtaining drinking water from an existing larger supplier. It is possible that the South Midland County Water System could turn over provision of drinking water to the City of Odessa instead of purchasing water. Also, other PWSs near South Midland County Water System are in need of compliant drinking water and could share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the purchase price for the treated water minus the cost related to current operation of the South Midland County Water System wells, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$4.86 million, and the alternatives' estimated annual O&M cost is \$63,400.

The reliability of adequate amounts of compliant water under this alternative should be good. City of Odessa provides treated surface water on a large scale, facilitating adequate O&M resources. From South Midland County Water System's perspective, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood. If the decision was made to perform blending then the operational complexity would increase.

The feasibility of this alternative is dependent on an agreement being reached with the City of Odessa to purchase treated drinking water.

4.5.5 Alternative SMC-5: Central RO Treatment

This system would continue to pump water from the South Midland County Water System well field, and would treat the water through an RO system prior to distribution. For this option, a fraction of the raw water would be treated and then blended with the untreated stream to obtain overall compliant water. The RO process concentrates

impurities in the reject stream which would require disposal. It is estimated the RO reject generation would be 45,000 gpd when the system is operated at full flow.

This alternative consists of constructing the RO treatment plant near the existing South Midland County Water System service pumps. The plant is composed of a 500 square foot building with a paved driveway; a skid with the pre-constructed RO plant; two transfer pumps, a 20,000-gallon tank for storing the treated water, and a 260,000-gallon pond for storing reject water. The treated water would be chlorinated and stored in the new treated water tank prior to being pumped into the distribution system. The existing above-grade storage tank would continue to be used to accumulate feed water from the well field. The entire facility is fenced. The capital cost includes purchase of a water truck-trailer to periodically haul reject water for disposal.

The estimated capital cost for this alternative is \$655,900, and the estimated annual O&M cost is \$71,300.

The reliability of adequate amounts of compliant water under this alternative is good, since RO treatment is a common and well-understood treatment technology. However, O&M efforts required for the central RO treatment plant may be significant, and O&M personnel would require training with RO. The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.6 Alternative SMC-6: Central EDR Treatment

The system would continue to pump water from the South Midland County Water System well field, and would treat the water through an EDR system prior to distribution. For this option the EDR would treat the full flow without bypass as the EDR operation can be tailored for desired removal efficiency. It is estimated the EDR reject generation would be 26,000 gpd when the system is operated at full flow.

This alternative consists of constructing the EDR treatment plant near the existing South Midland County Water System service pumps. The plant is composed of a 600 square foot building with a paved driveway; a skid with the pre-constructed EDR system; two transfer pumps; a 20,000-gallon tank for storing the treated water, and a 260,000-gallon pond for storing reject water. The treated water would be chlorinated and stored in the new treated water tank prior to being pumped into the distribution system. The existing above-grade storage tank would continue to be used to accumulate feed water from the well field. The entire facility is fenced. The capital cost includes purchase of a water truck-trailer to periodically haul reject water for disposal.

The estimated capital cost for this alternative is \$900,500, and the estimated annual O&M cost is \$66,500.

The reliability of adequate amounts of compliant water under this alternative is good, since EDR treatment is a common and well-understood treatment technology. However, O&M efforts required for the central EDR treatment plant may be significant, and O&M

personnel would require training with EDR. The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.7 Alternative SMC-7: Point-of-Use Treatment

This alternative consists of the continued operation of the two existing active South Midland County Water System wells, plus treatment of water to be used for drinking or food preparation at the point of use to remove nitrate and arsenic. The purchase, installation, and maintenance of POU treatment systems to be installed "under the sink" would be necessary for this alternative. Blending is not an option in this case. Reverse osmosis POU treatment units would also be effective for reducing other potential contaminants such as TDS and sulfate.

This alternative would require installing the POU treatment units in dwellings and other buildings that provide drinking or cooking water. South Midland County Water System would be responsible for purchasing and maintaining the treatment units, including membrane and filter replacement, periodic sampling, and necessary repairs. In residences, the most convenient point for installing treatment units is typically under the kitchen sink, with a separate tap installed for dispensing treated water. Installation of the treatment units in kitchens would require entry by South Midland County Water System or contract personnel into residences of customers. As a result, the cooperation of customers would be important for success in implementing this alternative. The treatment units could be installed so access could be made without entry into the residence, which would complicate the installation and increase costs.

POU RO treatment processes typically produce liquid waste streams equal in volume to the treated water and require disposal. These waste streams result in an increased overall volume of water used. POU systems have the advantage that only a minimum volume of water is treated (only that for human consumption). This minimizes the size of the treatment units, the increase in water required, and the waste for disposal. For this alternative, it is assumed that the increase in water consumption is insignificant in terms of supply cost and that the waste stream can be recovered for reuse or discharged to the house sewer or septic system.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the POU treatment systems. The estimated O&M cost for this alternative includes purchasing and replacing filters and membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is \$49,500, and the estimated annual O&M cost for this alternative is \$46,900. For the cost estimate, it is assumed that one POU treatment unit would be required for each of the 75 existing connections to the South Midland County Water System. It should be noted that the POU treatment units would need to be more complex than units typically found in commercial retail outlets in order to meet regulatory requirements, making purchase and installation more expensive.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of the customers for system installation, use, and maintenance, and only provides compliant water to single tap within a house. Additionally, the O&M efforts required for the POU systems will be significant, and South Midland County Water System personnel are inexperienced in this type of work. From the perspective of South Midland County Water System, this alternative would be characterized as more difficult to operate due to the in-home requirements.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.8 Alternative SMC-8: Point-of-Entry Treatment

This alternative consists of the continued operation of the two existing active South Midland County Water System wells, plus treatment of water to remove nitrate and arsenic as it enters the residence. The purchase, installation, and maintenance of the treatment systems at the POE would be necessary for this alternative. Blending is not an option in this case. Reverse osmosis POE treatment units would also be effective for reducing other potential contaminants such as TDS and sulfate.

This alternative would require installing the POE treatment units at dwellings and other buildings that provide water for drinking or cooking. South Midland County Water System would be responsible for purchasing and maintaining the treatment units, including membrane and filter replacement, periodic sampling, and necessary repairs. The plumbing in houses should be investigated to ensure that the aggressive water that would result from RO treatment would not cause damage. It may also be desirable to modify piping so that water for non-consumptive uses could be withdrawn upstream of the treatment unit. The POE treatment units would be installed outside the residence, so entry would not be necessary for O&M. Some cooperation from customers would be necessary for installation and maintenance of the treatment systems.

POE RO treatment processes typically produce liquid waste streams that are equal in volume to the treated water and require disposal. These waste streams result in an increased overall volume of water used. POE systems treat a greater volume of water than POU systems. For this alternative, it is assumed that the increase in water consumption is insignificant in terms of supply cost and that the waste stream can be recovered for reuse or discharged to the house sewer or septic system.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the POE treatment systems. The estimated O&M cost for this alternative includes purchasing and replacing filters and membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is \$866,300, and the estimated annual O&M cost for this alternative is \$105,000. For the cost estimate, it is assumed that one POE treatment unit would be required for each of the 75 existing connections to the South Midland County Water System.

The reliability of adequate amounts of compliant water under this alternative is fair, but better than POU systems since it relies less on the active cooperation of customers for system installation, use, and maintenance, and compliant water is supplied to all taps within a residence. Additionally, the O&M efforts required for the POE systems would be significant, and South Midland County Water System personnel are inexperienced in this type of work. From the perspective of South Midland County Water System, this alternative would be characterized as more difficult to operate due to the on-property requirements.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.9 Alternative SMC-9: New Well at 10 miles

This alternative consists of installing a new well within 10 miles of South Midland County Water System that would produce compliant water in place of the water produced by the existing two active wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed. To address a range of solutions, three different well alternatives are developed, assuming the new well is located within 10 miles, 5 miles, and 1 mile from the existing intake point.

This alternative would require constructing a new 300-foot well, a new pump station with storage tank near the new well, and a pipeline from the new well/tank to the existing intake point for the South Midland County Water System. The pump station and storage tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be approximately 10 miles long, and would be a 4-inch PVC line that discharges to the existing storage tank at South Midland County Water System. The pump station would include two pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with one or more nearby systems.

The estimated capital cost for this alternative includes installing the well and constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the cost for O&M for the pipeline and pump station, plus an amount for plugging and abandoning (in accordance with TCEQ requirements) the South Midland County Water System well field. The estimated capital cost for this alternative is \$2.30 million, and the estimated annual O&M cost for this alternative is \$17,100.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of South Midland County Water System, this alternative would be similar to the existing system in terms of operation. South Midland County Water System has experience with O&M of wells and pumps.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is likely an alternate groundwater source would not be found on land controlled by South Midland County Water System, so landowner cooperation would be required.

4.5.10 Alternative SMC-10: New Well at 5 miles

This alternative consists of installing a new well within 5 miles that would produce compliant water in place of the water produced by the South Midland County Water System wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing a new 300-foot well, a new pump station with storage tank near the new well, and a pipeline from the new well/tank to the existing intake point for the South Midland County Water System. The pump station and storage tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be approximately 5 miles long, and would be a 4-inch PVC line that discharges to the existing storage tank at South Midland County Water System. The pump station would include two pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with one or more nearby systems.

The estimated capital cost for this alternative includes installing the well, and constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the cost for O&M for the pipeline and pump station, plus an amount for plugging and abandoning (in accordance with TCEQ requirements) the South Midland County Water System well field. The estimated capital cost for this alternative is \$1.30 million, and the estimated annual O&M cost for this alternative is \$12,600.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of South Midland County Water System, this alternative would be similar in terms of operation as the existing system. South Midland County Water System has experience with O&M of wells and pumps.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is likely an alternate groundwater source would not be found on land controlled by South Midland County Water System, so landowner cooperation would be required.

4.5.11 Alternative SMC-11: New Well at 1 mile

This alternative consists of installing a new well within 1 mile that would produce compliant water in place of the water produced by the existing two active wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing a new 300-foot well, and a pipeline from the new well to the existing intake point for the South Midland County Water System. For this alternative, the pipeline is assumed to be approximately 1 mile long, and would be a 4-inch PVC line that discharges to the existing storage tank at South Midland County Water System.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes cost to install the well, and construct the pipeline. The estimated O&M cost for this alternative includes the cost for O&M for the pipeline, plus an amount for plugging and abandoning (in accordance with TCEQ requirements) the South Midland County Water System well field. The estimated capital cost for this alternative is \$266,400, and the estimated annual O&M cost for this alternative is \$6,400 less than current costs.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells and pipelines are commonly employed. From the perspective of South Midland County Water System, this alternative would be similar in terms of operation compared to the existing system. South Midland County Water System has experience with O&M of wells.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is possible that an alternate groundwater source would not be found on land controlled by South Midland County Water System, so landowner cooperation may be required.

4.5.12 Alternative SMC-12: Public Dispenser for Treated Drinking Water

This alternative consists of the continued operation of the existing two active South Midland County Water System wells, plus dispensing treated water for drinking and cooking at a publicly accessible location. Implementing this alternative would require purchasing and installing a treatment unit where customers would be able to fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of water requires treatment, but customers would be required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

South Midland County Water System would be responsible for maintenance of the treatment unit, including membrane and filter replacement, periodic sampling, and necessary repairs. A method for disposal of the reject waste stream produced by the treatment system will have to be found. This alternative relies on a great deal of cooperation and action from the customers to be effective.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the treatment system to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes purchasing and replacing filters and membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is \$11,600, and the estimated annual O&M cost for this alternative is \$16,700.

The reliability of adequate amounts of compliant water under this alternative is fair, because of the large amount of effort required from the customers and the associated inconvenience. South Midland County Water System has not provided this type of service in the past. From the perspective of South Midland County Water System, this alternative would be characterized as relatively easy to operate, since these types of treatment units are highly automated, and there is only one unit.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.13 Alternative SMC-13: 100 Percent Bottled Water Delivery

This alternative consists of the continued operation of the existing two active South Midland County Water System wells, but compliant drinking water will be delivered to customers in containers. This alternative involves setting up and operating a bottled water delivery program to serve all the customers in the system. It is expected that South Midland County Water System would find it convenient and economical to contract a bottled water service. The bottle delivery program would have to be flexible enough to allow for delivery of smaller containers should customers be incapable of lifting and manipulating 5-gallon bottles. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

This alternative does not involve capital cost for construction, but would require some initial costs for system setup, and then ongoing costs to have the bottled water furnished. It is assumed for this alternative that bottled water is provided to 100 percent of the South Midland County Water System customers.

This alternative does not present options for a regional solution.

The estimated initial capital cost is for setting up the program. The estimated O&M cost for this alternative includes program administration and purchase of the bottled water. The estimated initial cost for this alternative is \$23,900, and the estimated annual

O&M cost for this alternative is \$173,200. For the cost estimate, it is assumed that each person requires 1 gallon of bottled water per day.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of customers to order and utilize the water. Management and administration of the bottled water delivery program will require attention from South Midland County Water System.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.14 Alternative SMC-14: Public Dispenser for Trucked Drinking Water

This alternative consists of continued operation of the existing two active South Midland County Water System wells, plus dispensing compliant water for drinking and cooking at a publicly accessible location. The compliant water would be purchased from a nearby system with compliant drinking water, and delivered by truck to a tank at a central location where customers would be able to fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of compliant water is required, but customers are required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

South Midland County Water System would contract a trucked drinking water service and install a storage tank. It is assumed the storage tank would be filled once a week, and that the chlorine residual would be tested for each truckload. This alternative relies on cooperation and action from the customers for it to be effective.

