DRAFT FEASIBILITY REPORT
FEASIBILITY ANALYSIS OF WATER SUPPLY FOR SMALL PUBLIC WATER SYSTEMS

ROOSEVELT ISD
PWS ID# 1520123

Prepared for:

THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Prepared by:

THE UNIVERSITY OF TEXAS BUREAU OF ECONOMIC GEOLOGY

AND

PARSONS

Preparation of this report was financed by the Texas Commission on Environmental Quality through the Drinking Water State Revolving Fund Small Systems Assistance Program

AUGUST 2007

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EXECUTIVE SUMMARY

INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), was contracted by the Texas Commission on Environmental Quality (TCEQ) to conduct a project to assist with identifying and analyzing alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project was to promote compliance using sound engineering and financial methods and data for PWSs that had recently recorded sample results exceeding maximum contaminant levels (MCL). The primary objectives of this project were to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS for future implementation.

This feasibility report provides an evaluation of water supply alternatives for the Roosevelt Independent School District (ISD) Water System PWS. The Roosevelt ISD Water System is located north of Ransom Canyon, Texas at 1406 County Road (CR) 3300, and 7 miles east of Lubbock. Berhl Robertson is the ISD Superintendent; Roy Turner is the Maintenance Director, and Dale Lampe is the system operator. Roosevelt ISD serves 1,600 people with 11 service connections. The school district has held discussions with the City of Lubbock to provide water through the City of Ransom Canyon, which has a line about 4 miles south of the school.

Between January 1997 and April 2005, arsenic concentrations ranged from 0.006 mg/L to 0.0164 mg/L at the Roosevelt ISD PWS. Nitrate concentrations ranged from <0.01 mg/L to 20.6 mg/L. The majority of measurements were above the 0.010 mg/L MCL for arsenic and 10 mg/L for nitrate. Therefore, the Roosevelt ISD faces compliance issues under the water quality standards for arsenic and nitrate.

Basic system information for the Roosevelt ISD Water System PWS is shown in Table ES.1.

Table ES.1 Roosevelt ISD Water System PWS Basic System Information

Population served	1,600
Connections	11
Average daily flow rate	0.048 million gallons per day (mgd)
Peak demand flow rate	133 gallons per minute (0.192 mgd), estimated
Water system peak capacity	0.192 mgd (estimated)
Typical arsenic range	0.006 to 0.0164 mg/L
Typical nitrate range	<0.01 to 20.6 mg/L

STUDY METHODS

The methods used for this project were based on those of a pilot project performed in 2004 and 2005 by TCEQ, BEG, and Parsons. Methods for identifying and analyzing compliance options were developed in the pilot project (a decision tree approach).

The process for developing the feasibility study used the following general steps:

1. Gather data from the TCEQ and Texas Water Development Board databases, from TCEQ files, and from information maintained by the PWS;
2. Conduct financial, managerial, and technical (FMT) evaluations of the PWS;
3. Perform a geologic and hydrogeologic assessment of the study area;
4. Develop treatment and non-treatment compliance alternatives which, in general, consist of the following possible options:

- Connecting to neighboring PWSs via new pipeline or by pumping water from a newly installed well or an available surface water supply within the jurisdiction of the neighboring PWS;
- Installing new wells within the vicinity of the PWS into other aquifers with confirmed water quality standards meeting the MCLs;
- Installing a new intake system within the vicinity of the PWS to obtain water from a surface water supply with confirmed water quality standards meeting the MCLs;
- Treating the existing non-compliant water supply by various methods depending on the type of contaminant; and
- Delivering potable water by way of a bottled water program or a treated water dispenser as an interim measure only.

5. Assess each of the potential alternatives with respect to economic and non-economic criteria;
6. Prepare a feasibility report and present the results to the PWS.

This basic approach is summarized in Figure ES-1.

HYDROGEOLOGICAL ANALYSIS

The major aquifer in the study area is the High Plains or Ogallala aquifer. The main geologic unit that makes up the High Plains aquifer is the Ogallala Formation, which consists of coarse fluvial sandstones and conglomerates. The Roosevelt ISD PWS obtains groundwater from three wells designated as being at a depth of 150 feet within the Ogallala aquifer.

There are no obvious groundwater sources in the vicinity (10 km) of the PWS that can serve as alternative sources. Because no wells in the vicinity of the PWS wells show acceptable water quality, it may be necessary to look for new supplies in or near wells farther from the PWS. Acceptable groundwater quality increases to the northeast, coinciding with a regional change in water quality in the Ogallala aquifer. This area is a significant distance away.

In addition, regional analyses show that water quality increases with depth. This suggests that tapping deeper water by increasing the depth of one or more wells and screening only the deeper portion may decrease concentrations of these constituents in drinking water. However, there are not enough local data available to evaluate this option.

COMPLIANCE ALTERNATIVES

Overall, the system had an adequate level of FMT capacity. The system had some areas that needed improvement to be able to address future compliance issues; however, the system does have several positive aspects, including knowledgeable and dedicated staff and efforts toward compliance of nitrate problem. Areas of concern for the system included lack of long-term capital improvement planning and lack of compliance with water quality standards.

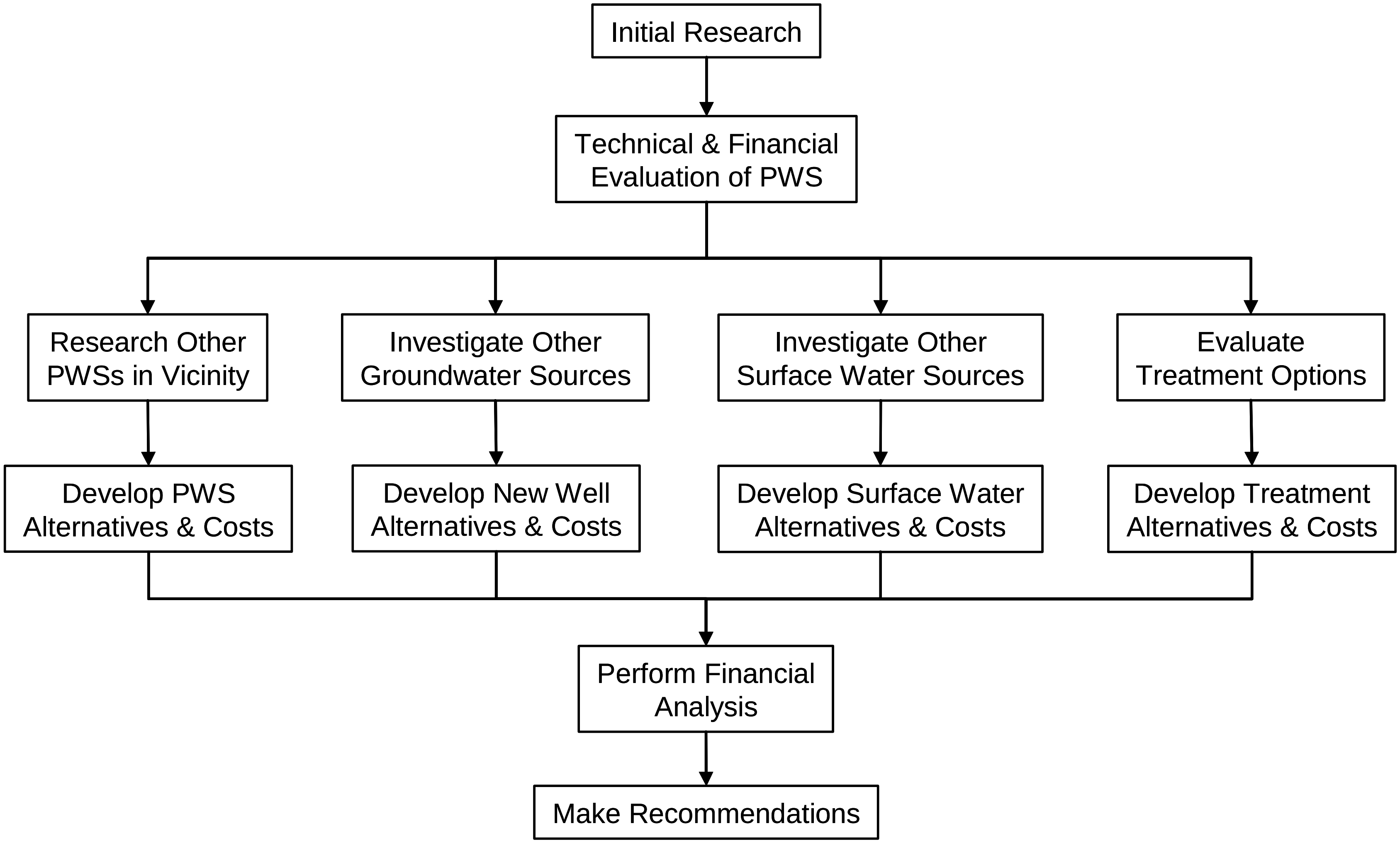
There are several PWSs within 15 miles of Roosevelt ISD. Many of these nearby systems also have water quality problems, but the City of Lubbock has good quality water. Specific feasibility alternatives were developed based on obtaining water from the City of Lubbock. The City of Lubbock utilizes a mix of surface water and groundwater as a source of water. A purchase treated water alternative was developed that involves purchasing compliant water from the City of Lubbock via a pipeline running between Ransom Canyon/Buffalo Springs to the Roosevelt ISD PWS.

Developing a new well close to Roosevelt ISD Water System is another likely alternative if compliant groundwater can be found. Having a new well close to Roosevelt ISD Water System was considered since the PWS already possesses the technical and managerial expertise needed to implement this option. The cost of new well alternatives quickly increases with pipeline length, making proximity of the alternate source a key concern. A new compliant well or obtaining water from a neighboring compliant PWS has the advantage of providing compliant water to all taps in the system.

Reverse osmosis (RO) and electrodialysis (EDR) centralized treatment alternatives for arsenic and nitrate removal have been developed and were considered for this report.

Central treatment can be cost-competitive with the alternative of new nearby wells, but would require significant institutional changes to manage and operate. Like obtaining an alternate compliant water source, central treatment would provide compliant water to all water taps.

Figure ES-1 Summary of Project Methods



FINANCIAL ANALYSIS

Financial analysis of the Roosevelt ISD Water System PWS was performed using estimated expenses and revenues, since detailed financial information pertaining to the water system was not available. Table ES.2 provides a summary of the financial impact of implementing selected compliance alternatives, including the rate increase necessary to meet current operating expenses. The alternatives were selected to highlight results for the best alternatives from each different type or category.