This alternative presents limited options for a regional solution if two or more systems share the purchase and operation of the water truck.

The estimated capital cost for this alternative includes constructing the storage tank to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes the contract water delivery service, maintenance for the tank, water quality testing, and record keeping. The estimated capital cost for this alternative is \$103,000, and the estimated annual O&M cost for this alternative is \$14,900.

The reliability of adequate amounts of compliant water under this alternative is fair because of the large amount of effort required from the customers and the associated inconvenience. South Midland County Water System has not provided this type of service in the past. From the perspective of South Midland County Water System, this alternative would be characterized as relatively easy to operate, but the water hauling and storage would have to be done with care to ensure sanitary conditions.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

1 4.5.15 Summary of Alternatives

- Table 4.3 provides a summary of the key features of each alternative for South
- 3 Midland County Water System.

Table 4.3 Summary of Compliance Alternatives for South Midland County Water System

Alt No.	Alternative Description	Major Components	Capital Cost ¹	Annual O&M Cost	Total Annualized Cost	Reliability	System Impact	Remarks	
SMC-1	Purchase treated water from the City of Midland	- Storage tank - Pump station - 2-mile pipeline	\$552,500	\$18,400	\$66,500	Good	N	Agreement must be successfully negotiated with the City of Midland. City currently requires annexation before it will do this. Blending may be possible.	
SMC-2	New Well at Midland International Airport	- New well - Storage tank - Pump station - 11-mile pipeline	\$2,694,100	\$33,000	\$267,900	Good	N	Agreement must be successfully negotiated with Midland International Airport, or land must be purchased. Blending not possible. Costs could be shared with other nearby small systems.	
SMC-3	New Well at Canyon Dam Mobile Home Park	- New well - Storage tank - Pump station - 23-mile pipeline	\$5,386,400	\$58,200	\$527,800	Good	Z	Agreement must be successfully negotiated with Canyon Dam Mobile Home Park, or land must be purchased. Blending not possible. Costs could be shared with other nearby small systems.	
SMC-4	Purchase treated water from the City of Odessa	- Storage tank - Pump station - 21-mile pipeline	\$4,857,400	\$63,400	\$486,900	Good	N	Agreement must be successfully negotiated with the City of Midland. Blending may be possible. Costs could be shared with other nearby small systems.	
SMC-5	Continue operation of current well field with central RO treatment	- Central RO treatment plant	\$655,900	\$71,300	\$128,500	Good	Т	Costs could possibly be shared with other nearby small systems.	
SMC-6	Continue operation of current well field with central EDR treatment	- Central EDR treatment plant	\$900,500	\$66,500	\$145,000	Good	Т	Costs could possibly be shared with other nearby small systems.	
SMC-7	Continue operation of current well field, with POU treatment	- POU treatment units	\$49,500	\$46,900	\$51,200	Fair	T, M	Only one compliant tap in home. Cooperation of residents required for installation, maintenance, and testing.	
SMC-8	Continue operation of current well field, with POE treatment	- POE treatment units	\$866,300	\$105,000	\$180,500	Fair (better than POU)	T, M	All home taps compliant and less resident cooperation required.	
SMC-9	Install new compliant well within 10 miles	- New well - Storage tank - Pump station - 10-mile pipeline	\$2,296,000	\$17,100	\$217,300	Good	N	May be difficult to find well with good water quality. Costs could be shared with other nearby small systems.	
SMC-10	Install new compliant well within 5 miles	- New well - Storage tank - Pump station - 5-mile pipeline	\$1,296,800	\$12,600	\$125,600	Good	N	May be difficult to find well with good water quality. Costs could be shared with other nearby small systems.	
SMC-11	Install new compliant well within 1 mile	- New well - Storage tank - Pump station - 1-mile pipeline	\$266,400	\$(6,400)	\$16,800	Good	N	May be difficult to find well with good water quality. Costs could be shared with other nearby small systems.	

Alt No.	Alternative Description	Major Components	Capital Cost ¹	Annual O&M Cost	Total Annualized Cost	Reliability	System Impact	Remarks
SMC-12	Continue operation of current well field, but furnish public dispenser for treated drinking water	- Water treatment and dispenser unit	\$11,600	\$16,700	\$17,700	Fair/interim measure	Т	INTERIM SOLUTION: Does not provide compliant water to all taps, and requires a lot of effort by customers.
SMC-13	Continue operation of current well field, but furnish bottled drinking water for all customers	- Set up bottled water system	\$23,900	\$173,200	\$175,300	Fair/interim measure	М	INTERIM SOLUTION: Does not provide compliant water to all taps, and requires customers to order and use. Management of program may be significant.
SMC-14	Continue operation of current well field, but furnish public dispenser for trucked drinking water	- Construct storage tank and dispenser - Purchase potable water truck	\$103,000	\$14,900	\$23,900	Fair/interim measure	М	INTERIM SOLUTION: Does not provide compliant water to all taps, and requires a lot of effort by customers.

2	Notes:	N – No significant increase required in technical or management capability
3		T – Implementation of alternative will require increase in technical capability
4		M – Implementation of alternative will require increase in management capability
5		1 – See cost breakdown in Appendix C
6		2 – 20-year return period and 6 percent interest

4.6 **COST OF SERVICE AND FUNDING ANALYSIS**

To evaluate the financial impact of implementing the compliance alternatives, a 30-year financial planning model was developed. This model can be found in Appendix D. The financial model is based on estimated cash flows, with and without implementation of the compliance alternatives. Data for such models are derived from established budgets, audited financial reports, published water tariffs, and consumption data.

This analysis will need to be performed in a more detailed fashion and applied to alternatives that are deemed attractive and worthy of more detailed evaluation. A more detailed analysis should include additional factors such as:

• Cost escalation,

1

2

4

5

6 7

8

9

10

11

12

13

18 19

22

23

24

25

26 27

28

29

30

- Price elasticity effects where increased rates may result in lower water consumption,
- 14 • Costs for other system upgrades and rehabilitation needed to maintain compliant operation. 15

4.6.1 **South Midland County Financial Data** 16

17 The South Midland County Water System is currently in court-appointed receivership, with the current appointed operator having limited financial data on the system. Financial data on system expenditures for this water system were based on 20 estimates and limited financial data provided by the court-appointed receiver.

Current Financial Condition 21 4.6.2

4.6.2.1 Cash Flow Needs

Based on estimates provided by the system operator, the current average annual water cost for residential customers of the South Midland County Water System is estimated to be \$480, or approximately 1.0 percent of the annual household income of \$31,152, as given in the 2000 Census. The basic monthly rate structure is \$20 for the first 2,000 gallons, with a second rate of \$4 for each additional 1,000 gallons. Because of the lack of financial data for the water system, it is difficult to determine exact cash flow needs.

4.6.2.2 Ratio Analysis

- 31 Current Ratio
- 32 The Current Ratio for the South Midland County Water System could not be 33 determined due to lack of the necessary financial data to determine this ratio.

1 Debt to Net Worth Ratio

- A Debt-to-Net-Worth Ratio also could not be determined due to the lack of the necessary financial data to determine this ratio.
- *Operating Ratio* = 1.0

Because of the lack of complete separate financial data for expenses specifically related to the South Midland County Water System, the Operating Ratio could not be accurately determined. The system's estimated operating revenues based on 100 percent of account collection approximates \$26,000. However, the monthly collection rate ranged from only to 50-75 percent of the monthly amount due.

4.6.3 Financial Plan Results

Each compliance alternative for the South Midland County Water System was evaluated, with emphasis on the impact on affordability (expressed as a percentage of household income), and the overall increase in water rates necessary to pay for the improvements. Each alternative was examined under the various funding options described in Section 2.4.

For State Revolving Fund funding options, customer MHI compared to the state average determines the availability of subsidized loans. Since the MHI for customers of South Midland County Water System was not available, county-wide data were used. Midland County, where the South Midland County Water System is located, had an annual household income of \$39,082 according to the 2000 U.S. Census compared to a statewide average of \$39,927. Consequently, South Midland County Water System would not qualify for an interest rate of 0 or 1 percent since county incomes are in excess of 70 percent of the state average.

Results of the financial impact analysis are provided in Table 4.4 and Figure 4.2. Figure 4.2 provides a bar chart that in terms of the yearly billing to an average customer (6,000 gallons/month consumption) shows the following:

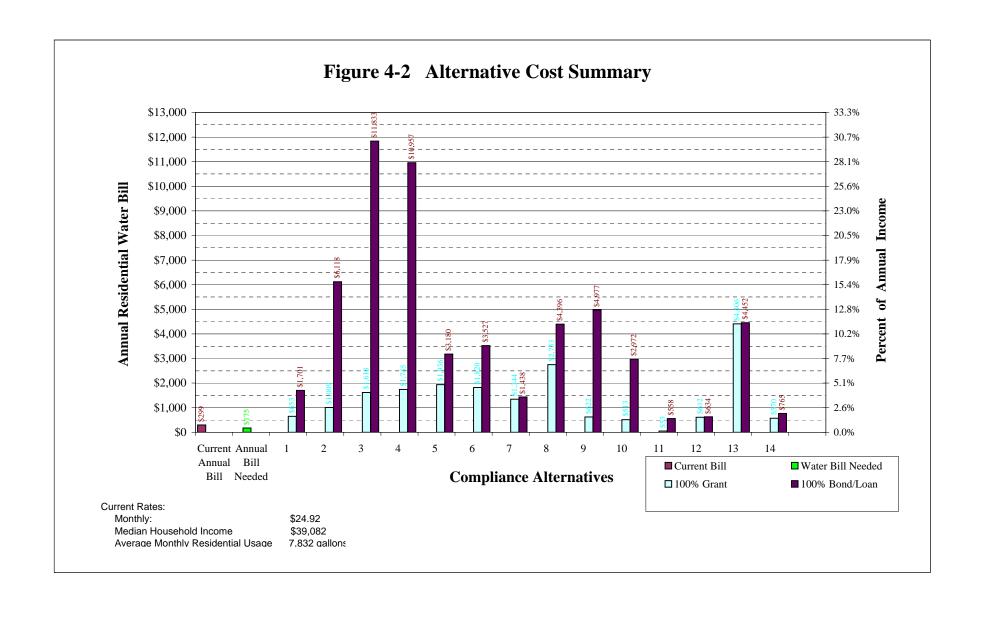
- Current yearly billing, and
- Projected yearly billing including rate increases to maintain financial viability and also for implementing the various compliance alternatives.

The two bars shown for each compliance alternative represent the maximum rate increases necessary assuming 100 percent grant funding and 100 percent loan/bond funding. Most funding options would fall between 100 percent grant and 100 percent loan/bond funding, with the exception of 100 percent revenue financing. If existing reserves are insufficient to fund a compliance alternative, rates would need to be raised before implementing the compliance alternative. This would allow for accumulation of sufficient reserves to avoid larger but temporary rate increases during the years the compliance alternative was being implemented.