Some of the compliance alternatives offer potential for shared or regional solutions. A group of PWSs could work together to implement alternatives for developing a new groundwater source or expanding an existing source, obtaining compliant water from a large regional provider, or for central treatment. Sharing the cost for implementation of these alternatives could reduce the cost on a per user basis. Additionally, merging PWSs or management of several PWSs by a single entity offers the potential for reduction in administrative costs.

Table ES.2 Selected Financial Analysis Results

Alternative	Funding Option	Average Annual Water Bill per Student	Percent of MHI
Current	NA	$9.50	n/a
To meet current expenses	NA	$9.50	n/a
Purchase Water from Lubbock-Ransom Canyon	100% Grant	$55	n/a
	Loan/Bond	$104	n/a
Central treatment	100% Grant	$57	n/a
	Loan/Bond	$111	n/a

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ACRONYMS AND ABBREVIATIONS

°F	degrees Fahrenheit
µg/L	micrograms per liter
AA	activated alumina
BAT	best available technology
BEG	Bureau of Economic Geology
CA	chemical analysis
CCN	Certificate of Convenience and Necessity
CDBG	Community Development Block Grant
CFR	Code of Federal Regulations
CR	county road
CRMWA	Canadian River Municipal Water Authority
EDR	electrodialysis reversal
FMT	financial, managerial, and technical
GAM	groundwater availability model
gpd	gallons per day
gpm	gallons per minute
HUD	U.S. Department of Housing and Urban Development
ISD	independent school district
LARS	Lubbock Area Regional Solution
MCL	maximum contaminant level
MF	microfiltration
mg/L	milligram per liter
mgd	million gallons per day
MHI	median household income
NF	nanofiltration
NMEFC	New Mexico Environmental Financial Center
NURE	National Uranium Resource Evaluation
O&M	operation and maintenance
ORCA	Office of Rural Community Affairs
Parsons	Parsons Infrastructure and Technology, Inc.
POU	point-of-use
psi	pounds per square inch
PWS	public water system
RO	reverse osmosis
SDWA	Safe Drinking Water Act
SRF	state revolving fund
TCEQ	Texas Commission on Environmental Quality
TCF	Texas Capital Fund
TDA	Texas Department of Agriculture
TDS	total dissolved solids
TFC	thin film composite
TWDB	Texas Water Development Board
USEPA	United States Environmental Protection Agency
WAM	water availability model

SECTION 1
INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), have been contracted by the Texas Commission on Environmental Quality (TCEQ) to assist with identifying and analyzing compliance alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project is to promote compliance using sound engineering and financial methods and data from PWSs that have recently had sample results that exceed maximum contaminant levels (MCL). The primary objectives of this project are to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS with regard to future implementation. The feasibility studies identify a range of potential compliance alternatives and present basic data that can be used for evaluating feasibility. The compliance alternatives addressed include a description of what would be required for implementation, conceptual cost estimates for implementation, and non-cost factors that could be used to differentiate between alternatives. The cost estimates are intended for comparing compliance alternatives and to give a preliminary indication of potential impacts on water rates resulting from implementation.

It is anticipated that the PWS will review the compliance alternatives in this report to determine if there are promising alternatives, and then select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation. This report contains a decision tree approach that guided the efforts for this project and also contains steps to guide a PWS through the subsequent evaluation, selection, and implementation of a compliance alternative.

This feasibility report provides an evaluation of water supply compliance options for the Roosevelt ISD Water System, PWS ID# 1520123, located in Lubbock County, Texas. Recent sample results from the Roosevelt ISD water system exceeded the MCL for arsenic of 10 micrograms per liter (µg/L) that went into effect January 23, 2006 (USEPA 2007a; TCEQ 2004). Recent sample results also exceeded the MCL for nitrate of 10 milligrams per liter (mg/L). The location of the Roosevelt ISD Water System is shown on Figure 1.1. Various water supply and planning jurisdictions are shown on Figure 1.2. These water supply and planning jurisdictions are used in the evaluation of alternate water supplies that may be available in the area.

1.1 PUBLIC HEALTH AND COMPLIANCE WITH MCLS

The goal of this project is to promote compliance for PWSs that supply drinking water exceeding regulatory MCLs. This project only addresses those contaminants and does not address any other violations that may exist for a PWS. As mentioned above, Roosevelt ISD water system had recent sample results that exceed the MCL for arsenic and nitrate. In general, contaminant(s) in drinking water above the MCL(s) can have both short-term (acute) and long-term or lifetime (chronic) effects. Health concerns related to drinking water above MCLs for these two chemicals are briefly described below.

Potential health effects from long-term ingestion of water with levels of arsenic above the MCL (0.010 mg/L) include non-cancerous effects, such as cardiovascular, pulmonary, immunological, neurological and endocrine effects, and cancerous effects, including skin, bladder, lung, kidney, nasal passage, liver and prostate cancer (USEPA 2007b).

Short-term effects of nitrate in drinking water above the MCL have caused serious illness and sometimes death. Drinking water health publications conclude that the most susceptible population to adverse nitrate health effects includes infants less than 6 months of age; women who are pregnant or nursing; and individuals with enzyme deficiencies or a lack of free hydrochloric acid in the stomach. The serious illness in infants is due to the conversion of nitrate to nitrite by the body, which can interfere with the oxygen-carrying capacity of the child's blood. Symptoms include shortness of breath and blue-baby syndrome. Lifetime exposure to nitrates at levels above the MCL has the potential to cause the following effects: diuresis, increased starchy deposits, and hemorrhaging of the spleen (USEPA 2007c).

1.2 METHODS

The methods for this project follow that of a pilot project performed by TCEQ, BEG, and Parsons. The pilot project evaluated water supply alternatives for PWSs that supply drinking water with nitrate concentrations above U.S. Environmental Protection Agency (USEPA) and Texas drinking water standards. Three PWSs were evaluated in the pilot project to develop the method (*i.e.*, decision tree approach) for analyzing options for provision of compliant drinking water. This project is performed using the decision tree approach that was developed for the pilot project, and which was also used for subsequent projects in 2005 and 2006.

Other tasks of the feasibility study are as follows:

- Identifying available data sources;
- Gathering and compiling data;
- Conducting financial, managerial, and technical (FMT) evaluations of the selected PWSs;
- Performing a geologic and hydrogeologic assessment of the area;
- Developing treatment and non-treatment compliance alternatives;
- Assessing potential alternatives with respect to economic and non-economic criteria;
- Preparing a feasibility report; and
- Suggesting refinements to the approach for future studies.