Table 4.4 Financial Impact on Households for South Midland County Water System

	Funding Source #	0	1	2	3	4	5
		All Revenue	100% Grant	75% Grant	50% Grant	SRF	Loan/Bond
ALTERNATIVES							
SMC-1	% of HH Income	36%	3%	4%	5%	7%	8%
	Rate Increase %	4544%	245%	420%	595%	860%	945%
	Year	2006	2007	2006	2006	2006	2006
SMC-2	% of HH Income	169%	4%	11%	18%	27%	31%
	Rate Increase %	22006%	482%	1336%	2189%	3484%	3896%
	Year	2006	2007	2006	2006	2006	2006
SMC-3	% of HH Income	338%	8%	21%	34%	54%	60%
	Rate Increase %	44013%	889%	2595%	4302%	6890%	7714%
	Year	2006	2007	2006	2006	2006	2006
SMC-4	% of HH Income	305%	8%	20%	32%	50%	55%
	Rate Increase %	39771%	974%	2513%	4051%	6385%	7129%
	Year	2006	2007	2006	2006	2006	2006
SMC-5	% of HH Income	45%	9%	11%	12%	15%	16%
	Rate Increase %	5810%	1102%	1310%	1517%	1832%	1933%
	Year	2006	2007	2006	2006	2006	2006
SMC-6	% of HH Income	60%	9%	11%	13%	16%	17%
	Rate Increase %	7751%	1024%	1309%	1594%	2027%	2165%
	Year	2006	2007	2006	2006	2006	2006
SMC-7	% of HH Income	6%	6%	6%	6%	7%	7%
	Rate Increase %	706%	706%	722%	738%	761%	769%
	Year	2006	2007	2007	2007	2007	2007
SMC-8	% of HH Income	60%	13%	15%	18%	21%	22%
	Rate Increase %	7786%	1648%	1922%	2197%	2613%	2745%
	Year	2006	2007	2006	2006	2006	2006
SMC-9	% of HH Income	144%	2%	8%	14%	22%	25%
	Rate Increase %	18653%	224%	951%	1679%	2782%	3133%
_	Year	2006	2007	2006	2006	2006	2006
SMC-10	% of HH Income	81%	2%	5%	8%	13%	15%
	Rate Increase %	10525%	151%	562%	972%	1595%	1794%
	Year	2006	2007	2006	2006	2006	2006

SMC-11	% of HH Income	17%	1%	1%	1%	2%	2%
	Rate Increase %	2104%	0%	0%	38%	166%	206%
	Year	2006	2007	2006	2006	2006	2006
SMC-12	% of HH Income	2%	2%	2%	2%	3%	3%
	Rate Increase %	217%	217%	220%	224%	230%	231%
	Year	2006	2007	2007	2007	2007	2007
SMC-13	% of HH Income	22%	22%	22%	22%	22%	22%
	Rate Increase %	2752%	2752%	2760%	2767%	2779%	2782%
	Year	2006	2007	2007	2007	2007	2007
SMC-14	% of HH Income	7%	2%	2%	3%	3%	3%
	Rate Increase %	876%	189%	222%	254%	304%	319%
	Year	2006	2008	2008	2008	2007	2007



1 2	SECTION 5 REFERENCES
3 4 5 6	Anaya, R. and I. Jones. 2004. Groundwater Availability Model for the Edwards-Trinity (Plateau) and Cenozoic Pecos Alluvium Aquifer Systems, Texas. Texas Water Development Board GAM Report (available online at http://www.twdb.state.tx.us/gam/index.htm).
7 8	Ashworth, J. B., and J. Hopkins. 1995. Aquifers of Texas. Texas Water Development Board Report 345:68 p.
9 10	Barker, R. A., and A. F. Ardis. 1996. Hydrogeologic framework of the Edwards-Trinity aquifer system, west-central Texas. U.S. Geol. Surv. Prof. Paper 1421-B, 61 p.
11 12	Bartolino, J. R. 1994. Source of nitrate nitrogen in the Seymour Aquifer, Knox County, Texas. AGU Abs. with Programs, Spring Meeting, May 23-27, Baltimore, Maryland, H22A-2.
13 14 15 16	Blandford, T.N., D.J. Blazer, K.C. Calhoun, A.R. Dutton, T. Naing, R.C. Reedy, and B.R. Scanlon. 2003. GAM of the Southern Ogallala Aquifer in Texas and New Mexico: Numerical Simulations Through 2050 (available online at http://www.twdb.state.tx.us/gam/index.htm).
17	Carlson, G. 2003. Point-of-Entry, Point-of-Use, Bottled Water. Powerpoint® Presentation.
18 19	Dow Chemicals Design Center. 2004. <i>Ion Exchange or Reverse Osmosis</i> . www.dow.com/liquidseps.
20 21 22	Ewing, J., T. Jones, J. Pickens, A. Chastain-Howley, and K. E. Dean. 2004. Groundwater availability model for the Seymour aquifer. Final Contract Rept for the Texas Water Development Board: variably paginated.
23 24 25	GE Infrastructure Water & Process Technologies. 2004. Advances and Changing Costs in Reverse Osmosis and Ion Exchange Systems. www.gewater.com/library/tp/733_Advances_and.jsp.
26 27 28	Harden, R. W. and Associates. 1978. The Seymour Aquifer, ground-water quality and availability in Haskell and Knox counties, Texas. Texas Dept. Water Resources Rept. 226 1:261.
29 30 31	Hartsough, P., S. W. Tyler, J. Sterling, and M. A. Walvoord. 2001. A 14.6 kyr record of nitrogen flux from desert soil profiles as inferred from vadose zone pore waters. Geophys. Res. Lett. 28:2955-2958.
32 33	Kempic, J. and Khera, R. 2003. Point-of-Use/Point-of-Entry Devices Cost Considerations. Powerpoint® Presentation at POU/POE Treatment Strategies Workshop. EPA.
34 35	Kommineni, S., et al. 2003. Point-of-Use/Point-of-Entry Treatment for Arsenic Removal: Operational Issues and Costs. Malcolm Pirnie, Inc. Phoenix, Arizona.

- Nativ, R., and G. N. Gutierrez. 1988. Hydrogeology and hydrochemistry of Cretaceous aquifers, Texas Panhandle and Eastern New Mexico. The University of Texas at Austin, Bureau of Economic Geology Geological Circular 88-3:32 p.
- 4 Raucher, Robert S., et al. 2004. *Conventional and Unconventional Approaches to Water Service*5 *Provision.* AWWA Research Foundation and American Water Works Association.
- 6 Remco Engineering Water Systems and Controls. 2004. *Ion Exchange*. www.remco.com.ix.htm.
- Stonestrom, D. A., D. E. Prudic, R. J. Laczniak, and K. C. Akstin. 2004. Tectonic, climatic, and land-use controls on groundwater recharge in an arid alluvial basin: Amargosa Desert, U.S.A. in F. M. Phillips, J. F. Hogan, and B. R. Scanlon, editors. Groundwater Recharge in a Desert Environment: The Southwestern United States, edited by J.F. Hogan, F.M.
- Phillips, and B.R. Scanlon, Water Science and Applications Series, vol. 9, American
- Geophysical Union, Washington, D.C., 29-47.
- TCEQ. 2004a. Drinking Water Quality and Reporting Requirements for PWSs: 30 TAC 290 Subchapter F (290.104. Summary of Maximum Contaminant Levels, Maximum Residual Disinfectant Levels, Treatment Techniques, and Action Levels). Revised February 2004.
- TCEQ. 2004b. How to Conduct Radionuclide Testing for Well Completion Interim Approval.
 Revised July 1, 2004.
- 18 TNRCC. 2002. State of Texas Environment Strategic Plan Fiscal Years 2003-2007. Volume 2.
- U.S. Department of the Interior, Bureau of Reclamation, Technical Services Center. 1999. Water
 Treatment Estimation Routine (WaTER) User Manual. Water Desalination Research &
 Development Program Report No. 43. R-99-04. Denver, Colorado.
- USEPA. 1980. Innovative and Alternative Technology Assessment Manual. EPA 430/9-78-009.
- 24 USEPA. 1983. Nitrate Removal for Small Public Water Systems. EPA 570/9-83-009.
- USEPA. 1992. Standardized Costs for Water Supply Distribution Systems. EPA/600/R-92/009.
- USEPA. 1998. Information for States on Developing Affordability Criteria for Drinking Water.
 EPA 816-R-98-002.
- USEPA. 2001. National Primary Drinking Water Regulations; Arsenic Contaminants Monitoring. Final Rule. *Federal Register*: January 22, 2001 (Volume 66, Number 14, p. 6975-7066).
- 31 USEPA. 2004. Capital Costs of Arsenic Removal Technologies, U.S. EPA Arsenic Removal Technology Demonstration Program Round 1. EPA 600/R-04/201.
- USEPA. 2005a. List of Drinking Water Contaminants & MCLs. Online. Last updated February 23, 2005. www.epa.gov/safewater/mcl.html.
- USEPA. 2005b. Consumer Factsheet on: NITRATES/NITRITES. Online. Last updated February 14, 2005. www.testproducts.com/water/consumer_fact_sheet_on_nitrates-nitrites.htm.

2	USEPA. 2005c. Technical Fact Sheet: Final Rule for Arsenic in Drinking Water. EPA 815-F-00-016. Online. Last updated February 14, 2005 www.epa.gov/safewater/ars/ars_rule_techfactsheet.html.
1	Walvoord, M. A., F. M. Phillips, D. A. Stonestrom, E. C. Evans, P. C. Hartsough, B. D. Newman, and R. G. Striegl. 2003. A reservoir of nitrate beneath desert soils. Science 302:5647.
б 7	White, D. E. 1971. Water resources of Ward County, Texas. Texas Water Development Board Report 125, 235 p.

1 APPENDIX A 2 PWS INTERVIEW FORM

CAPACITY DEVELOPMENT ASSESSMENT FORM

Prepared By	
Section 1. Public Water System	Information
1. PWS ID # 2. W	Vater System Name
3. County	
4. Owner	Address
Tele.	E-mail
Fax	Message
5. Admin	Address
Tele.	E-mail
Fax	Message
6. Operator	Address
Tele.	E-mail
Fax	Message
7. Population Served	8. No. of Service Connections
9. Ownership Type	10. Metered (Yes or No)
11. Source Type	
12. Total PWS Annual Water Used	
13. Number of Water Quality Violations (Pri	ior 36 months)
Total Coliform	Chemical/Radiological
Monitoring (CCR, Public Notification	on, etc.) Treatment Technique, D/DBP

A. Basic Information

Name of Water System:

7b. How long have you been certified?

Describe your water system related duties on a typical day.

1.

8.

2.	Name of Person Interviewed:
3.	Position:
4.	Number of years at job:
5.	Number of years experience with drinking water systems:
6.	Percent of time (day or week) on drinking water system activities, with current position (how much time is dedicated exclusively to the water system, not wastewater, solid waste or other activities):
7.	Certified Water Operator (Yes or No):
	If Yes, 7a. Certification Level (water):

B. Organization and Structure

1. Describe the organizational structure of the Utility. Please provide an organizational chart. (Looking to find out the governance structure (who reports to whom), whether or not there is a utility board, if the water system answers to public works or city council, etc.)

3.	Do all of the positions have a written job description?
	3a. If yes, is it available to employees?
	3b. May we see a copy?
	C. Personnel
1.	What is the current staffing level (include all personnel who spend more than 10% of their time working on the water system)?
2.	Are there any vacant positions? How long have the positions been vacant?
3.	In your opinion, is the current staffing level adequate? If not adequate, what are the issues or staffing needs (how many and what positions)?
4.	What is the rate of employee turnover for management and operators? What are the major issues involved in the turnover (e.g., operator pay, working conditions, hours)?
5.	Is the system staffed 24 hours a day? How is this handled (on-site or on-call)? Is there an alarm system to call an operator if an emergency occurs after hours?

If not already covered in Question 1, to whom do you report?

2.

D. Communication

1.	Does the utility have a mission statement? If yes, what is it?
2.	Does the utility have water quality goals? What are they?
3.	How are your work priorities set?
4.	How are work tasks delegated to staff?
5.	Does the utility have regular staff meetings? How often? Who attends?
6.	Are there separate management meetings? If so, describe.
7.	Do management personnel ever visit the treatment facility? If yes, how often?
8.	Is there effective communication between utility management and state regulators (e.g., NMED)?
9.	Describe communication between utility and customers.

E. Planning and Funding

1.	Describe the rate structure for the utility.
2.	Is there a written rate structure, such as a rate ordinance? May we see it?
	2a. What is the average rate for 6,000 gallons of water?
3.	How often are the rates reviewed?
4.	What process is used to set or revise the rates?
5.	In general, how often are the new rates set?
6.	Is there an operating budget for the water utility? Is it separate from other activities, such as wastewater, other utilities, or general city funds?
7.	Who develops the budget, how is it developed and how often is a new budget created or the old budget updated?
8.	How is the budget approved or adopted?

9.	In the last 5 years, how many budget shortfalls have there been (i.e., didn't collect enough money to cover expenses)? What caused the shortfall (e.g., unpaid bills, an emergency repair, weather conditions)?
	9a. How are budget shortfalls handled?
10.	In the last 5 years how many years have there been budget surpluses (i.e., collected revenues exceeded expenses?
	10a. How are budget surpluses handled (i.e., what is done with the money)?
11.	Does the utility have a line-item in the budget for emergencies or some kind of emergency reserve account?
12.	How do you plan and pay for short-term system needs?
13.	How do you plan and pay for long- term system needs?
14.	How are major water system capital improvements funded? Does the utility have a written capital improvements plan?
15.	How is the facility planning for future growth (either new hook-ups or expansion into new areas)?
16.	Does the utility have and maintain an annual financial report? Is it presented to policy makers?

17.	Has an independent financial audit been conducted of the utility finances? If so, how often? When was the last one?
18.	Will the system consider any type of regionalization with any other PWS, such as system interconnection, purchasing water, sharing operator, emergency water connection, sharing bookkeeper/billing or other?
	F. Policies, Procedures, and Programs
1.	Are there written operational procedures? Do the employees use them?
2.	Who in the utility department has spending authorization? What is the process for obtaining needed equipment or supplies, including who approves expenditures?
3.	Does the utility have a source water protection program? What are the major components of the program?
4.	Are managers and operators familiar with current SDWA regulations?
5.	How do the managers and operators hear about new or proposed regulations, such as arsenic, DBP, Groundwater Rule? Are there any new regulations that will be of particular concern to the utility?
6.	What are the typical customer complaints that the utility receives?
7.	Approximately how many complaints are there per month?