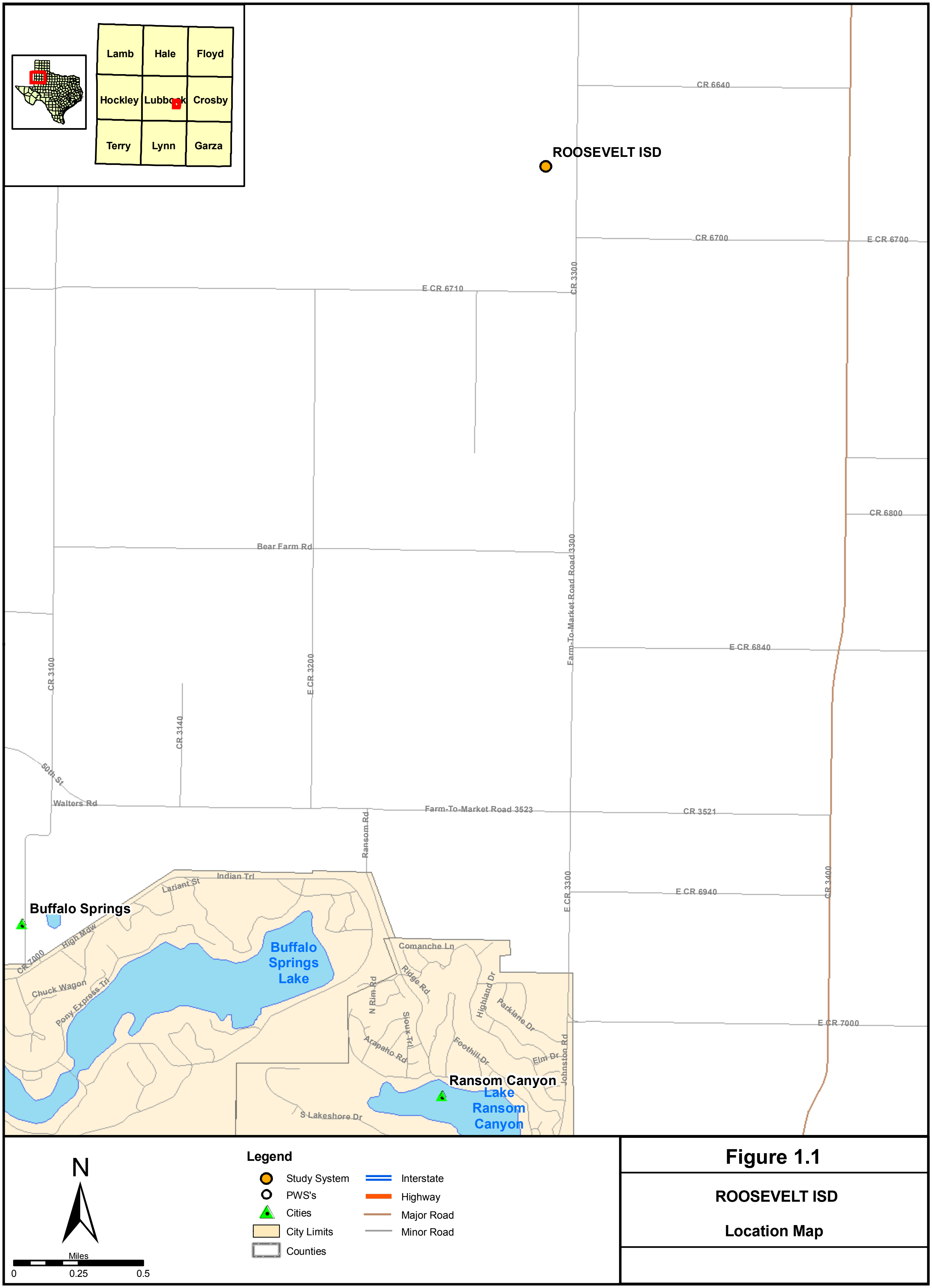


Figure 1.1

ROOSEVELT ISD

Location Map

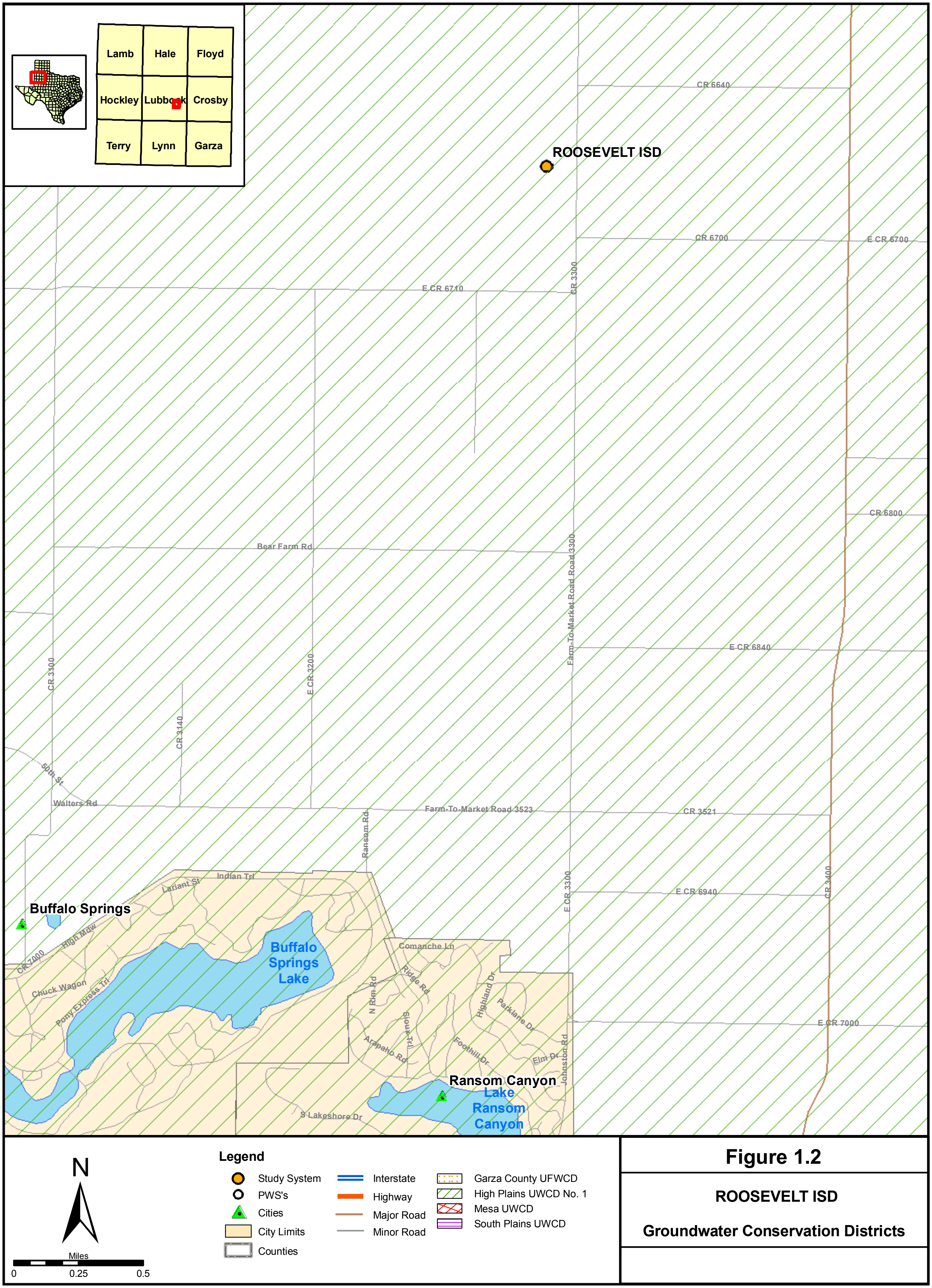


Figure 1.2

ROOSEVELT ISD

Groundwater Conservation Districts

The remainder of Section 1 of this report addresses the regulatory background, and provides a summary of nitrate and arsenic abatement options. Section 2 describes the method used to develop and assess compliance alternatives. The groundwater sources of nitrate and arsenic are addressed in Section 3. Findings for the Roosevelt ISD PWS, along with compliance alternatives development and evaluation, can be found in Section 4. Section 5 references the sources used in this report.

1.3 REGULATORY PERSPECTIVE

The Utilities & Districts and Public Drinking Water Sections of the TCEQ Water Supply Division are responsible for implementing requirements of the Federal Safe Drinking Water Act (SDWA) which include oversight of PWSs and water utilities. These responsibilities include:

- Monitoring public drinking water quality;
- Processing enforcement referrals for MCL violators;
- Tracking and analyzing compliance options for MCL violators;
- Providing FMT assessment and assistance to PWSs;
- Participating in the Drinking Water State Revolving Fund (SRF) program to assist PWSs in achieving regulatory compliance; and
- Setting rates for privately-owned water utilities.

This project was conducted to assist in achieving these responsibilities.

1.4 ABATEMENT OPTIONS

When a PWS exceeds a regulatory MCL, the PWS must take action to correct the violation. The MCL exceedances at the Roosevelt ISD PWS involve nitrate and arsenic. The following subsections explore alternatives considered as potential options for obtaining/providing compliant drinking water.

1.4.1 Existing Public Water Supply Systems

A common approach to achieving compliance is for the PWS to make arrangements with a neighboring PWS for water supply. For this arrangement to work, the PWS from which water is being purchased (supplier PWS) must have water in sufficient quantity and quality, the political will must exist, and it must be economically feasible.

1.4.1.1 Quantity

For purposes of this report, quantity refers to water volume, flowrate, and pressure. Before approaching a potential supplier PWS, the non-compliant PWS should determine its water demand on the basis of average day and maximum day. Peak instantaneous demands can be met through proper sizing of storage facilities. Further, the potential for obtaining the appropriate quantity of water to blend to achieve compliance should be considered. The concept of blending involves combining water with low levels of contaminants with non-compliant water in sufficient quantity that the resulting blended water is compliant. The exact blend ratio would depend on the quality of the water a potential supplier PWS can provide, and would likely vary over time. If high quality water is purchased, produced or otherwise obtained, blending can reduce the amount of high quality water required. Implementation of blending will require a control system to ensure the blended water is compliant.

If the supplier PWS does not have sufficient quantity, the non-compliant community could pay for the facilities necessary to increase the quantity to the extent necessary to supply the needs of the non-compliant PWS. Potential improvements might include, but are not limited to:

- Additional wells;
- Developing a new surface water supply,
- Additional or larger-diameter piping;
- Increasing water treatment plant capacity
- Additional storage tank volume;
- Reduction of system losses,
- Higher-pressure pumps; or
- Upsized, or additional, disinfection equipment.

In addition to the necessary improvements, a transmission pipeline would need to be constructed to tie the two PWSs together. The pipeline must tie-in at a point in the supplier PWS where all the upstream pipes and appurtenances are of sufficient capacity to handle the new demand. In the non-compliant PWS, the pipeline must tie in at a point where no down stream bottlenecks are present. If blending is the selected method of operation, the tie-in point must be at the proper point of the existing non-compliant PWS to ensure that all the water in the system is blended to achieve regulatory compliance.

1.4.1.2 Quality

If a potential supplier PWS obtains its water from the same aquifer (or same portion of the aquifer) as the non-compliant PWS, the quality of water may not be significantly better. However, water quality can vary significantly due to well location, even within the same aquifer. If localized areas with good water quality cannot be identified, the non-compliant PWS would need to find a potential supplier PWS that obtains its water from a different aquifer or from a surface water source. Additionally, a potential supplier PWS may treat non-compliant raw water to an acceptable level.

Surface water sources may offer a potential higher-quality source. Since there are significant treatment requirements, utilization of surface water for drinking water is typically most feasible for larger local or regional authorities or other entities that may provide water to several PWSs. Where PWSs that obtain surface water are neighbors, the non-compliant PWS may need to deal with those systems as well as with the water authorities that supply the surface water.

1.4.2 Potential for New Groundwater Sources

1.4.2.1 Existing Non-Public Supply Wells

Often there are wells not associated with PWSs that are located in the vicinity of the non-compliant PWS. The current use of these wells may be for irrigation, industrial purposes, domestic supply, stock watering, and other purposes. The process for investigating existing wells is as follows:

- Existing data sources (see below) are used to identify wells in the areas that have satisfactory quality. For Roosevelt ISD, the following standards could be used in a rough screening to identify compliant groundwater in surrounding systems:
    - Nitrate (measured as nitrogen) concentrations less than 8 mg/L (below the MCL of 10 mg/L);
    - Fluoride concentration less than 2.0 mg/L (below the Secondary MCL of 2 mg/L);
    - Arsenic concentration less than 0.008 mg/L (below the MCL of 0.010 mg/L);
    - Uranium concentration less than 24 µg/L (below the MCL of 30 µg/L; and
    - Selenium concentration less than 0.04 mg/L (below the MCL of 0.050 mg/L).
- The recorded well information are reviewed to eliminate those wells that appear to be unsuitable for the application. Often, the "Remarks" column in the Texas Water Development Board (TWDB) hard-copy database provides helpful information. Wells eliminated from consideration generally include domestic and stock wells, dug wells, test holes, observation wells, seeps and springs, destroyed wells, wells used by other communities, *etc*;
- Wells of sufficient size are identified. Some may be used for industrial or irrigation purposes. Often the TWDB database will include well yields, which may indicate the likelihood that a particular well is a satisfactory source;
- At this point in the process, the local groundwater control district (if one exists) should be contacted to obtain information about pumping restrictions. Also, preliminary cost estimates should be made to establish the feasibility of pursuing further well development options;
- If particular wells appear to be acceptable, the owner(s) should be contacted to ascertain their willingness to work with the PWS. Once the owner agrees to participate in the program, questions should be asked about the wells. Many owners have more than one well, and would probably be the best source of information

regarding the latest test dates, who tested the water, flowrates, and other well characteristics;

- After collecting as much information as possible from cooperative owners, the PWS would then narrow the selection of wells and sample and analyze them for quality. Wells with good quality would then be potential candidates for test pumping. In some cases, a particular well may need to be refurbished before test pumping. Information obtained from test pumping would then be used in combination with information about the general characteristics of the aquifer to determine whether a well at this location would be suitable as a supply source;
- It is recommended that new wells be installed instead of using existing wells to ensure the well characteristics are known and the well meets construction standards; and
- Permit(s) would then be obtained from the groundwater control district or other regulatory authority, and an agreement with the owner (purchase or lease, access easements, *etc*.) would then be negotiated.

1.4.2.2 Develop New Wells

If no existing wells are available for development, the PWS or group of PWSs has an option of developing new wells. Records of existing wells, along with other hydrogeologic information and modern geophysical techniques, should be used to identify potential locations for new wells. In some areas, the TWDB's Groundwater Availability Model (GAM) may be applied to indicate potential sources. Once a general area has been identified, land owners and regulatory agencies should be contacted to determine an exact location for a new well or well field. Pump tests and water quality tests would be required to determine if a new well will produce an adequate quantity of good quality water. Permits from the local groundwater control district or other regulatory authority could also be required for a new well.

1.4.3 Potential for Surface Water Sources

Water rights law dominates the acquisition of water from surface water sources. For a PWS, 100 percent availability of water is required, except where a back-up source is available. For PWSs with an existing water source, although it may be non-compliant because of elevated concentrations of one or more parameters, water rights may not need to be 100 percent available.

1.4.3.1 Existing Surface Water Sources

"Existing surface water sources" of water refers to municipal water authorities and cities that obtain water from surface water sources. The process of obtaining water from such a source is generally less time consuming and less costly than the process of developing a new source; therefore, it should be a primary course of investigation. An existing source would be limited by its water rights, the safe yield of a reservoir or river, or by its water treatment or water conveyance capability. The source must be able to meet the current demand and honor contracts with communities it currently supplies. In many cases, the contract amounts reflect projected future water demand based on population or industrial growth.

A non-compliant PWS would look for a source with sufficient spare capacity. Where no such capacity exists, the non-compliant PWS could offer to fund the improvements necessary to obtain the capacity. This approach would work only where the safe yield could be increased (perhaps by enlarging a reservoir) or where treatment capacity could be increased. In some instances water rights, where they are available, could possibly be purchased.

In addition to securing the water supply from an existing source, the non-compliant PWS would need to arrange for transmission of the water to the PWS. In some cases, that could require negotiations with, contracts with, and payments to an intermediate PWS (an intermediate PWS is one where the infrastructure is used to transmit water from a "supplier" PWS to a "supplied" PWS, but does not provide any additional treatment to the supplied water). The non-compliant PWS could be faced with having to fund improvements to the intermediate PWS in addition to constructing its own necessary transmission facilities.

1.4.3.2 New Surface Water Sources

Communication with the TCEQ and relevant planning groups from the beginning is essential in the process of obtaining a new surface water source. Preliminary assessment of the potential for acquiring new rights may be based on surface water availability maps located on the TWDB website. Where water rights appear to be available, the following activities need to occur:

- Discussions with TCEQ to indicate the likelihood of obtaining those rights. The TCEQ may use the Water Availability Model (WAM) to assist in the determination.
- Discussions with land owners to indicate potential treatment plant locations.
- Coordination with US Army Corps of Engineers and local river authorities.
- Preliminary engineering design to determine the feasibility, costs, and environmental issues of a new treatment plant.

Should these discussions indicate that a new surface water source is the best option, the community would proceed with more intensive planning (initially obtaining funding), permitting, land acquisition, and detailed designs.

1.4.4 Identification of Treatment Technologies

Various treatment technologies were also investigated as compliance alternatives for treatment of nitrate and arsenic to regulatory levels (*i.e.*, MCLs). Numerous options have been identified by the USEPA as best available technologies (BAT) for non-compliant constituents. Identification and descriptions of the various BATs are provided in the following sections.

1.4.4.1 Treatment Technologies for Nitrate

The MCL for nitrate (as nitrogen) was set at 10 mg/L by the USEPA on January 30, 1992, as part of the Phase II Rules, and became effective on July 30, 1992 (USEPA 2007c). This MCL applies to all community water systems, regardless of size.

BATs identified by USEPA for removal of nitrates include:

- Reverse Osmosis (RO);
- Ion Exchange (IX); and
- Electrodialysis Reversal (EDR).

1.4.4.2 Treatment Technologies for Arsenic

In January 2001, the USEPA published a final rule in the Federal Register that established an MCL for arsenic of 0.010 mg/L (USEPA 2001). The regulation applies to all community water systems and non-transient, non-community water systems, regardless of size.

The new arsenic MCL of 0.010 mg/L becomes effective January 23, 2006, at which time the running average annual arsenic level must be at or below 0.010 mg/L at each entry point to the distribution system, although point-of-use (POU) treatment can be instituted in place of centralized treatment. All surface water systems must complete initial monitoring for the new arsenic MCL or have a state-approved waiver by December 31, 2006. All groundwater systems must complete initial monitoring or have a state-approved waiver by December 31, 2007.

Various treatment technologies were investigated as compliance alternatives for treatment of arsenic to regulatory levels (*i.e.,* MCL). According to a recent USEPA report for small water systems with less than 10,000 customers (EPA/600/R-05/001) a number of drinking water treatment technologies are available to reduce arsenic concentrations in source water to below the new MCL of 0.010 mg/L, including:

- IX;
- RO;
- EDR;
- Adsorption; and
- Coagulation/filtration.

1.4.5 Treatment Technologies Description

Reverse Osmosis, IX, and EDR are identified by USEPA as BATs for removal of nitrates. These three treatment technologies are also applicable to arsenic, and are the only three technologies common to both nitrate and arsenic treatment. In this case, IX is not a feasible technology because of the high total dissolved solids (TDS) of the groundwater. A description of these technologies follows

1.4.5.1 Reverse Osmosis

Process**.** RO is a physical process in which contaminants are removed by applying pressure on the feed water to force it through a semi-permeable membrane. RO membranes reject ions based on size and electrical charge. The raw water is typically called feed; the product water is called permeate; and the concentrated reject is called concentrate. Common RO membrane materials include asymmetric cellulose acetate (CA) or polyamide thin film composite (TFC). The TFC membrane operates at much lower pressure and can achieve higher salt rejection than the CA membranes but is less chlorine resistant. Common membrane construction includes spiral wound or hollow fine fiber. Each material and construction method has specific benefits and limitations depending on the raw water characteristics and pre-treatment. Spiral wound has been the dominant membrane type in typical RO systems. A newer, lower pressure type membrane that is similar in operation to spiral wound RO, is nanofiltration (NF), which has higher rejection for divalent ions than mono-valent ions. NF is sometimes used instead of RO for treating water with high hardness and sulfate concentrations. A typical RO installation includes a high pressure feed pump; parallel first and second stage membrane elements (in pressure vessels); and valves and piping for feed, permeate, and concentrate streams. Factors influencing membrane selection are cost, recovery, rejection, raw water characteristics, and pre-treatment. Factors influencing performance are raw water characteristics, pressure, temperature, and regular monitoring and maintenance. Depending on the membrane type and operating pressure, RO is capable of removing 85-95 percent of fluoride, and over 95 percent of nitrate and arsenic. The treatment process is relatively insensitive to pH. Water recovery is 60-80 percent, depending on raw water characteristics. The concentrate volume for disposal can be significant. The conventional RO treatment train for well water uses anti-scalant addition, cartridge filtration, RO membranes, chlorine disinfection, and clearwell storage.

Pre-treatment. RO requires careful review of raw water characteristics, and pre-treatment needs to prevent membranes from fouling, scaling, or other membrane degradation. Removal or sequestering of suspended solids is necessary to prevent colloidal and bio-fouling, and removal of sparingly soluble constituents such as calcium, magnesium, silica, sulfate, barium, *etc*., may be required to prevent scaling. Pretreatment can include media filters to remove suspended particles; IX softening to remove hardness; antiscalant feed; temperature and pH adjustment to maintain efficiency; acid to prevent scaling and membrane damage; activated carbon or bisulfite to remove chlorine (post-disinfection may be required); and cartridge filters to remove any remaining suspended particles to protect membranes from upsets.

Maintenance. Rejection percentages must be monitored to ensure contaminant removal below MCLs. Regular monitoring of membrane performance is necessary to determine fouling, scaling, or other membrane degradation. Use of monitoring equipment to track membrane performance is recommended. Acidic or caustic solutions are regularly flushed through the system at high volume/low pressure with a cleaning agent to remove fouling and scaling. The system is flushed and returned to service. RO stages are cleaned sequentially. Frequency of membrane replacement is dependent on raw water characteristics, pre-treatment, and maintenance.

Waste Disposal. Pre-treatment waste streams, concentrate flows, and spent filters and membrane elements all require approved disposal methods. Disposal of the significant volume of the concentrate stream is a problem for many utilities.

Advantages (RO)

- Produces the highest water quality.
- Can effectively treat a wide range of dissolved salts and minerals, turbidity, health and aesthetic contaminants, and certain organics. Some highly-maintained units are capable of treating biological contaminants.
- Low pressure - less than 100 pounds per square inch (psi), compact, self-contained, single membrane units are available for small installations.

Disadvantages (RO)

- Relatively expensive to install and operate.
- Frequent membrane monitoring and maintenance; pressure, temperature, and pH requirements to meet membrane tolerances. Membranes can be chemically sensitive.
- Additional water usage depending on rejection rate.

A concern with RO for treatment of inorganics is that if the full stream is treated, then most of the alkalinity and hardness would also be removed. In that event, post-treatment may be necessary to avoid corrosion problems. If feasible, a way to avoid this issue is to treat a slip stream of raw water and blend the slip stream back with the raw water rather than treat the full stream. The amount of water rejected is also an issue with RO. Discharge concentrate can be between 10 and 50 percent of the influent flow.

1.4.5.2 Electrodialysis Reversal

Process. EDR is an electrochemical process in which ions migrate through ion-selective semi-permeable membranes as a result of their attraction to two electrically charged electrodes. A typical EDR system includes a membrane stack with a number of cell pairs, each consisting of a cation transfer membrane, a demineralized flow spacer, an anion transfer membrane, and a concentrate flow spacer. Electrode compartments are at opposite ends of the stack. The influent feed water (chemically treated to prevent precipitation) and the concentrated reject flow in parallel across the membranes and through the demineralized and concentrate flow spacers, respectively. The electrodes are continually flushed to reduce fouling or scaling. Careful consideration of flush feed water is required. Typically, the membranes are cation or anion exchange resins cast in sheet form; the spacers are high density polyethylene; and the electrodes are inert metal. EDR stacks are tank-contained and often staged. Membrane selection is based on review of raw water characteristics. A single-stage EDR system usually removes 40-50 percent of fluoride, nitrate, arsenic, and TDS.