8.	How are customer complaints handled? Are they recorded?
9.	(If not specifically addressed in Question 7) If the complaint is of a water quality nature, how are these types of complaints handled?
10.	Does the utility maintain an updated list of critical customers?
11.	Is there a cross-connection control plan for the utility? Is it written? Who enforces the plan's requirements?
12.	Does the utility have a written water conservation plan?
13.	Has there been a water audit of the system? If yes, what were the results?
14.	(If not specifically answered in 11 above) What is the estimated percentage for loss to leakage for the system?
15.	Are you, or is the utility itself, a member of any trade organizations, such as AWWA or Rural Water Association? Are you an active member (i.e., attend regular meetings or participate in a leadership role)? Do you find this membership helpful? If yes, in what ways does it help you?

G. Operations and Maintenance

1.

How is decision-making authority split between operations and management for the following items:

	a.	Process Control
	b.	Purchases of supplies or small equipment
	c.	Compliance sampling/reporting
	d.	Staff scheduling
2.	Describe your	utility's preventative maintenance program.
3.	Do the operate	ors have the ability to make changes or modify the preventative maintenance program?
4.		nagement prioritize the repair or replacement of utility assets? Do the operators play a role zation process?
5.	Does the utilit	y keep an inventory of spare parts?
6.	Where does st	aff have to go to buy supplies/minor equipment? How often?
	examp	w do you handle supplies that are critical, but not in close proximity (for le if chlorine is not available in the immediate area or if the components for a critical are not in the area)

7.	Describe the system's disinfection process. Have you had any problems in the last few years with the disinfection system?
	7a. Who has the ability to adjust the disinfection process?
8. Ho	w often is the disinfectant residual checked and where is it checked? 8a. Is there an official policy on checking residuals or is it up to the operators?
9.	Does the utility have an O & M manual? Does the staff use it?
10.	Are the operators trained on safety issues? How are they trained and how often?
11.	Describe how on-going training is handled for operators and other staff. How do you hear about appropriate trainings? Who suggests the trainings – the managers or the operators? How often do operators, managers, or other staff go to training? Who are the typical trainers used and where are the trainings usually held?
12.	In your opinion is the level of your on-going training adequate?
13.	In your opinion is the level of on-going training for other staff members, particularly the operators, adequate?

14.	Does the facility have mapping of the water utility components? Is it used on any routine basis by the operators or management? If so, how is it used? If not, what is the process used for locating utility components?
15.	In the last sanitary survey, were any deficiencies noted? If yes, were they corrected?
16.	How often are storage tanks inspected? Who does the inspection?
	16a. Have you experienced any problems with the storage tanks?
	H. SDWA Compliance
1.	Has the system had any violations (monitoring or MCL) in the past 3 years? If so, describe.
2.	How were the violations handled?
3.	Does the system properly publish public notifications when notified of a violation?
4.	Is the system currently in violation of any SDWA or state regulatory requirements, including failure to pay fees, fines, or other administrative type requirements?
5.	Does the utility prepare and distribute a Consumer Confidence Report (CCR)? Is it done every year? What type of response does the utility get to the CCR from customers?

I. Emergency Planning

1.	Does the system have a written emergency plan to handle emergencies such as water outages, weather issues, loss of power, loss of major equipment, etc?
2.	When was the last time the plan was updated?

4. Describe the last emergency the facility faced and how it was handled.

Do all employees know where the plan is? Do they follow it?

3.

Attachment A

A. Technical Capacity Assessment Questions

1.	Based on available information of water rights in the past year? YES	rights o	on record and NO	water pun	nped has	s the system exceeded its wate		
	In any of the past 5 years? YES		NO		How	many times?		
2.	Does the system have the proper level o	f certific	ed operator? (Use quest	tions a –	- c to answer.)		
	a. What is the Classification Level of the	ie systei	n by NMED?					
	b. Does the system have one or more ce	ertified o	operator(s)?	[20 NMA	C 7.4.20	0]		
	YES NO							
	c. If YES, provide the number of operation	tors at e	ach New Mex	ico Certif	ication I	Level. [20 NMAC 7.4.12]		
	NM Small System			Class 2				
	NM Small System Advan	iced		Class 3				
	Class 1			Class 4				
3.	Did the system correct any sanitary defi-	Did the system correct any sanitary deficiency noted on the most recent sanitary survey within 6 months of						
	receiving that information? [20 NMAC	7.20.50	04]					
	YES NO		No I	Deficienci	es			
	What was the type of deficiency? (Chec	ck all th	at are applica	ble.)				
	Source		Storage					
	Treatment		Distribution					
	Other							
	From the system's perspective, were the	ere any o	other deficienc	cies that w	ere not	noted on the sanitary survey?		
	Please describe.							
4.	Will the system's current treatment proc	cess mee	et known futur	re regulati	ons?			
	Radionuclides	YES		NO		Doesn't Apply		
	Arsenic	YES		NO		Doesn't Apply		
	Stage 1 Disinfectants and Disinf	fection 1	By-Product (E	OBP)				
		YES		NO		Doesn't Apply		
	Surface Water Treatment Rule	YES		NO		Doesn't Apply		
5.	Does the system have a current site plan	/map?	[20 NMAC 7.	10.302 A.	1.]	•		
	YES NO							

o. Has	s me system	i nau a wate	r suppry outag	e in the prior 24 month	1S ?	
	YES		NO			
	What	were the cau	ses of the outa	age(s)? (Include numbe	er of outo	ages for each cause.)
	Droug	ht		Limited Supply	-	
	System	n Failure		Other	-	
7. Has	s the system	n ever had a	water audit or	a leak evaluation?		
	YES		NO	Don't Know		
	If YES	S, please con	nplete the follo	owing table.		
Type of		Date	Water Loss	What approach or		Was any follow-up done? If
Investigation	on	Done	(%)	technology was used	to	so, describe
				complete the investig	ation?	
					10	
8. Hav	ve all drink YES	ing water pro	ojects received NO	l NMED review and ap	oproval?	[20 NMAC 7.10.201]
	If NO.	, what types	of projects hav	— ve not received NMED	review a	and approval.
	Source]	Storage		••
	Treatn	nent]	Distribution		
	Other					
9. Wh	nat are the ty	ypical custor	ner complaint	s that the utility receive	es?	
10. App	proximately	y how many	complaints are	e there per month?		
11. Ho	w are custo	mer complai	ints handled?	Are they recorded?		
11. 110	w are custo	mer compia	ints nandica:	The they recorded:		

Sanitary Survey Distribution Sys Attached Are there any dead end lines in the system? YES	
Does the system have a flushing program? YES	tem Record
Does the system have a flushing program? YES	
Does the system have a flushing program? YES	
If YES, please describe. Are there any pressure problems within the system? YES	
If YES, please describe. Are there any pressure problems within the system? YES	
Are there any pressure problems within the system? YES	
Types NO	
If YES, please describe. Does the system disinfect the finished water? YES NO Siff yes, which disinfectant product is used? Ewer Comments on Technical Capacity: Managerial Capacity Assessment Questions Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan? YES NO Siff YES, has the plan been submitted to Local Government Division? YES NO SIFF YES YES NO SIFF YES YES NO SIFF YES	
Does the system disinfect the finished water? YES	
YES NO	
If yes, which disinfectant product is used? ewer Comments on Technical Capacity: Managerial Capacity Assessment Questions Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan? YES NO If YES, has the plan been submitted to Local Government Division? YES NO	
Managerial Capacity Assessment Questions Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan? YES NO If YES, has the plan been submitted to Local Government Division? YES NO NO NO NO NO NO NO NO	
Managerial Capacity Assessment Questions Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan? YES NO STATE NO S	
Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan? YES NO STATE S	
YES NO STATE	
If YES, has the plan been submitted to Local Government Division? YES NO	
YES NO	
Does the system have written operating procedures?	
YES NO Does the system have written job descriptions for all staff?	

NO

YES

20.	Does the system have:
	A preventative maintenance plan? YES NO
	A source water protection plan? YES NO N/A
	An emergency plan?
	YES NO A cross-connection control program?
	YES NO
	An emergency source?
	YES NO
	System security measures?
	YES L NO L
21.	Does the system report and maintain records in accordance with the drinking water regulations concerning: Water quality violations
	YES NO
	Public notification YES NO
	Sampling exemptions YES NO
22.	Please describe how the above records are maintained:
23.	Describe the management structure for the water system, including board and operations staff. Please include examples of duties, if possible.
24.	Please describe type and quantity of training or continuing education for staff identified above.
25.	Describe last major project undertaken by the water system, including the following: project in detail, positive aspects, negative aspects, the way in which the project was funded, any necessary rate increases, the public response to the project, whether the project is complete or not, and any other pertinent information.

26.	Does the system have any debt? YES NO
	If yes, is the system current with all debt payments? YES NO
	If no, describe the applicable funding agency and the default.
27.	Is the system currently contemplating or actively seeking funding for any project? YES NO
	If yes, from which agency and how much?
	Describe the project?
	Is the system receiving assistance from any agency or organization in its efforts?
28.	Will the system consider any type of regionalization with other PWS? (Check YES if the system has already regionalized.) YES \Boxedot NO \Boxedot \Boxedot
	If YES, what type of regionalization has been implemented/considered/discussed? (Check all that apply.)
	System interconnection
	Sharing operator
	Sharing bookkeeper
	Purchasing water
	Emergency water connection
	Other:
29.	Does the system have any of the following? (Check all that apply.)
	Water Conservation Policy/Ordinance Current Drought Plan
	Water Use Restrictions
Inte	rviewer Comments on Managerial Capacity:

<u>C.</u>	Financial Capacity Assessment
30.	Does the system have a budget?
	YES NO
	If YES, what type of budget?
	Operating Budget
	Capital Budget
31.	Have the system revenues covered expenses and debt service for the past 5 years?
	YES NO
	If NO, how many years has the system had a shortfall?
32.	Does the system have a written/adopted rate structure?
	YES NO
33.	What was the date of the last rate increase?
34.	Are rates reviewed annually?
	YES NO
	IF YES, what was the date of the last review?
35.	Did the rate review show that the rates covered the following expenses? (Check all that apply.)
	Operation & Maintenance
	Infrastructure Repair & replacement
	Staffing
	Emergency/Reserve fund
	Debt payment
36.	Is the rate collection above 90% of the customers?
	YES NO
37.	Is there a cut-off policy for customers who are in arrears with their bill or for illegal connections?
	YES NO
	If yes, is this policy implemented?
38.	What is the residential water rate for 6,000 gallons of usage in one month
39.	In the past 12 months, how many customers have had accounts frozen or dropped for non-payment?
	[Convert to % of active connections
	Less than 1%
	11% - 20%

40.	The following questions refer to the process of obtaining needed equipment and supplies.
	a. Can the water system operator buy or obtain supplies or equipment when they are needed?
	YES NO
	b. Is the process simple or burdensome to the employees?
	c. Can supplies or equipment be obtained quickly during an emergency?
	YES NO
	d. Has the water system operator ever experienced a situation in which he/she couldn't purchase the needed supplies?
	YES NO
	e. Does the system maintain some type of spare parts inventory?
	YES NO
	If yes, please describe.
	ii yes, pieuse describe.
41.	Has the system ever had a financial audit? YES NO I If YES, what is the date of the most recent audit?
42.	Has the system ever had its electricity or phone turned off due to non-payment? Please describe.
In	nterviewer Comments on Financial Assessment:

43.	What do you think the system capabilities are now and what are the issues you feel your system will be
	facing in the future? In addition, are there any specific needs, such as types of training that you would
	like to see addressed by NMED or its contractors?

APPENDIX B COST BASIS

This section presents the basis for unit costs used to develop the conceptual cost estimates for the compliance alternatives. Cost estimates are conceptual in nature (+50%/-30%), and are intended to make comparisons between compliance options and to provide a preliminary indication of possible rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. Capital cost includes an allowance for engineering and construction management. It is assumed that adequate electrical power is available near the site. The cost estimates specifically do not include costs for the following:

- Obtaining land or easements.
- Surveying.

- Mobilization/demobilization for construction.
- Insurance and bonds.

In general, unit costs are based on recent construction bids for similar work in the area; when possible, consultations with vendors or other suppliers; published construction and O&M cost data; and USEPA cost guidance. Unit costs used for the cost estimates are summarized in Table B.1.

Unit costs for pipeline components are based on recent bids on Texas Department of Highways projects. The amounts of boring and encasement and open cut and encasement were estimated by counting the road, highway, railroad, stream, and river crossings for a conceptual routing of the pipeline. The number of air release valves is estimated by examining the land surface profile along the conceptual pipeline route. It is assumed gate valves and flush valves would be installed on average every 5,000 feet along the pipeline. Pipeline cost estimates are based on use of C-900 PVC pipe. Other pipe materials could be considered for more detailed development of attractive alternatives.

Pump station unit costs are based on experience with similar installations. The cost estimate for the pump stations include two pumps, station piping and valves, station electrical and instrumentation, minor site improvement, installation of a concrete pad and building, and tools. Construction cost of a storage tank is based on similar recent installations.

Electrical power cost is estimated to be \$0.128 per kWH. The annual cost for power to a pump station is calculated based on the pumping head and volume, and includes 11,800 kWH for pump building heating, cooling, and lighting, as recommended in USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992).