Additional stages are required to achieve higher removal efficiency (85-95% for fluoride). EDR uses the technique of regularly reversing the polarity of the electrodes, thereby freeing accumulated ions on the membrane surface. This process requires additional plumbing and electrical controls, but it increases membrane life, may require less added chemicals, and eases cleaning. The conventional EDR treatment train typically includes EDR membranes, chlorine disinfection, and clearwell storage. Treatment of surface water may also require pre-treatment steps such as raw water pumps, debris screens, rapid mix with addition of an anti-scalant, slow mix flocculator, sedimentation basin or clarifier, and gravity filters. Microfiltration (MF) could be used in place of flocculation, sedimentation, and filtration. Additional treatment or management of the concentrate and the removed solids would be necessary prior to disposal.

Pre-treatment**.** There are pretreatment requirements for pH, organics, turbidity, and other raw water characteristics. EDR typically requires chemical feed to prevent scaling, acid addition for pH adjustment, and a cartridge filter for prefiltration.

Maintenance. EDR membranes are durable, can tolerate a pH range from 1 to 10, and temperatures to 115 degrees Fahrenheit (^{o}F) for cleaning. They can be removed from the unit and scrubbed. Solids can be washed off by turning the power off and letting water circulate through the stack. Electrode washes flush out byproducts of electrode reaction. The byproducts are hydrogen, formed in the cathode space, and oxygen and chlorine gas, formed in the anode space. If the chlorine is not removed, toxic chlorine gas may form. Depending on raw water characteristics, the membranes would require regular maintenance or replacement. EDR requires reversing the polarity. Flushing at high volume/low pressure continuously is required to clean electrodes. If used, pre-treatment filter replacement and backwashing would be required. The EDR stack must be disassembled, mechanically cleaned, and reassembled at regular intervals.

Waste Disposal. Highly concentrated reject flows, electrode cleaning flows, and spent membranes require approved disposal methods. Pre-treatment processes and spent materials also require approved disposal methods.

Advantages (EDR)

- EDR can operate with minimal fouling or scaling, or chemical addition.
- Low pressure requirements; typically quieter than RO.
- Long membrane life expectancy; EDR extends membrane life and reduces maintenance.
- More flexible than RO in tailoring treated water quality requirements.

Disadvantages (EDR)

- Not suitable for high levels of iron, manganese, and hydrogen sulfide.
- High energy usage for high TDS water.

EDR can be quite expensive to run because of the energy it uses. However, because it is generally automated and allows for part-time operation, it may be an appropriate technology for small systems. It can be used to simultaneously reduce fluoride, selenium, nitrate, arsenic and TDS.

1.4.6 Point-of-Entry and Point-of-Use Treatment Systems

Point of entry treatment, while a possible alternative for residences, was not considered for Roosevelt ISD, since the large demands for the school connections would require treatment units similar in size to central treatment units. Similarly, a POU alternative was not considered for Roosevelt ISD due to the difficulty in providing POU units for all possible drinking water taps.

1.4.7 Water Delivery or Central Drinking Water Dispensers

Water delivery and central drinking water dispensers were not considered viable alternatives for a school application.

SECTION 2
EVALUATION METHODS

2.1 DECISION TREE

The decision tree is a flow chart for conducting feasibility studies for a non-compliant PWS. The decision tree is shown in Figures 2.1 through 2.4. The tree guides the user through a series of phases in the design process. Figure 2.1 shows Tree 1, which outlines the process for defining the existing system parameters, followed by optimizing the existing treatment system operation. If optimizing the existing system does not correct the deficiency, the tree leads to six alternative preliminary branches for investigation. The groundwater branch leads through investigating existing wells to developing a new well field. The treatment alternatives address centralized and on-site treatment. The objective of this phase is to develop conceptual designs and cost estimates for the six types of alternatives. The work done for this report follows through Tree 1 and Tree 2, as well as a preliminary pass through Tree 4.

Tree 3, which begins at the conclusion of the work for this report, starts with a comparison of the conceptual designs, selecting the two or three alternatives that appear to be most promising, and eliminating those alternatives which are obviously infeasible. It is envisaged that a process similar to this would be used by the study PWS to refine the list of viable alternatives. The selected alternatives are then subjected to intensive investigation, and highlighted by an investigation into the socio-political aspects of implementation. Designs are further refined and compared, resulting in the selection of a preferred alternative. The steps for assessing the financial and economic aspects of the alternatives (one of the steps in Tree 3) are given in Tree 4 in Figure 2.4.

2.2 DATA SOURCES AND DATA COLLECTION

2.2.1 Data Search

2.2.1.1 Water Supply Systems

The TCEQ maintains a set of files on public water systems, utilities, and districts at its headquarters in Austin, Texas. The files are organized under two identifiers: a PWS identification number and a Certificate of Convenience and Necessity (CCN) number. The PWS identification number is used to retrieve four types of files:

- CO – Correspondence,
- CA – Chemical analysis,
- MOR – Monthly operating reports (quality/quantity), and
- FMT – Financial, managerial and technical issues.

Figure 2.1
TREE 1 – EXISTING FACILITY ANALYSIS

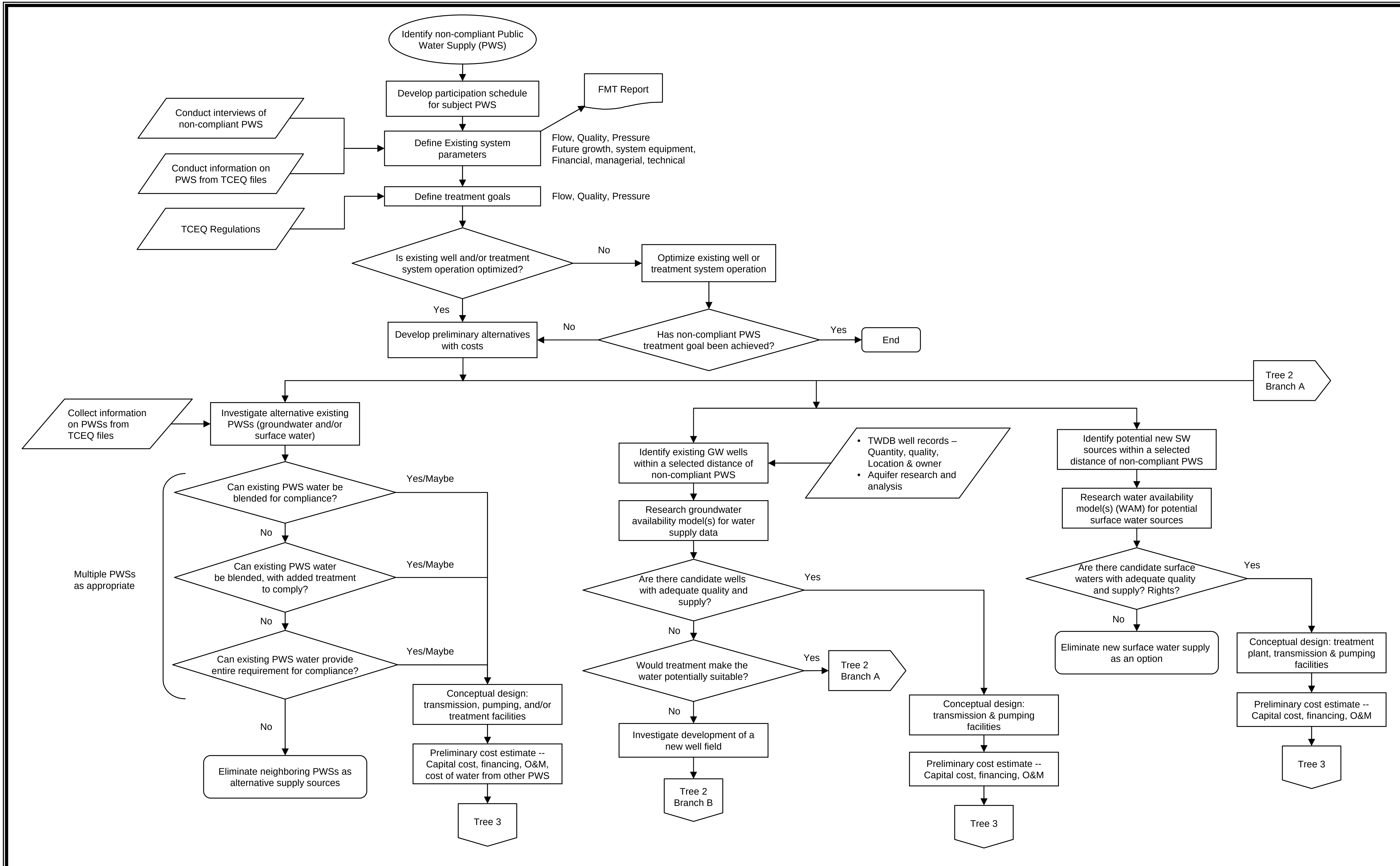
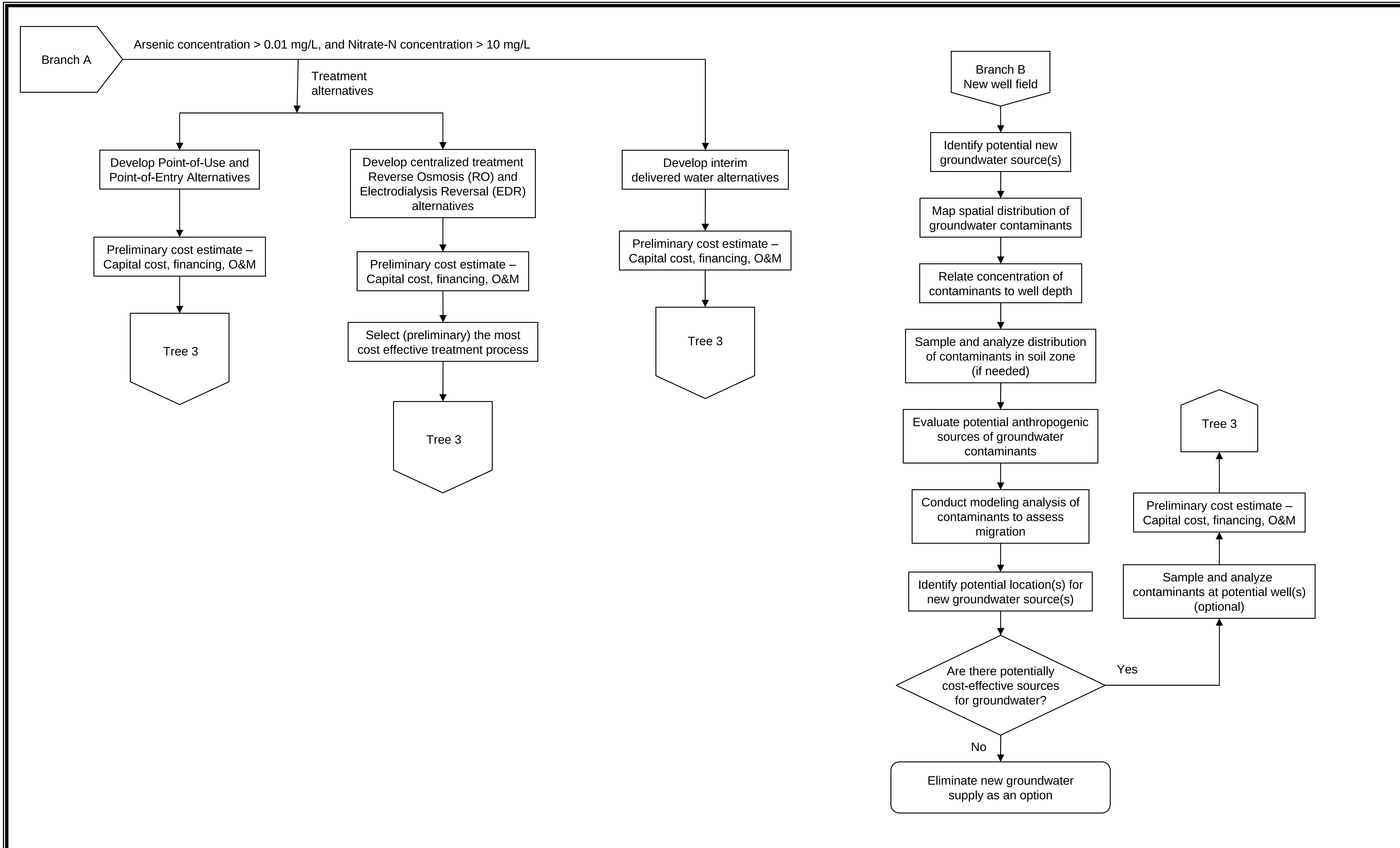


Figure 2.2
TREE 2 – DEVELOP TREATMENT ALTERNATIVES



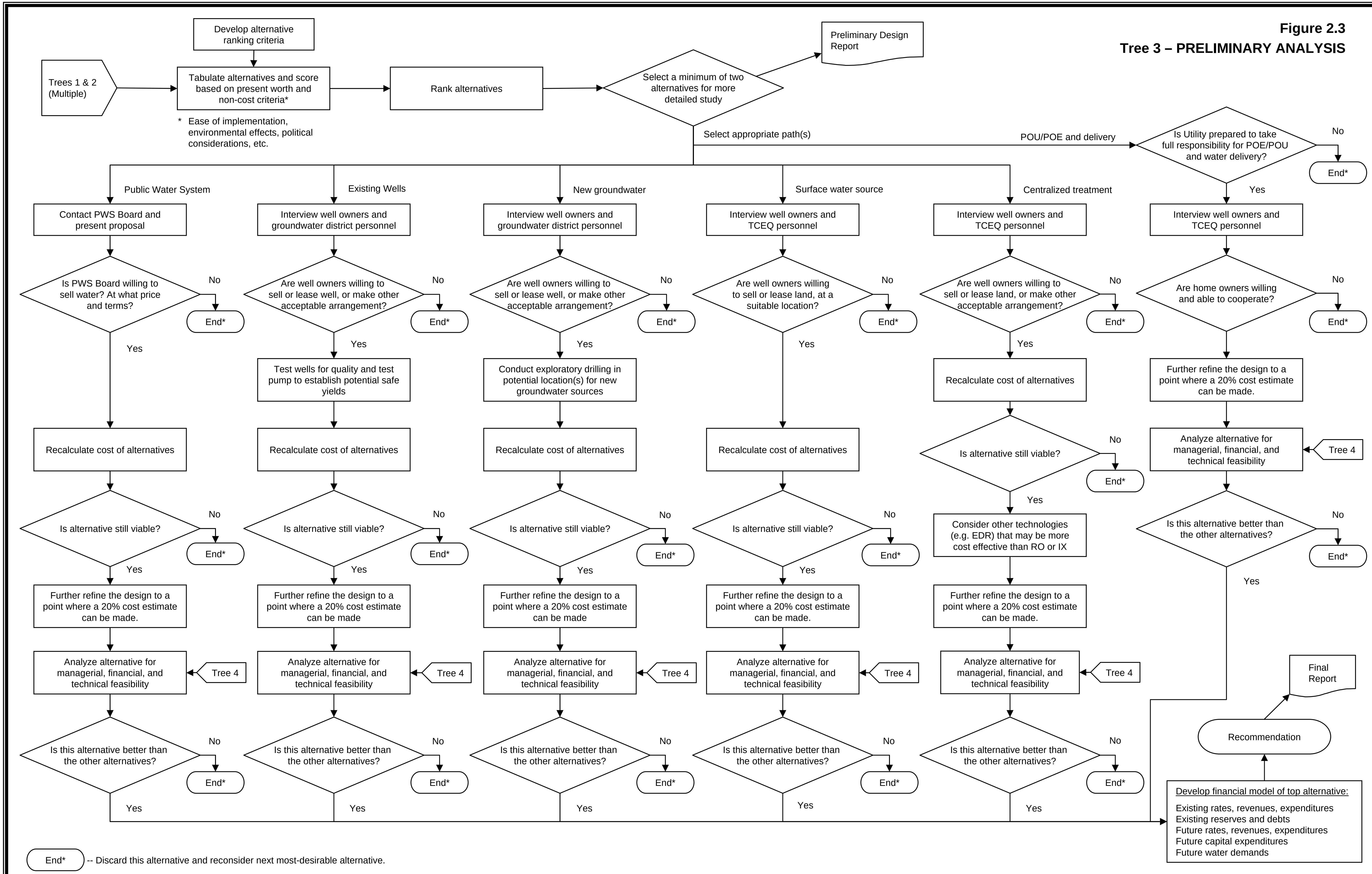
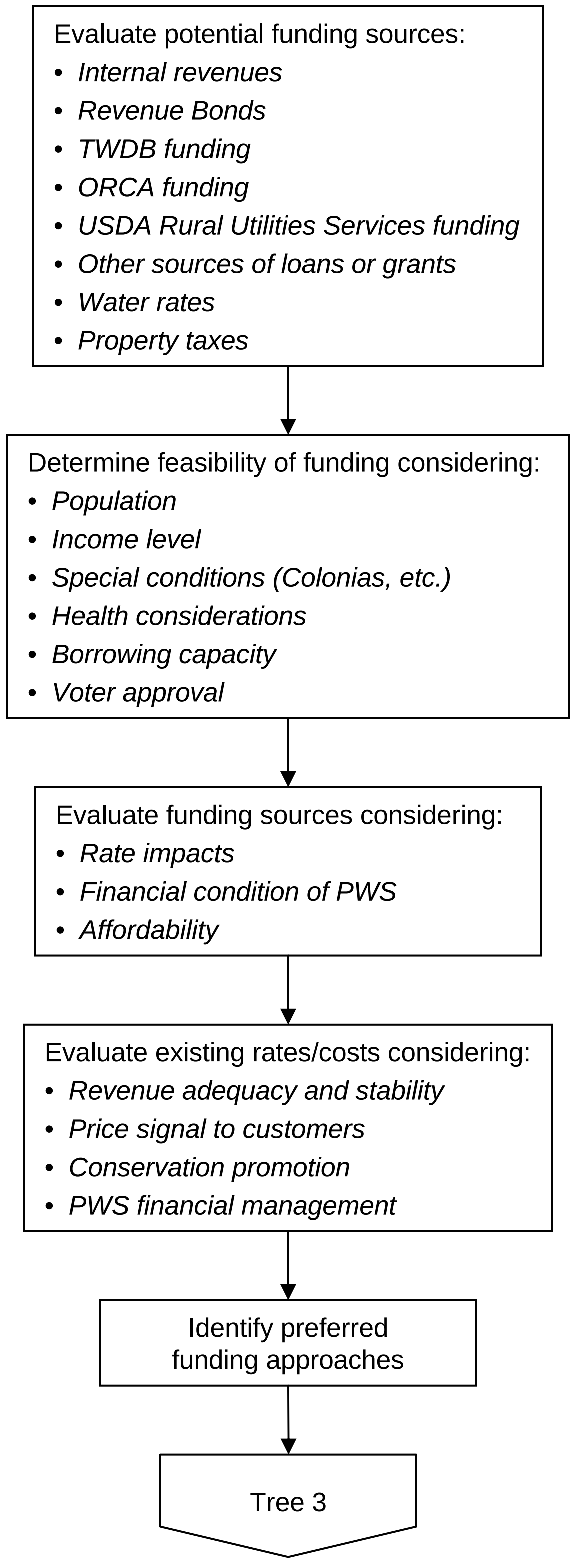


Figure 2.3
Tree 3 – PRELIMINARY ANALYSIS

Figure 2.4
TREE 4 – FINANCIAL



The CCN files generally contain a copy of the system's Certificate of Convenience and Necessity, along with maps and other technical data.

These files were reviewed for the PWS and surrounding systems.

The following websites were consulted to identify the water supply systems in the area:

- Texas Commission on Environmental Quality http://www3.tceq.state.tx.us/iwud/. Under "Advanced Search," type in the name(s) of the County(ies) in the area to get a listing of the public water supply systems.
- USEPA Safe Drinking Water Information System www.epa.gov/safewater/data/getdata.html

Groundwater Control Districts were identified on the TWDB web site, which has a series of maps covering various groundwater and surface water subjects. One of those maps shows groundwater control districts in the State of Texas.

2.2.1.2 Existing Wells

The TWDB maintains a groundwater database available at www.twdb.state.tx.us that has two tables with helpful information. The "Well Data Table" provides a physical description of the well, owner, location in terms of latitude and longitude, current use, and for some wells, items such as flowrate, and nature of the surrounding formation. The "Water Quality Table" provides information on the aquifer and the various chemical concentrations in the water.

The TWDB maintains a groundwater database available at www.twdb.state.tx.us that has two tables with helpful information. The "Well Data Table" provides a physical description of the well, owner, location in terms of latitude and longitude, current use, and for some wells, items such as flowrate, and nature of the surrounding formation. The "Water Quality Table" provides information on the aquifer and the various chemical concentrations in the water. For this project, it was assumed that the nitrate concentration given in this database was the concentration of nitrate, with a molecular weight of 62. To convert to the same basis used for the MCL (Nitrate-N), the value given in the TWDB database was divided by 4.5.