In addition to the cost of electricity, pump stations have other maintenance costs. These costs cover: materials for minor repairs to keep the pumps operating; purchase of a maintenance vehicle, fuel costs, and vehicle maintenance costs; utilities; office

1 2

supplies, small tools and equipment; and miscellaneous materials such as safety, clothing, chemicals, and paint. The non-power O&M costs are estimated based on the USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992), which provides cost curves for O&M components. Costs from the 1992 report are adjusted to 2005 dollars based on the ENR construction cost index.

Pipeline maintenance costs include routine cleaning and flushing, as well as minor repairs to lines. The unit rate for pipeline maintenance is calculated based on the USEPA technical report, *Innovative and Alternate Technology Assessment Manual MCD 53* (1978). Costs from the 1978 report are adjusted to 2005 dollars based on the ENR construction cost index.

Storage tank maintenance costs include cleaning and renewal of interior lining and exterior coating. Unit costs for storage tank O&M are based on USEPA publication *Standardized Costs for Water Supply Distribution Systems* (1992). Costs from the 1992 report are adjusted to 2005 dollars based on the ENR construction cost index.

The purchase price for point-of-use (POU) water treatment units is based on vendor price lists for treatment units, plus installation. O&M costs for POU treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

The purchase price for point-of-entry (POE) water treatment units is based on vendor price lists for treatment units, plus an allowance for installation, including a concrete pad and shed, piping modifications, and electrical connection. O&M costs for POE treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

Central treatment plant costs, for both adsorption and coagulation/filtration, include pricing for buildings, utilities, and site work. Costs are based on pricing given in the various R.S. Means Construction Cost Data References, as well as prices obtained from similar work on other projects. Pricing for treatment equipment was obtained from vendors.

Well installation costs are based on quotations from drillers for installation of similar depth wells in the area. Well installation costs include drilling, a well pump, electrical and instrumentation installation, well finishing, piping, and water quality testing. O&M costs for water wells include power, materials, and labor. It is assumed that new wells located more than 1 mile from the intake point of an existing system would require a storage tank and pump station.

Purchase price for the treatment unit dispenser is based on vendor price lists, plus an allowance for installation at a centralized public location. The O&M costs are also based on vendor price lists. It is assumed that weekly water samples would be analyzed for the contaminant of concern.

1

2

3

4

5

6

7

8

Costs for bottled water delivery alternatives are based on consultation with vendors that deliver residential bottled water. The cost estimate includes an initial allowance for set-up of the program, and a yearly allowance for program administration.

The cost estimate for a public dispenser for trucked water includes the purchase price for a water truck and construction of a storage tank. Annual costs include labor for purchasing the water, picking up and delivering the water, truck maintenance, and water sampling and testing. It is assumed the water truck would be required to make one trip each week, and that chlorine residual would be determined for each truck load.

Table B.1

Summary of General Data South Midland County Water Systems Inc. PWS #1650077

General PWS Information

Service Population 256
Total PWS Daily Water Usage 0.024 (mgd)

Number of Connections 75 Source 2005 Report

Unit Cost Data West Texas

General Items	Unit	Unit Cost	Central Treatment Unit Costs	Unit	Unit Cost
Treated water purchase cost	See alte	rnative	Site preparation	acre	\$ 4,000
Water purchase cost (trucked)	\$/1,000 gals	\$ 1.80	Slab	CY	\$ 1,000
. ,	. , ,		Building	SF	\$ 60
Contingency	20%	n/a	Building electrical	SF	\$ 8.00
Engineering & Constr. Management	25%	n/a	Building plumbing	SF	\$ 8.00
Procurement/admin (POU/POE)	20%	n/a	Heating and ventilation	SF	\$ 7.00
,			Fence	LF	\$ 15
Pipeline Unit Costs	Unit	Unit Cost	Paving	SF	\$ 2.00
PVC water line, Class 200, 04"	LF	\$ 26	Electrical, RO	JOB	\$ 50,000
Bore and encasement, 10"	LF	\$ 60	Electrical, EDR	JOB	\$ 50,000
Open cut and encasement, 10"	LF	\$ 30	Piping, RO	JOB	\$ 20,000
Gate valve and box, 04"	EA	\$ 340	Piping, EDR	JOB	\$ 20,000
Air valve	EA	\$ 1,000	RO package	UNIT	\$ 125,000
Flush valve	EA	\$ 750	IX package	UNIT	\$ 279,000
Metal detectable tape	LF	\$ 0.15	Transfer pumps (5 hp)	EA	\$ 5,000
Wetar detectable tape		φ 0.10	Permeate Tank	GAL	\$ 3.00
Bore and encasement, length	Feet	200	Backwash tank	GAL	\$ 2.00
Open cut and encasement, length	Feet	50	Mixer on tank	EA	\$ 15,000
Open cut and encasement, length	1 661	30	Salt feeder	EA	\$ 20,000
Pump Station Unit Costs	Unit	Unit Cost	Tank, 20,000 GAL	GAL	\$ 1.00
•					
Pump	EA	\$ 7,500	Tank, 10,000 GAL	GAL	
Pump Station Piping, 04"	EA	\$ 4,000	Excavation	CYD	\$ 3.00 \$ 7.00
Gate valve, 04"	EA	\$ 370	Compacted fill	CYD	
Check valve, 04"	EA	\$ 430	Lining	SF	\$ 0.50
Electrical/Instrumentation	EA	\$ 10,000	Vegetation	SY	\$ 1.00
Site work	EA	\$ 2,000	Access road	LF	\$ 30
Building pad	EA	\$ 4,000	Reject water haul truck	EA	\$ 100,000
Pump Building	EA	\$ 10,000			
Fence	EA	\$ 5,870	Building Power	kwh/yr	\$ 0.128
Tools	EA	\$ 1,000	Equipment power	kwh/yr	\$ 0.128
			Labor	hr	\$ 40
Well Installation Unit Costs	Unit	Unit Cost	RO Materials	year	\$ 5,000
Well installation	See alte		EDR Materials	year	\$ 5,000
Water quality testing	EA	\$ 1,500	Chemicals, RO	year	\$ 1,800
Well pump	EA	\$ 7,500	Chemicals, EDR	year	\$ 1,800
Well electrical/instrumentation	EA	\$ 5,000	Analyses	test	\$ 200
Well cover and base	EA	\$ 3,000	Haul reject water	miles	1.0
Piping	EA	\$ 2,500	Truck rental	day	\$ 700
Storage Tank - 5,000 gals	EA	\$ 7,025	Mileage	mile	\$ 1.00
			Disposal fee	kgal	\$ 5.00
Electrical Power	\$/kWH	\$ 0.128			
Building Power	kWH	11,800			
Labor	\$/hr	\$ 30			
Materials	EA	\$ 1,200			
Transmission main O&M	\$/mile	\$ 200			
Tank O&M	EA	\$ 1,000			
POU/POE Unit Costs					
POU treatment unit purchase	EA	\$ 250			
POU treatment unit installation	EA	\$ 150			
POE treatment unit purchase	EA	\$ 3,000			
POE - pad and shed, per unit	EA	\$ 2,000			
POE - piping connection, per unit	EA	\$ 1,000			
POE - electrical hook-up, per unit	EA	\$ 1,000			
POU treatment O&M, per unit	\$/year	\$ 225			
POE treatment O&M, per unit	\$/year	\$ 1,000			
Contaminant analysis	\$/year	\$ 100			
POU/POE labor support	\$/hr	\$ 30			
	•	*			
Dispenser/Bottled Water Unit Costs					
Treatment unit purchase	EA	\$ 3,000			
Treatment unit installation	EA	\$ 5,000			
Treatment unit 0&M	EA	\$ 500			
Administrative labor	hr	\$ 40			
Bottled water cost (inc. delivery)	gallon	\$ 1.60			
Water use, per capita per day	gpcd	1.00			
Bottled water program materials	EA	\$ 5,000			
Storage Tank - 5,000 gals	EA	\$ 7,025			
Site improvements	EA	\$ 4,000			
Potable water truck	EA	\$ 60,000			
Water analysis, per sample	EA	\$ 60,000			
Potable water truck O&M costs	\$/mile	\$ 1.00			
. Stable water truck Odivi 60515	ψ/IIIIE	ψ 1.00			

1

2

3

4

6 7

8

APPENDIX C COMPLIANCE ALTERNATIVE CONCEPTUAL COST ESTIMATES

This appendix presents the conceptual cost estimates developed for the compliance alternatives. The conceptual cost estimates are given in Tables C.1 through C.14. The cost estimates are conceptual in nature +50%/-30%), and are intended for making comparisons between compliance options and to provide a preliminary indication of possible water rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation.

PWS Name South Midland County Water Systems Inc.
Alternative Name Purchase Water from City of Midland

Alternative Number SMC-1

Distance from Alternative to PWS (along pipe)2.0milesTotal PWS annual water usage8.760MG

Treated water purchase cost \$ 1.65 per 1,000 gals

Number of Pump Stations Needed

Capital Costs

		Annual Operations and maintenance code											
Cost Item Pipeline Construction	Quantity	Unit	Unit	Cost	To	otal Cost	Cost Item Pipeline O&M	Quantity	Unit	Un	it Cost	Tota	l Cost
Number of Crossings, bore	3	n/a	n/a		n/a		Pipeline O&M	2.0	mile	\$	200	\$	394
Number of Crossings, open cut	10		n/a		n/a		Subtotal	2.0	IIIIIC	Ψ	200	φ \$	394 394
PVC water line, Class 200, 04"	10,403		\$	26.00		270,478	Subtotal					φ	334
Bore and encasement, 10"	600		\$	60.00	\$	36,000	Water Purchase Cost						
Open cut and encasement, 10"	500		\$		\$	15,000	From Source	8 760	1,000 ga	\$	1.65	\$ 1.	4 454
Gate valve and box, 04"		EA	\$	340.00	\$	707	Subtotal	0,700	1,000 gc	Ψ	1.00		4,4 5 4
Air valve	_	EA	*	1,000.00	\$	2.000						Ψ.	.,
Flush valve	_	EA	\$	750.00	\$	1,560							
Metal detectable tape	10,403	LF	\$	0.15	\$	1,560							
Subtotal	,				\$	327,306							
Pump Station(s) Installation							Pump Station(s) O&M						
Pump	1	EA	\$	7,500	\$	7,500	Building Power	11,800	kWH	\$	0.128	\$	1,510
Pump Station Piping, 04"		EA	\$	4,000	\$	4,000	Pump Power	17,050		\$	0.128	\$	2,182
Gate valve, 04"		EA	\$	370	\$	1,480	Materials		EA		1,200		1,200
Check valve, 04"		EA	\$	430	\$	860	Labor		Hrs	\$	30		0,950
Electrical/Instrumentation		EA	\$	10,000	\$	10,000	Tank O&M	1	EA	\$	1,000		1,000
Site work		EA	\$		\$	2,000	Subtotal					\$ 1	6,843
Building pad		EA	\$,	\$	4,000							
Pump Building		EA	\$	- ,	\$	10,000							
Fence		EA	\$	5,870		5,870							
Tools		EA EA	\$ \$	1,000	\$	1,000							
Storage Tank - 5,000 gals Subtotal		EA	Ф	7,025	\$ \$	7,025 53,735							
							O&M Credit for Existing W	ell Closure					
							Pump power	922	kWH	\$	0.128	\$	(118)
							Well O&M matl		EA		1,200		2,400)
							Well O&M labor		Hrs	\$	30		0,800)
							Subtotal			•			3,318)
Subtotal of (Subtotal of Component Costs												
Contingency	20%				\$	76,208							
Design & Constr Management	25%				\$	95,260							
TOTAL CAPITAL COSTS					\$	552,510	TOTAL ANI	NUAL O&N	I COSTS			\$ 1	8,373

PWS Name South Midland County Water Systems Inc.
Alternative Name New Well at Midland International Airport
Alternative Number SMC-2