2.2.1.3 Surface Water Sources

Regional planning documents were consulted for lists of surface water sources.

2.2.1.4 Groundwater Availability Model

GAMs, developed by the TWDB, are planning tools and should be consulted as part of a search for new or supplementary water sources. The GAM for the Ogallala aquifer was investigated as a potential tool for identifying available and suitable groundwater resources.

2.2.1.5 Water Availability Model

The WAM is a computer-based simulation predicting the amount of water that would be in a river or stream under a specified set of conditions. WAMs are used to determine whether water would be available for a newly requested water right or amendment. If water is available, these models estimate how often the applicant could count on water under various conditions (*e.g.*, whether water would be available only 1 month out of the year, half the year, or all year, and whether that water would be available in a repeat of the drought of record).

WAMs provide information that assist TCEQ staff in determining whether to recommend the granting or denial of an application.

2.2.1.6 Financial Data

Financial data were collected through a site visit. Data sought included:

- Annual Budget
- Audited Financial Statements
    - Balance Sheet
    - Income & Expense Statement
    - Cash Flow Statement
    - Debt Schedule
- Water Rate Structure
- Water Use Data
    - Production
    - Billing
    - Customer Counts

2.2.1.7 Demographic Data

Basic demographic data were collected from the 2000 Census to establish incomes and eligibility for potential low cost funding for capital improvements. Median household income (MHI) and number of families below poverty level were the primary data points of significance. If available, MHI for the customers of the PWS should be used. In addition, unemployment data were collected from current U.S. Bureau of Labor Statistics. These data were collected for the following levels: national, state, and county.

2.2.2 PWS Interviews

2.2.2.1 PWS Capacity Assessment Process

A capacity assessment is the industry standard term for an evaluation of a water system's financial, managerial, and technical capacity to effectively deliver safe drinking water to its customers now and in the future at a reasonable cost, and to achieve, maintain and plan for compliance with applicable regulations. The assessment process involves interviews with staff and management who have a responsibility in the operations and management of the system.

Financial, managerial, and technical capacity are individual yet highly interrelated components of a system's capacity. A system cannot sustain capacity without maintaining adequate capability in all three components.

Financial capacity is a water system's ability to acquire and manage sufficient financial resources to allow the system to achieve and maintain compliance with SDWA regulations. Financial capacity refers to the financial resources of the water system, including but not limited to revenue sufficiency, credit worthiness, and fiscal controls.

Managerial capacity is the ability of a water system to conduct its affairs so that the system is able to achieve and maintain compliance with SDWA requirements. Managerial capacity refers to the management structure of the water system, including but not limited to ownership accountability, staffing and organization, and effective relationships to customers and regulatory agencies.

Technical capacity is the physical and operational ability of a water system to achieve and maintain compliance with the SDWA regulations. It refers to the physical infrastructure of the water system, including the adequacy of the source water, treatment, storage and distribution infrastructure. It also refers to the ability of system personnel to effectively operate and maintain the system and to otherwise implement essential technical knowledge.

Many aspects of water system operations involve more than one component of capacity. Infrastructure replacement or improvement, for example, requires financial resources, management planning and oversight, and technical knowledge. A deficiency in any one area could disrupt the entire effort. A system that is able to meet both its immediate and long-term challenges demonstrates that it has sufficient financial, managerial, and technical capacity.

Assessment of the FMT capacity of the PWS was based on an approach developed by the New Mexico Environmental Finance Center (NMEFC), which is consistent with TCEQ FMT assessment process. This method was developed from work the NMEFC did while assisting USEPA Region 6 in developing and piloting groundwater comprehensive performance evaluations. The NMEFC developed a standard list of questions that could be asked of water system personnel. The list was then tailored slightly to have two sets of questions – one for managerial and financial personnel, and one for operations personnel (the questions are included in Appendix A). Each person with a role in the FMT capacity of the system was asked the applicable standard set of questions individually. The interviewees were not given the questions in advance and were not told the answers others provided. Also, most of the questions are open ended type questions so they were not asked in a fashion to indicate what would be the "right" or "wrong" answer. The interviews lasted between 45 minutes to 75 minutes depending on the individual's role in the system and the length of the individual's answers.

In addition to the interview process, visual observations of the physical components of the system were made. A technical information form was created to capture this information. This form is also contained in Appendix A. This information was considered supplemental to the interviews because it served as a check on information provided in the interviews. For example, if an interviewee stated he or she had an excellent preventative maintenance schedule and the visit to the facility indicated a significant amount of deterioration (more than would be expected for the age of the facility) then the preventative maintenance program could be further investigated or the assessor could decide that the preventative maintenance program was inadequate.

Following interviews and observations of the facility, answers that all personnel provided were compared and contrasted to provide a clearer picture of the true operations at the water system. The intent was to go beyond simply asking the question, "Do you have a budget?" to actually finding out if the budget was developed and being used appropriately. For example, if a water system manager was asked the question, "Do you have a budget?" he or she may say, "yes" and the capacity assessor would be left with the impression that the system is doing well in this area. However, if several different people are asked about the budget in more detail, the assessor may find that although a budget is present, operations personnel do not have input into the budget, the budget is not used by the financial personnel, the budget is not updated regularly, or the budget is not used in setting or evaluating rates. With this approach, the inadequacy of the budget would be discovered and the capacity deficiency in this area would be noted.

Following the comparison of answers, the next step was to determine which items noted as a potential deficiency truly had a negative effect on the system's operations. If a system had what appeared to be a deficiency, but this deficiency was not creating a problem in terms of the operations or management of the system, it was not considered critical and may not have needed to be addressed as a high priority. As an example, the assessment may have revealed an insufficient number of staff members to operate the facility. However, it may also have been revealed that the system was able to work around that problem by receiving assistance from a neighboring system, so no severe problems resulted from the number of staff members. Although staffing may not be ideal, the system does not need to focus on this particular issue. The system needs to focus on items that are truly affecting operations. As an example of this type of deficiency, a system may lack a reserve account which can then lead the system to delay much-needed maintenance or repair on its storage tank. In this case, the system needs to address the reserve account issue so that proper maintenance can be completed.

The intent was to develop a list of capacity deficiencies with the greatest impact on the system's overall capacity. Those were the most critical items to address through follow-up technical assistance or by the system itself.

2.2.2.2 Interview Process

PWS personnel were interviewed by the project team, and each was interviewed separately. Interview forms were completed during each interview.

2.3 ALTERNATIVE DEVELOPMENT AND ANALYSIS

The initial objective for developing alternatives to address compliance issues is to identify a comprehensive range of possible options that can be evaluated to determine which are the most promising for implementation. Once the possible alternatives are identified, they must be defined in sufficient detail so a conceptual cost estimate (capital and operation and maintenance [O&M] costs) can be developed. These conceptual cost estimates are used to compare the affordability of compliance alternatives, and to give a preliminary indication of rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. The basis for the unit costs used for the compliance alternative cost estimates is summarized in Appendix B. Other non-economic factors for the alternatives, such as reliability and ease of implementation, are also addressed

2.3.1 Existing PWS

The neighboring PWSs were identified, and the extents of their systems were investigated. PWSs farther than 15 miles from the non-compliant PWSs were not considered because the length of the pipeline required would make the alternative cost prohibitive. The quality of water provided was also investigated. For neighboring PWSs with compliant water, options for water purchase and/or expansion of existing well fields were considered. The neighboring PWSs with non-compliant water were considered as possible partners in sharing the cost for obtaining compliant water either through treatment or developing an alternate source.

The neighboring PWSs were investigated to get an idea of the water sources in use and the quantity of water that might be available for sale. They were contacted to identify key locations in their systems where a connection might be made to obtain water and to explore on a preliminary basis their willingness to partner or sell water. Then, the major system components that would be required to provide compliant water were identified. The major system components included treatment units, wells, storage tanks, pump stations, and pipelines.

Once the major components were identified, a preliminary design was developed to identify sizing requirements and routings. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.2 New Groundwater Source

It was not possible in the scope of this project to determine conclusively whether new wells could be installed to provide compliant drinking water. In order to evaluate potential new groundwater source alternatives, three test cases were developed based on distance from the PWS intake point. The test cases were based on distances of 10 miles, 5 miles, and 1 mile. It was assumed that a pipeline would be required for all three test cases. A storage tank and pump station would be required for the 10-mile and 5-mile alternatives. It was also assumed that new wells would be installed, and that their depths would be similar to the depths of the existing wells, or other existing drinking water wells in the area.

A preliminary design was developed to identify sizing requirements for the required system components. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change (*i.e.*, from current expenditures) in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.3 New Surface Water Source

New surface water sources were investigated. Availability of adequate quality water was investigated for the main rivers in the area, as well as the major reservoirs. TCEQ WAMs were inspected, and the WAM was run, where appropriate.

2.3.4 Treatment

Treatment technologies considered potentially applicable to both nitrate and arsenic removal are RO, IX, and EDR since they are proven technologies with numerous successful installations. However, the system has TDS higher than 1,000 mg/L and thus, IX is not economically feasible. RO treatment is considered for central treatment alternatives. EDR treatment is considered for central treatment alternatives only. Both RO and EDR treatment produce a liquid waste: a reject stream from RO treatment and a concentrate stream from EDR treatment. As a result, the treated volume of water is less than the volume of raw water that enters the treatment system. The amount of raw water used increases to produce the same amount of treated water if RO or EDR treatment is implemented. The treatment units were sized based on flow rates, and capital and annual O&M cost estimates were made based on the size of the treatment equipment required. Neighboring non-compliant PWSs were identified to look for opportunities where the costs and benefits of central treatment could be shared between systems.

Non-economic factors were also identified. Ease of implementation was considered, as well as reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.4 COST OF SERVICE AND FUNDING ANALYSIS

The primary purpose of the cost of service and funding analysis is to determine the financial impact of implementing compliance alternatives, primarily by examining the required rate increases, and also the fraction of household income that water bills represent. The current financial situation is also reviewed to determine what rate increases are necessary for the PWS to achieve or maintain financial viability.

2.4.1 Financial Feasibility

A key financial metric is the comparison of average annual household water bill for a PWS customer to the MHI for the area. MHI data from the 2000 Census are used, at the most detailed level available for the community. Typically, county level data are used for small rural water utilities due to small population sizes. Annual water bills are determined for existing, base conditions, including consideration of additional rate increases needed under current conditions. Annual water bills are also calculated after adding incremental capital and operating costs for each of the alternatives to determine feasibility under several potential funding sources.