 Distance from PWS to new well location
 11.31 miles

 Estimated well depth
 300 feet

 Number of wells required
 1

 Well installation cost (location specific)
 \$25 per foot

 Number of pump stations needed
 2

Capital Costs

Capital Costs							Annual Operations a	ina waint	enance (osts			
Cost Item Pipeline Construction	Quantity	Unit	Un	it Cost	T	otal Cost	Cost Item Pipeline O&M	Quantity	Unit	Uni	t Cost	Total (Cost
Number of Crossings, bore	8	n/a	n/a	1	n/a	ì	Pipeline O&M	11.3	3 mile	\$	200	\$ 2,2	262
Number of Crossings, open cut		n/a	n/a		n/a		Subtotal		7 111110	Ψ	200	. ,	262
PVC water line, Class 200, 04"	59,714		\$	26		1,552,564						¥ -,-	
Bore and encasement, 10"	1,600		\$	60	\$	96,000							
Open cut and encasement, 10"	,	LF	\$	30	\$	25,500							
Gate valve and box, 04"		EA	\$	340	\$	4.061							
Air valve		EA	\$	1,000	\$	11,000							
Flush valve		EA	\$	750	\$	8,957							
Metal detectable tape	59.714		\$	0.15	\$	8,957							
Subtota	,		•	00		1,707,039							
Pump Station(s) Installation							Pump Station(s) O&M						
Pump	4	EA	\$	7,500	\$	30,000	Building Power	23,600	kWH	\$	0.128	\$ 3,0	021
Pump Station Piping, 04"	2	EA	\$	4,000	\$	8,000	Pump Power	63,000	kWH	\$	0.128	\$ 8,0	064
Gate valve, 04"	8	EA	\$	370	\$	2,960	Materials	2	EA	\$	1,200	\$ 2,4	400
Check valve, 04"	4	EA	\$	430	\$	1,720	Labor	730	Hrs	\$	30	\$ 21,9	900
Electrical/Instrumentation	2	EA	\$	10,000	\$	20,000	Tank O&M	2	EA	\$	1,000	\$ 2,0	000
Site work	2	EA	\$	2,000	\$	4,000	Subtotal					\$ 37,	385
Building pad		EA	\$	4,000	\$	8,000							
Pump Building		EA	\$	10,000	\$	20,000							
Fence		EA	\$	5,870	\$	11,740							
Tools		EA	\$	1,000	\$	2,000							
Storage Tank - 5,000 gals	_	EA	\$	7,025	\$	14,050							
Subtota	ıl				\$	122,470							
Well Installation							Well O&M						
Well installation		LF	\$	25	\$	7,500	Pump power		kWH	\$	0.128	*	118
Water quality testing		EA	\$	1,500	\$	3,000	Well O&M matl		EA	\$	1,200	. ,	200
Well pump		EA	\$	7,500		7,500	Well O&M labor		Hrs	\$	30	. ,	400
Well electrical/instrumentation	1	EA	\$	5,000	\$	5,000	Subtotal					\$ 6,	718
Well cover and base	1		\$	3,000	\$	3,000							
Piping		EA	\$	2,500	\$	2,500							
Subtota	ıl				\$	28,500							
							O&M Credit for Existing						
							Pump power		kWH	\$	0.128	. ,	118)
							Well O&M matl		EA	\$	1,200	\$ (2,	,
							Well O&M labor		Hrs	\$	30	+ (- /	,
Subtotal	of Compone	ent Cost	s		\$	1,858,009	Subtotal					\$(13,	318)
	•												
Contingency	20%				\$	371,602							
Design & Constr Management	25%	ó			\$	464,502							
тот	AL CAPITA	L COST	S		\$ 2	2,694,113	TOTAL A	ANNUAL C	&M COS	ΓS		\$ 33,0	047

PWS Name South Midland County Water Systems Inc.

Alternative Name New Well at Canyon Dam

Alternative Number SMC-3

 Distance from PWS to new well location
 23.31 miles

 Estimated well depth
 300 feet

 Number of wells required
 1

 Well installation cost (location specific)
 \$25 per foot

 Number of pump stations needed
 3

Capital Costs

Cost Item Pipeline Construction	Quantity	Unit	Un	it Cost	Te	otal Cost	Cost Item Pipeline O&M	Quantity	Unit	Uni	t Cost	Total Cos	t
Number of Crossings, bore	15	n/a	n/a		n/a		Pipeline O&M	22.2	mile	\$	200	\$ 4,662	
Number of Crossings, pore cut		n/a	n/a		n/a		Subtota		TIME	Ψ	200	\$ 4.662	
PVC water line, Class 200, 04"	123,087		\$	26		3,200,262	Oubtota					Ψ 4,002	
Bore and encasement, 10"	3,000		\$	60	\$	180,000							
Open cut and encasement, 10"	1,800		\$	30	\$	54,000							
Gate valve and box, 04"	,	EA	\$	340	\$	8,370							
Air valve		EA	\$	1,000	\$	23,000							
Flush valve		EA	\$	750	Ф \$	18,463							
			\$	0.15	\$	18,463							
Metal detectable tape Subtotal	123,087	LF	Ф	0.15		3,502,558							
Pump Station(s) Installation							Pump Station(s) O&M						
Pump	6	EA	\$	7,500	\$	45,000	Building Power	35.400	kWH	\$	0.128	\$ 4,531	
Pump Station Piping, 04"	3	EA	\$	4,000	\$	12,000	Pump Power	126,000	kWH	\$	0.128	\$ 16,128	
Gate valve, 04"		EA	\$	370	\$	4,440	Materials		EA	\$	1,200	\$ 3,600	
Check valve, 04"	6	EA	\$	430	\$	2,580	Labor	1,095	Hrs	\$	30	\$ 32,850	
Electrical/Instrumentation	3	EA	\$	10,000	\$	30,000	Tank O&M	,	EA	\$	1.000	\$ 3,000	
Site work	3	EA	\$	2,000	\$	6,000	Subtota			•	,	\$ 60,109	
Building pad	3	EA	\$	4,000	\$	12,000							
Pump Building		EA		10,000	\$	30,000							
Fence	3	EA	\$	5,870	\$	17,610							
Tools	3	EA	\$	1,000	\$	3,000							
Storage Tank - 5,000 gals	3	EA	\$	7,025	\$	21,075							
Subtotal					\$	183,705							
Well Installation							Well O&M						
Well installation	300	LF	\$	25	\$	7,500	Pump power	922	kWH	\$	0.128	\$ 118	
Water quality testing	2	EA	\$	1,500	\$	3,000	Well O&M matl	1	EA	\$	1,200	\$ 1,200	
Well pump	1	EA	\$	7,500	\$	7,500	Well O&M labor	180	Hrs	\$	30	\$ 5,400	
Well electrical/instrumentation	1	EA	\$	5,000	\$	5,000	Subtota					\$ 6,718	
Well cover and base	1	EA	\$	3,000	\$	3,000							
Piping	1	EA	\$	2,500	\$	2,500							
Subtotal					\$	28,500							
							O&M Credit for Existing						
							Pump power	922	kWH	\$	0.128	\$ (118))
							Well O&M matl	2	EA	\$	1,200	\$ (2,400))
							Well O&M labor	360	Hrs	\$	30	\$(10,800))
							Subtota					\$(13,318))
Subtotal of	Componen	t Costs	3		\$	3,714,763							
Contingency	20%				\$	742,953							
Design & Constr Management	25%				\$	928,691							
TOTAL	CAPITAL	COSTS	6		\$	5,386,406	TOTAL	ANNUAL O	&M COSTS			\$ 58,172]

PWS Name South Midland County Water Systems Inc.
Alternative Name Purchase Water from City of Odessa

Alternative Number SMC-4

Distance from Alternative to PWS (along pipe)21.4milesTotal PWS annual water usage8.760MG

Treated water purchase cost \$ 1.60 per 1,000 gals

Number of Pump Stations Needed 3

Capital Costs

Cost Item	Quantity	Unit	Uni	it Cost	Т	otal Cost	Cost Item	Quantity	Unit	Un	it Cost	Total Cos
Pipeline Construction	40	/	/		/ -		Pipeline O&M	04.4		•	000	Ф 400 г
Number of Crossings, bore		n/a	n/a		n/a		Pipeline O&M	21.4	mile	\$	200	\$ 4,285
Number of Crossings, open cut		n/a	n/a		n/a		Subtotal					\$ 4,285
PVC water line, Class 200, 04"	113,137		\$	26.00		2,941,562						
Bore and encasement, 10"	2,000		\$	60.00	\$	120,000	Water Purchase Cost	0.700	4 000 1	•	4.00	C 44 040
Open cut and encasement, 10"	2,150		\$	30.00	\$	64,500	From Source	8,760	1,000 gal	\$	1.60	\$ 14,016
Gate valve and box, 04"		EA	\$	340.00	\$	7,693	Subtotal					\$ 14,016
Air valve	21			1,000.00	\$	21,000						
Flush valve		EA	\$ \$	750.00	\$	16,971						
Metal detectable tape Subtotal	113,137	LF	Ф	0.15	\$	16,971 3,188,696						
Subtotal					Φ	3,100,090						
Pump Station(s) Installation							Pump Station(s) O&M	1				
Pump	3	EA	\$	7,500	\$	22,500	Building Power	35,400	kWH	\$	0.128	\$ 4,531
Pump Station Piping, 04"	3	EA	\$	4,000	\$	12,000	Pump Power	112,850		\$	0.128	\$ 14,445
Gate valve, 04"	12	EA	\$	370	\$	4,440	Materials		EA	\$	1,200	\$ 3,600
Check valve, 04"	6		\$	430	\$	2,580	Labor	1,095		\$	30	\$ 32,850
Electrical/Instrumentation		EA	\$	10,000	\$	30,000	Tank O&M		EA	\$	1,000	\$ 3,000
Site work		EA	\$	2,000	\$	6,000	Subtotal					\$ 58,426
Building pad		EA	\$	4,000	\$	12,000						
Pump Building		EA	\$	10,000	\$	30,000						
Fence		EA	\$	5,870	\$	17,610						
Tools		EA	\$	1,000	\$	3,000						
Storage Tank - 5,000 gals		EA	\$	7,025	\$	21,075						
Subtotal					\$	161,205						
							O&M Credit for Existin	ng Well Clo	sure			
							Pump power	922	kWH	\$	0.128	\$ (118
							Well O&M matl	2	EA	\$	1,200	\$ (2,400
							Well O&M labor	360	Hrs	\$	30	\$(10,800
							Subtotal					\$(13,318
Subtotal	sts		\$	3,349,901								
Continuos	000				۴	000 000						
Contingency	20%				\$	669,980						
Design & Constr Management	25%	0			\$	837,475						
TO.	TAL CAPIT	AL COS	TS		\$	4,857,357	TOTAL A	NNUAL O	&M COSTS	3		\$ 63,409

Table C.5

PWS Name South Midland County Water Systems Inc.

Alternative Name Central Treatment - RO

Alternative Number SMC-5

Capital Costs

Cost Item Central-RO	Quantity	Unit	Un	it Cost	To	otal Cost	Cost Item O&M	Quantity	Unit	Un	it Cost	Total Cost
Site preparation	0.5	acre	\$	4,000	\$	2,000	Building Power	7,500	kwh/yr	\$	0.128	\$ 960
Slab	15	CY	\$	1,000	\$	15,000	Equipment power	6500	kwh/yr	\$	0.128	\$ 832
Building	500	SF	\$	60	\$	30,000	Labor	1,000	hrs/yr	\$	40	\$ 40,000
Building electrical	500	SF	\$	8.00	\$	4,000	Materials	1	year	\$	5,000	\$ 5,000
Building plumbing	500	SF	\$	8.00	\$	4,000	Chemicals	1	year	\$	1,800	\$ 1,800
Heating and ventilation	500	SF	\$	7.00	\$	3,500	Analyses	24	test	\$	200	\$ 4,800
Fence	700	LF	\$	15	\$	10,500	Subtota	l				\$ 53,392
Paving	2,000	SF	\$	2.00	\$	4,000						
-							Backwash Disposal					
Electrical	1	JOB	\$	50,000	\$	50,000	Mileage	10,000	miles	\$	1.00	\$ 10,000
Piping	1	JOB	\$	20,000	\$	20,000	Disposal fee	1580	kgal/yr	\$	5.00	\$ 7,900
RO package including: High Pressure pumps-15 hp Cartridge filters & vessels RO membranes & vessels Control system Chemical feed systems			Ť	,	•	,	Subtota		,.	Ť		\$ 17,900
Freight cost and startup services by vendor	1	UNIT	\$	125,000	\$	125,000						
Transfer pumps (5 hp)	2	EA	\$	5,000	\$	10,000						
Permeate tank	20,000	GAL	\$	3	\$	60,000						
Reject pond												
Excavation	1,500		\$	3.00	\$	4,500						
Compacted fill	1,250	CYD	\$	7.00	\$	8,750						
Lining	21,750	SF	\$	0.50	\$	10,875						
Vegetation	2,500	SY	\$	1.00	\$	2,500						
Access road	625	LF	\$	30.00	\$	18,750						
Subtotal	l				\$	383,375						
Contingency	20%	•				76,675						
Design & CM	25%)				95,844						
Reject water haul truck	1	EA	\$	100,000	\$	100,000						
Total	Ī				\$	655,894	Tota	I				\$ 71,292

PWS Name South Midland County Water Systems Inc.

Alternative Name Central Treatment - EDR

Alternative Number SMC-6

Capital Costs

Cost Item Central-EDR	Quantity	Unit	Un	it Cost	To	otal Cost	Cost Item O&M	Quantity	Unit	Un	it Cost	Total Cost
Site preparation	0.5	acre	\$	4,000	\$	2,000	Building Power	9,000	kwh/yr	\$	0.128	\$ 1,152
Slab	20	CY	\$	1,000	\$	20,000	Equipment power	9100	kwh/yr	\$	0.128	\$ 1,165
Building	600	SF	\$	60	\$	36,000	Labor	1,000	hrs/yr	\$	40	\$ 40,000
Building electrical	600	SF	\$	8.00	\$	4,800	Materials	1	year	\$	5,000	\$ 5,000
Building plumbing	600	SF	\$	8.00	\$	4,800	Chemicals	1	year	\$	1,800	\$ 1,800
Heating and ventilation	600	SF	\$	7.00	\$	4,200	Analyses	24	test	\$	200	\$ 4,800
Fence	800	LF	\$	15	\$	12,000	Subtotal					\$ 53,917
Paving	2,400	SF	\$	2.00	\$	4,800						
-							Backwash Disposal					
Electrical	1	JOB	\$	50,000	\$	50,000	Mileage	8000	miles	\$	1.00	\$ 8,000
Piping	1	JOB	\$	20,000	\$	20,000	Disposal fee	913	kgal/yr	\$	5.00	\$ 4,565
Product storage tank	20,000	GAL	\$	3.00	\$	60,000	Subtotal					\$ 12,565
EDR package including: Feed & concentrate pumps Cartridge filters & vessels EDR membrane stacks Electrical module Chemical feed systems Freight cost & startup services by vendor	1	UNIT	\$	279,000	\$	279,000						
Reject pond												
Excavation	1,800		\$	3.00	\$	5,400						
Compacted fill	1,500	CYD	\$	7.00	\$	10,500						
Lining	26,100	SF	\$	0.50	\$	13,050						
Vegetation	3,000	SY	\$	1.00	\$	3,000						
Access road	750	LF	\$	30.00	\$	22,500						
Subtotal					\$	552,050						
Contingency	20%					110,410						
Design & CM	25%					138,013						
Reject water haul truck	1	EA	\$	100,000	\$	100,000						
Total					\$	900,473	Total					\$ 66,482

PWS Name South Midland County Water Systems Inc.

Alternative Name Point-of-Use Treatment

Alternative Number SMC-7

Number of Connections for POU Unit Installation 75

Capital Costs

Cost Item POU-Treatment - Purchase/Installation	Quantity Unit Unit		Cost	Total Cost	Cost Item O&M	Quantity	Unit	Unit	Cost	Total Cost	
POU treatment unit purchase	75	EA	\$	250	\$ 18,750	POU materials, per unit	75	EA	\$	225	\$ 16,875
POU treatment unit installation	75	EA	\$	150	\$ 11,250	Contaminant analysis, 1/yr per unit	75	EA	\$	100	\$ 7,500
Subtota	ıl				\$ 30,000	Program labor, 10 hrs/unit	750	hrs	\$	30	\$ 22,500
						Subtota	ıl				\$ 46,875
Subtotal of Component Costs					\$ 30,000						
Contingency	20%	, D			\$ 6,000						
Design & Constr Management	25%	, D			\$ 7,500						
Procurement & Administration	20%	ò			\$ 6,000						
TOTAL CAPITAL COSTS					\$ 49,500	TOTAL A	ANNUAL O	&M COS	гѕ		\$ 46,875

PWS Name South Midland County Water Systems Inc.

Alternative Name Point-of-Entry Treatment

Alternative Number SMC-8

Number of Connections for POE Unit Installation 75

Capital Costs

Cost Item POE-Treatment - Purchase/Installation	Quantity	Unit	Ur	it Cost	T	otal Cost	Cost Item O&M	Quantity	Unit	Un	it Cost	Tot	tal Cost
POE treatment unit purchase	75	EA	\$	3,000	\$	225,000	POE materials, per unit	75	EA	\$	1,000	\$	75,000
Pad and shed, per unit	75	EA	\$	2,000	\$	150,000	Contaminant analysis, 1/yr per unit	75	EA	\$	100	\$	7,500
Piping connection, per unit	75	EΑ	\$	1,000	\$	75,000	Program labor, 10 hrs/unit	750	hrs	\$	30	\$	22,500
Electrical hook-up, per unit	75	EA	\$	1,000	\$	75,000	Subtotal					\$ 1	105,000
Subtota	I				\$	525,000							
Subtotal of	Componer	nt Cost	s		\$	525,000							
Contingency	20%)			\$	105,000							
Design & Constr Management	25%)			\$	131,250							
Procurement & Administration	20%)			\$	105,000							
ТОТА	L CAPITAL	COST	S		\$	866,250	TOTAL AN	NUAL O&	I COST	S		\$ 1	105,000

PWS Name South Midland County Water Systems Inc.

Alternative Name New Well at 10 Miles

Alternative Number SMC-9

 Distance from PWS to new well location
 10.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 1

 Well installation cost (location specific)
 \$25 per foot

 Number of pump stations needed
 1

Capital Costs

Cost Item Pipeline Construction	Quantity	Unit	Un	it Cost	To	otal Cost	Cost Item Pipeline O&M	Quantity	Unit	Un	it Cost	Tot	al Cost
Number of Crossings, bore	6	n/a	n/a		n/a		Pipeline O&M	10.0	mile	\$	200	\$	2,000
Number of Crossings, bore Number of Crossings, open cut		n/a	n/a		n/a		Subtotal	10.0	Time	Ψ	200	\$	2,000
PVC water line, Class 200, 04"	52,800		\$	26.00		1,372,800	Gubtotai					Ψ	2,000
Bore and encasement, 10"	1,200		\$	60.00	\$	72.000							
Open cut and encasement, 10"	900		\$	30.00	\$	27.000							
Gate valve and box, 04"	11		\$	340.00	\$	3,590							
Air valve	10			1,000.00	\$	10,000							
Flush valve	10		\$	750.00	\$	7,920							
Metal detectable tape	52,800		\$	0.15	\$	7,920							
Subtotal	,	LF	φ	0.15		1,501,230							
						, ,							
Pump Station(s) Installation							Pump Station(s) O&M						
Pump	1	EA	\$	7,500	\$	7,500	Building Power	11,800	kWH	\$	0.128	\$	1,510
Pump Station Piping, 04"	1	EA	\$	4,000	\$	4,000	Pump Power	54,965	kWH	\$	0.128	\$	7,035
Gate valve, 04"	4	EA	\$	370	\$	1,480	Materials	1	EA	\$	1,200	\$	1,200
Check valve, 04"	2	EA	\$	430	\$	860	Labor	365	Hrs	\$	30		10,950
Electrical/Instrumentation	1	EA	\$	10,000	\$	10,000	Tank O&M	1	EA	\$	1,000	\$	1,000
Site work	1	EA	\$	2,000	\$	2,000	Subtotal					\$ 2	21,696
Building pad	1	EA	\$	4,000	\$	4,000							
Pump Building	1	EA	\$	10,000	\$	10,000							
Fence	1	EA	\$	5,870	\$	5,870							
Tools	1	EA	\$	1,000	\$	1,000							
Storage Tank - 5,000 gals	1	EA	\$	7,025	\$	7,025							
Subtotal					\$	53,735							
Well Installation							Well O&M						
Well installation	300		Φ.	25	Φ.	7.500		000	kWH	Φ.	0.128	Φ.	440
		EA	\$ \$	1,500	\$ \$	7,500 3,000	Pump power Well O&M matl		EA	\$ \$	1,200	\$ \$	118 1.200
Water quality testing	1			,		7.500	Well O&M labor		Hrs	\$	30	\$,
Well pump Well electrical/instrumentation	1		\$ \$	7,500 5,000	\$ \$	5,000	Subtotal	100	піѕ	Ф	30	Φ \$	5,400 6,718
Well cover and base	1		э \$	3,000	\$ \$	3,000	Subtotai					Ф	0,710
	1		э \$,	\$ \$	2,500							
Piping	•	EA	Ф	2,500	Φ \$,							
Subtotal					Þ	28,500							
							O&M Credit for Existing	Well Closu	re				
							Pump power	922	kWH	\$	0.128	\$	(118)
							Well O&M matl	2	EA	\$	1,200	\$	(2,400)
							Well O&M labor	360	Hrs	\$	30		10,800)
							Subtotal			•			13,318)
Subtotal of	Compone	nt Cost	s		\$	1,583,465						. (,,
Contingency	20%				\$	316,693							
Design & Constr Management	25%				\$ \$	395,866							
Design & Constrivianagement	25%)			Ф	393,000							
ТОТА	L CAPITAI	COST	s		\$	2,296,025	TOTAL A	NNUAL O	&M COSTS			\$	17,096

PWS Name South Midland County Water Systems Inc.

Alternative Name New Well at 5 Miles

Alternative Number SMC-10

 Distance from PWS to new well location
 5.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 1

 Well installation cost (location specific)
 \$25 per foot

 Number of pump stations needed
 1

Capital Costs

Cost Item	Quantity	Unit	Un	it Cost	T	otal Cost	Cost Item	Quantity	Unit	Un	it Cost	Total Cost
Pipeline Construction	2	n/a	n/a		n/a		Pipeline O&M	E () mile	\$	200	\$ 1,000
Number of Crossings, open out	9		n/a n/a		n/a n/a		Pipeline O&M Subtotal		mile	Ф	200	\$ 1,000 \$ 1,000
Number of Crossings, open cut			11/a \$	26.00	11/a \$	000 400	Subiolai					\$ 1,000
PVC water line, Class 200, 04" Bore and encasement, 10"	26,400 1.800		\$ \$	60.00		686,400 108,000						
Open cut and encasement, 10"	1,000		\$	30.00	-	3.000						
Gate valve and box, 04"	5		\$	340.00		1,795						
Air valve	5 5			340.00		5,000						
Flush valve	5		Ф \$	750.00		3,960						
Metal detectable tape	26.400		\$	0.15	\$	3,960						
Subtotal	-,	Li	Ψ	0.13	\$	812,115						
Pump Station(s) Installation							Pump Station(s) O&M					
Pump	1	EA	\$	7,500	\$	7,500	Building Power	11.800	k\//H	\$	0.128	\$ 1,510
Pump Station Piping, 04"	1		\$	4.000	-	4.000	Pump Power	27.482		\$	0.128	\$ 3,518
Gate valve, 04"	4		\$	370		1,480	Materials	27,402		\$	1,200	\$ 1,200
Check valve, 04"		EA	\$	430		860	Labor		Hrs	\$	30	\$ 10,950
Electrical/Instrumentation	1		\$	10,000		10.000	Tank O&M		EA	\$	1.000	\$ 1,000
Site work	1	EA	\$	2,000		2.000	Subtotal			•	.,	\$ 18,178
Building pad	1	EA	\$	4.000		4.000						, ,,
Pump Building	1		\$	10,000		10,000						
Fence	1	EA	\$	5,870		5.870						
Tools	1	EA	\$	1,000		1,000						
Storage Tank - 5,000 gals	1	EA	\$	7,025	\$	7,025						
Subtotal			·	,	\$	53,735						
Well Installation							Well O&M					
Well installation	300	LF	\$	25	\$	7,500	Pump power	922	kWH	\$	0.128	\$ 118
Water quality testing	2	EA	\$	1,500		3,000	Well O&M matl	1	EA	\$	1,200	\$ 1,200
Well pump	1	EA	\$	7,500	\$	7,500	Well O&M labor	180	Hrs	\$	30	\$ 5,400
Well electrical/instrumentation	1	EA	\$	5,000		5,000	Subtotal					\$ 6,718
Well cover and base	1	EA	\$	3,000	\$	3,000						
Piping	1	EA	\$	2,500	\$	2,500						
Subtotal					\$	28,500						
							O&M Credit for Existing	Well Closu	ıre			
							Pump power		kWH	\$	0.128	\$ (118)
							Well O&M matl	2	EA	\$	1,200	\$ (2,400)
							Well O&M labor	360	Hrs	\$	30	\$(10,800)
							Subtotal			·		\$(13,318)
Subtotal of	Compone	nt Cost	:s		\$	894,350						-
Contingency	20%	,)			\$	178,870						
Design & Constr Management	25%)			\$	223,588						
TOTA	L CAPITAI	_ cost	s		\$	1,296,808	TOTAL A	NNUAL O	&M COSTS	}		\$ 12,578

PWS Name South Midland County Water Systems Inc.

Alternative Name New Well at 1 Mile

Alternative Number SMC-11

 Distance from PWS to new well location
 1.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 1

 Well installation cost (location specific)
 \$25 per foot

 Number of pump stations needed
 0

Capital Costs

Cost Item	Quantity	Unit	Unit	Cost	To	otal Cost	Cost Item	Quantity	Unit	Un	it Cost	Tota	ıl Cost
Pipeline Construction							Pipeline O&M						
Number of Crossings, bore			n/a		n/a		Pipeline O&M) mile	\$	200	\$	200
Number of Crossings, open cut			n/a		n/a		Subtotal					\$	200
PVC water line, Class 200, 04"	5,280		\$	26.00	\$	137,280							
Bore and encasement, 10"		LF	\$	60.00	\$	12,000							
Open cut and encasement, 10"	100		\$	30.00	\$	3,000							
Gate valve and box, 04"	1		\$	340.00	\$	359							
Air valve	1.00	EA		1,000.00	\$	1,000							
Flush valve	1		\$	750.00	\$	792							
Metal detectable tape	5,280	LF	\$	0.15	\$	792							
Subtota	ıl				\$	155,223							
Pump Station(s) Installation							Pump Station(s) O&M	1					
Pump	-	EA	\$	7,500	\$	-	Building Power	-	kWH	\$	0.128	\$	-
Pump Station Piping, 04"	-	EA	\$	4,000	\$	-	Pump Power	-	kWH	\$	0.128	\$	-
Gate valve, 04"	-	EA	\$	370	\$	-	Materials	-	EA	\$	1,200	\$	-
Check valve, 04"	-	EA	\$	430	\$	-	Labor	-	Hrs	\$	30	\$	-
Electrical/Instrumentation	-	EA	\$	10,000	\$	-	Tank O&M	-	EA	\$	1,000	\$	-
Site work	-	EA	\$	2,000	\$	-	Subtotal					\$	-
Building pad	-	EA	\$	4,000	\$	-							
Pump Building	-	EA	\$	10,000	\$	-							
Fence	-	EA	\$	5,870	\$	-							
Tools	-	EA	\$	1,000	\$	-							
Storage Tank - 5,000 gals	-	EA	\$	7,025	\$	-							
Subtota	ıl				\$	-							
Well Installation							Well O&M						
Well installation	300	LF	\$	25	\$	7,500	Pump power	922	kWH	\$	0.128	\$	118
Water quality testing	2		\$	1.500	\$	3.000	Well O&M matl	1	EA	\$	1.200		1.200
Well pump	1	EA	\$	7,500		7,500	Well O&M labor	180	Hrs	\$	30		5,400
Well electrical/instrumentation	1		\$	5,000	\$	5,000	Subtotal			•			6,718
Well cover and base	1	EA	\$	3,000	\$	3,000						•	,
Piping	1	FA	\$	2,500	\$	2,500							
Subtota			•	2,000	\$	28,500							
							O&M Credit for Existir	na Well Clo	sure				
							Pump power	•	kWH	\$	0.128	\$	(118)
							Well O&M matl		EA	\$	1,200		2,400)
							Well O&M labor	_	Hrs	\$	30		0,800)
							Subtotal		1113	Ψ	50		3,318)
Subtotal of	Component	Costs			\$	183,723	Subtotal					Ψ(I	3,310)
Contingency	20%				\$	36,745							
Design & Constr Management	25%				\$	45,931							
Design & Consti Management	23%				Φ	40,301							
TOTA	L CAPITAL (COSTS			\$	266,398	TOTAL A	ANNUAL O	&M COSTS			\$ (6,400)

South Midland County Water Systems Inc. Public Dispenser for Treated Drinking Water **PWS Name Alternative Name**

SMC-12 **Alternative Number**

Number of Treatment Units Recommended

1

Capital Costs	Annual Operations and Maintenance Costs
---------------	------------------------------------------------

Cost Item	Quantity Un	it Unit Cos	t Total Cost	Cost Item	Quantity	Unit	Unit	Cost	Total Cost
Public Dispenser Unit Installation				Program Operation					
POE-Treatment unit(s)	1 EA	\$ 3,000	\$ 3,000	Treatment unit O&M, 1 per unit	1	EA	\$	500	\$ 500
Unit installation costs	1 EA	\$ 5,000	\$ 5,000	Contaminant analysis, 1/wk per unit	52	EA	\$	100	\$ 5,200
Subtot	al		\$ 8,000	Sampling/reporting, 1 hr/day	365	HRS	\$	30	\$ 10,950
				Subtota	I				\$ 16,650
Subtotal o	f Component Co	osts	\$ 8,000						
Contingency	20%		\$ 1,600						
Design & Constr Management	25%		\$ 2,000						
тот	AL CAPITAL CO	STS	11,600	TOTAL A	ANNUAL O	&M COS	ΓS		\$ 16,650

PWS Name South Midland County Water Systems Inc.

Alternative Name Supply Bottled Water to Population

Alternative Number SMC-13

Service Population256Percentage of population requiring supply100%Water consumption per person1.00 gpcdCalculated annual potable water needs93,440 gallons

Capital Costs

Cost Item Program Implementation	Quantity	Unit	Unit	Cost	Total Cost	Cost Item Program Operation	Quantity	Unit	Un	it Cost	Total Cost
Initial program set-up	500	hours	\$	40	\$ 19,950	Water purchase costs	93,440	gals	\$	1.60	\$ 149,504
Subtota	al				\$ 19,950	Program admin, 9 hrs/wk	468	hours	\$	40	\$ 18,673
						Program materials	1	EA	\$	5,000	\$ 5,000
						Subtotal					\$ 173,177
Subtota	I of Compo	nent Cost	ts		\$ 19,950						
Contingency	20%	, D			\$ 3,990						
тс	TAL CAPIT	AL COST	s		\$ 23,940	TOTAL A	NNUAL O	&M COS	гs		\$ 173,177

PWS Name South Midland County Water Systems Inc.

Alternative Name Central Trucked Drinking Water

Alternative Number SMC-14

Service Population	256
Percentage of population requiring supply	100%
Water consumption per person	1.00 gpcd
Calculated annual potable water needs	93,440 gallons
Travel distance to compliant water source (roundtrip)	4 miles

Capital Costs

Cost Item Storage Tank Installation	Quantity	Unit	Unit Cost	To	tal Cost	Cost Item Program Operation	Quantity	Unit	Uni	t Cost	Tot	tal Cost
Storage Tank Installation Storage Tank - 5,000 gals	1	EΑ	\$ 7.025	\$	7.025	Water delivery labor, 4 hrs/wk	208	hrs	\$	30	\$	6,240
Site improvements		EA	\$ 4,000		4.000	Truck operation, 1 round trip/wk		miles	\$	1.00	\$	208
Potable water truck		EA	\$ 60.000		60.000	Water purchase		1,000 gals	Ψ	1.80	\$	168
Subtota			+,	\$	71,025	Water testing, 1 test/wk		EA	\$	100	\$	5,200
					•	Sampling/reporting, 2 hrs/wk	104	hrs	\$	30	\$	3,120
						Subtota	ıl				\$	14,936
Subtotal o	f Componen	nt Cost	s	\$	71,025							
Contingency	20%			\$	14,205							
Design & Constr Management	25%			\$	17,756							
тот	AL CAPITAL	COST	·s	\$	102,986	TOTAL	ANNUAL O	&M COSTS			\$	14,936

1

2

APPENDIX D EXAMPLE FINANCIAL MODEL

Table D.1 Example Financial Model

Step 1					
Water System:	South Midland County				
Water System.	Court Midiana County				
Step 2	Click Here to Update Verification and Raw				
Water System	South Midland County				
Alternative Description	Public Dispenser for Treated Drinking Water	4			
0		V		Francisco	- Alt C
Sum of Amount		Year	2007		g Alternative
0	In	1000/ 0	2007		
Group Capital Expenditures	Type	100% Grant		Bond \$	11.000
Capital Expenditures	Capital Expenditures-Funded from Bonds Capital Expenditures-Funded from Grants	\$ \$	11,600	*	11,600
	Capital Expenditures-Funded from Revenue/Reserves	\$	11,600	\$ \$	-
	Capital Expenditures-Funded from SRF Loans	\$	-	\$ \$	-
Capital Expenditures Sum	Capital Experiolitures-1 unded Ironi Sixi Edans	\$	11,600	\$	11,600
Debt Service	Revenue Bonds	\$	11,000	\$	907
Debt Service	State Revolving Funds	\$		\$	301
Debt Service Sum	State Revolving Funds	\$		\$	907
Operating Expenditures	Administrative Expenses	\$	500		500
Operating Expericitures	Chemicals, Treatment	\$	600	*	600
	Contract Labor	\$	900		900
	Repairs	\$	2,880		2,880
	Supplies	\$	600		600
	Utilities	\$	1,000		1.000
	Maintenance	\$	1,000		1,000
	Bad Debts	\$	1,100		1,100
Operating Expenditures Su		\$	8,580		8,580
Residential Operating Reve		\$	10.584	_	10,584
Trooldonial Operating New	Residential Tier 1 Monthly Rate	\$	1.764	\$	1,764
	Residential Tier2 Monthly Rate	\$	- 1,7 54	\$,704
	Residential Tier3 Monthly Rate	\$	-	\$	_
	Residential Tier4 Monthly Rate	\$	_	\$	<u>-</u>
	Residential Unmetered Monthly Rate	\$		\$	
Residential Operating Reve	,	\$	12.348	\$	12.348
Residential Operating Reve	HIUES OUIII	Φ	12,348	Ф	12,34

Location_Name	South Midland County		
Alt_Desc	Public Dispenser for Treated Drinking Water		
		Current	Year
Funding_Alt	Data		2007
100% Grant	Sum of Beginning_Cash_Bal	\$	3,268
	Sum of Total_Expenditures	\$	20,180
	Sum of Total_Receipts	\$	23,948
	Sum of Net_Cash_Flow	\$	3,768
	Sum of Ending_Cash_Bal	\$	7,036
	Sum of Working_Cap	\$	-
	Sum of Repl_Resv	\$	500
	Sum of Total_Reqd_Resv	\$	500
	Sum of Net_Avail_Bal	\$	6,536
	Sum of Add_Resv_Needed	\$	-
	Sum of Rate_Inc_Needed		0%
	Sum of Percent_Rate_Increase		0%
Bond	Sum of Beginning_Cash_Bal	\$	3,268
	Sum of Total_Expenditures	\$	21,087
	Sum of Total_Receipts	\$	23,948
	Sum of Net_Cash_Flow	\$	2,861
	Sum of Ending_Cash_Bal	\$	6,129
	Sum of Working_Cap	\$	-
	Sum of Repl_Resv	\$	500
	Sum of Total_Reqd_Resv	\$	500
	Sum of Net_Avail_Bal	\$	5,629
	Sum of Add_Resv_Needed	\$	-
	Sum of Rate_Inc_Needed	1	0%
	Sum of Percent_Rate_Increase		0%

APPENDIX E GENERAL GEOCHEMISTRY FOR ARSENIC AND NITRATE

General Arsenic Geochemistry

1

2

3

4

5

6

7 8

9

10

11

12

13

14

15

16 17

18

19

20

21

22

23

24

25

26

27

28

29

30

31

32

33

34

35

36

37

38

39

40

On January 22, 2001 the USEPA adopted a new standard for arsenic in drinking water at 10 ppb, replacing the old standard of 50 ppb. The rule became effective on February 22, 2002. The date by which systems must comply with the new 10 µg/L standard is January 23, 2006. The geochemistry of arsenic is complex because of the possible coexistence of two or even three redox states (-III, III, V) and because of the strong interaction of most arsenic compounds with soil particles, particularly iron oxides. Because groundwater is generally oxidizing in the High Plains, Edwards Trinity (Plateau), and Cenozoic Pecos Alluvium aquifers, it is expected to be in the arsenate form (V). Correlations between arsenic and vanadium and fluoride suggest a geologic rather than anthropogenic source of arsenic. The large number of potential geologic sources include: volcanic ashes in the Ogallala and underlying units, shales in the Cretaceous, and saline lakes in the Southern High Plains that were evaluated in a separate study and described in Scanlon, et al. (2005). Arsenic mobility is generally not controlled by solubility of arsenic-bearing minerals because these minerals are highly soluble. Under oxidizing conditions, arsenic mobility increases with increasing pH (Smedley and Kinniburg 2000). Phosphate can also increase arsenic mobility because phosphate preferentially sorbs onto clays and iron oxides relative to arsenic.

General Nitrate Geochemistry

Nitrate contamination occurs when nitrate-N concentrations exceed 10 mg/L nitrate-N (MCL for nitrate-N). Nitrate is negatively charged and behaves conservatively; *i.e.*, it does not sorb onto soil, volatilize, precipitate readily, *etc*. Natural sources of nitrate include fixed nitrogen by shrubs such as mesquite in rangeland settings. Nitrate concentrations in soil profiles in most rangeland settings in the Southern High Plains are generally low (Scanlon, *et al.* 2003; McMahon, *et al.* 2005). Conversion of rangeland to agriculture can result in nitrification of soil organic matter. Anthropogenic sources of nitrate include chemical and organic (manure) fertilizers, nitrogen fixation through growth of leguminous crops, and barnyard and septic tank effluent. Nitrogen isotopes have been used to distinguish these various sources; however, such a study has not been conducted in the Southern High Plains. Nitrogen profiles measured in soil in Dawson County indicated that nitrate concentrations in soil pore water were generally low to moderate (Scanlon, *et al.* 2003). The highest concentrations were found in irrigated areas because irrigation water contains higher nitrate concentrations than rain water and irrigation rates are low enough to result in evapoconcentration of nitrate in the soil.

Appendix References

McMahon PB, Dennehy KF, Bruce BW, Bohlke JK, Michel RL, Gurdak JJ, Hurlbut DB. 2005. Storage and transit time of chemicals in thick unsaturated zones under rangeland and irrigated cropland, High Plains, USA. *Water Resources Research*.

1 2 3	Scanlon BR, Reedy RC, Keese KE. 2003. Estimation of groundwater recharge in Texas related to aquifer vulnerability to contamination. <i>Bureau of Economic Geology, Univ. of Texas at Austin, Final Contract Report, 84 p.</i>
4	Scanlon, B.R., Nance, S., Nicot, J.P., Reedy, R.C., Smyth, R., Tachovsky, A., 2005,
5	Evaluation of arsenic concentrations in groundwater in Texas; The University of
6	Texas Bureau of Economic Geology, Final Report, Prepared for the Texas
7	Commission on Environmental Quality.
8	Smedley PL, Kinniburgh DG. 2002. A review of the source, behaviour and distribution of
9	arsenic in natural waters. Applied Geochemistry 17: 517-568.