Additionally, the use of standard ratios provides insight into the financial condition of any business. Three ratios are particularly significant for water utilities:

- Current Ratio = current assets divided by current liabilities provides insight into the ability to meet short-term payments. For a healthy utility, the value should be greater than 1.0.
- Debt to Net Worth Ratio = total debt divided by net worth shows to what degree assets of the company have been funded through borrowing. A lower ratio indicates a healthier condition.
- Operating Ratio = total operating revenues divided by total operating expenses show the degree to which revenues cover ongoing expenses. The value is greater than 1.0 if the utility is covering its expenses.

2.4.2 Median Household Income

The 2000 U.S. Census is used as the basis for MHI. In addition to consideration of affordability, the annual MHI may also be an important factor for sources of funds for capital programs needed to resolve water quality issues. Many grant and loan programs are available to lower income rural areas, based on comparisons of local income to statewide incomes. In the 2000 Census, MHI for the State of Texas was $39,927, compared to the U.S. level of $41,994. The census broke down MHIs geographically by block group and ZIP code. The MHIs can vary significantly for the same location, depending on the geographic subdivision chosen. The MHI for each PWS was estimated by selecting the most appropriate value based on block group or ZIP code based on results of the site interview and a comparison with the surrounding area.

2.4.3 Annual Average Water Bill

The annual average household water bill was calculated for existing conditions and for future conditions incorporating the alternative solutions. Average residential consumption is estimated and applied to the existing rate structure to estimate the annual water bill. The estimates are generated from a long-term financial planning model that details annual revenue, expenditure, and cash reserve requirements over a 30-year period.

2.4.4 Financial Plan Development

The financial planning model uses available data to establish base conditions under which the system operates. The model includes, as available:

- Accounts and consumption data
- Water tariff structure
- Beginning available cash balance
- Sources of receipts:
    - Customer billings
    - Membership fees
    - Capital Funding receipts from:
        - Grants
        - Proceeds from borrowing
- Operating expenditures:
    - Water purchases
    - Utilities
    - Administrative costs

  - Salaries
- Capital expenditures
- Debt service:
  - Existing principal and interest payments
  - Future principal and interest necessary to fund viable operations
- Net cash flow
- Restricted or desired cash balances:
  - Working capital reserve (based on 1-4 months of operating expenses)
  - Replacement reserves to provide funding for planned and unplanned repairs and replacements

From the model, changes in water rates are determined for existing conditions and for implementing the compliance alternatives.

2.4.5 Financial Plan Results

Results from the financial planning model are summarized in two areas: percentage of household income and total water rate increase necessary to implement the alternatives and maintain financial viability.

2.4.5.1 Funding Options

Results are summarized in a table that shows the following according to alternative and funding source:

- Percentage of the median annual household income that the average annual residential water bill represents.
- The first year in which a water rate increase would be required
- The total increase in water rates required, compared to current rates

Water rates resulting from the incremental capital costs of the alternative solutions are examined under a number of funding options. The first alternative examined is always funding from existing reserves plus future rate increases. Several funding options were analyzed to frame a range of possible outcomes.

- Grant funds for 100 percent of required capital. In this case, the PWS is only responsible for the associated O&M costs.
- Grant funds for 75 percent of required capital, with the balance treated as if revenue bond funded.
- Grant funds for 50 percent of required capital, with the balance treated as if revenue bond funded.

- SRF loan at the most favorable available rates and terms applicable to the communities.
- Terms of revenue bonds assumed to be 25-year term at 6.0 percent interest rate.

2.4.5.2 General Assumptions Embodied in Financial Plan Results

The basis used to project future financial performance for the financial plan model includes:

- No account growth (either positive or negative).
- No change in estimate of uncollectible revenues over time.
- Average consumption per account unchanged over time.
- No change in unaccounted for water as percentage of total (more efficient water use would lower total water requirements and costs).
- No inflation included in the analyses (although the model has provisions to add escalation of O&M costs, doing so would mix water rate impacts from inflation with the impacts from the alternatives being examined).
- Minimum working capital fund established for each district based on specified months of O&M expenditures.
- O&M for alternatives begins 1 year after capital implementation.
- Balance of capital expenditures not funded from primary grant program is funded through debt (bond equivalent).
- Cash balance drives rate increases, unless provision chosen to override where current net cash flow is positive.

2.4.5.3 Interpretation of Financial Plan Results

Results from the financial plan model are presented in Table 4.4. The table shows the percentage of MHI represented by the annual water bill that results from any rate increases necessary to maintain financial viability over time. In some cases, this may require rate increases even without implementing a compliance alternative (the no action alternative). The table shows any increases such as these separately. The results table shows the total increase in rates necessary, including both the no-action alternative increase and any increase required for the alternative. For example, if the no action alternative requires a 10 percent increase in rates and the results table shows a rate increase of 25 percent, then the impact from the alternative is an increase in water rates of 15 percent. Likewise, the percentage of household income in the table reflects the total impact from all rate increases.

2.4.5.4 Potential Funding Sources

A number of potential funding sources exist for rural utilities. Both state and federal agencies offer grant and loan programs to assist rural communities in meeting their infrastructure needs.

Within Texas, the following state agencies offer financial assistance if needed:

- Texas Water Development Board,
- Office of Rural Community Affairs, and
- Texas Department of Health (Texas Small Towns Environment Program).

Small rural communities can also get assistance from the federal government. The primary agencies providing aid are:

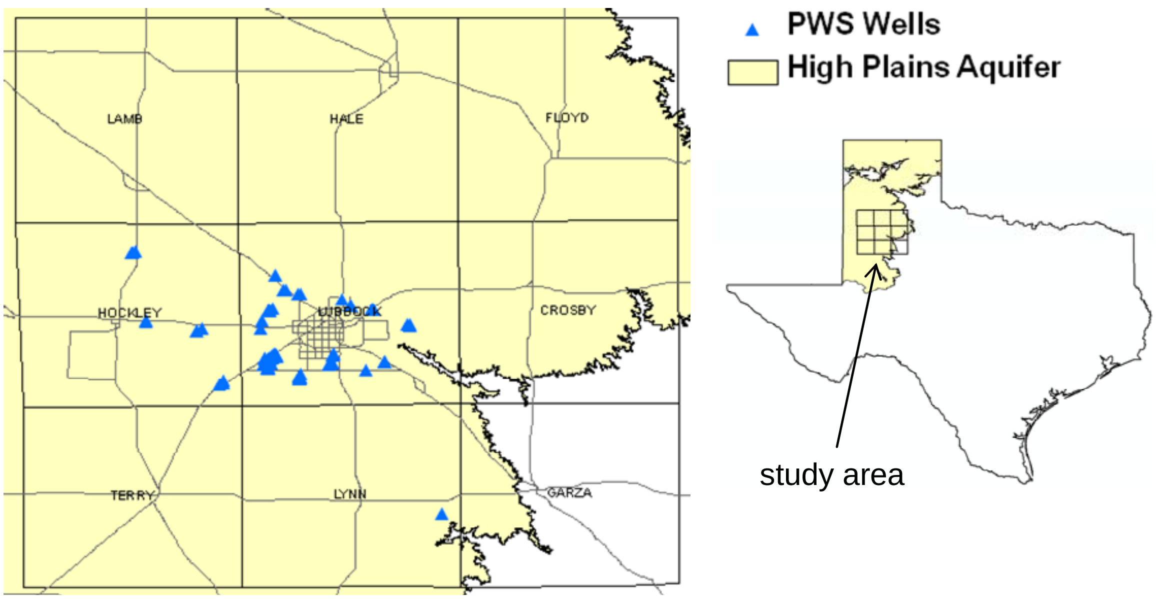
- United States Department of Agriculture, Rural Utilities Service, and
- United States Housing and Urban Development.

SECTION 3
UNDERSTANDING SOURCES OF CONTAMINANTS

3.1 REGIONAL HYDROGEOLOGY

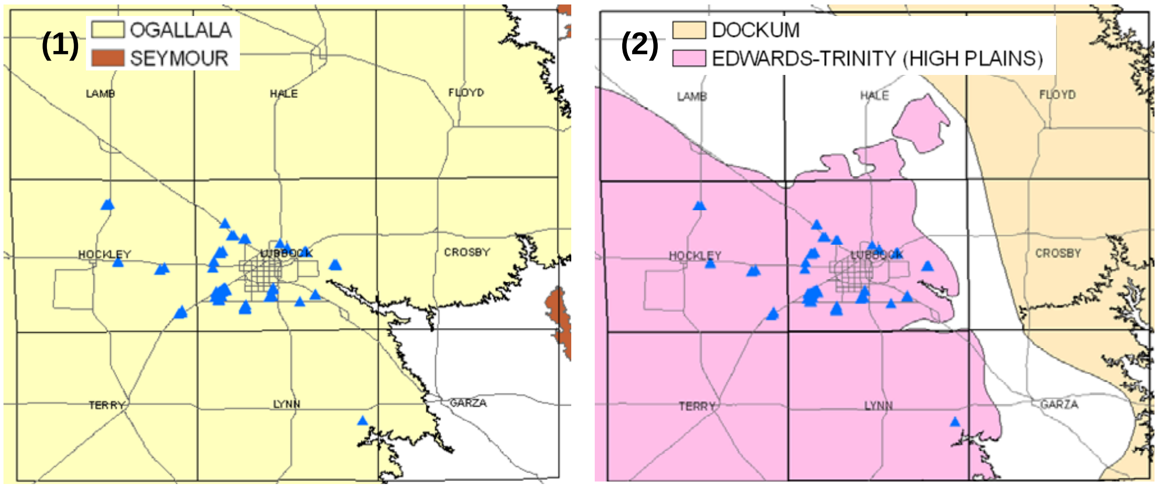
The assessed Public Water Supplies are located in Hockley, Lubbock, and Lynn Counties. For the regional analysis, data from nine counties covering the area around Lubbock were used, including: Lubbock, Lamb, Hale, Floyd, Hockley, Crosby, Terry, Lynn, and Garza Counties (Figure 3.1).

Figure 3.1 Nine Counties Study Area and PWS Well Locations



The major aquifer in the area is the Ogallala of late Tertiary age. Other aquifers in the region that may locally be hydraulically connected to the Ogallala aquifer include younger alluvial/fluvial deposits of Quaternary age (Blackwater Draw Formation) and underlying older aquifers, including the Edwards-Trinity High Plains aquifer of Cretaceous age, the Dockum aquifer of Triassic age, and undifferentiated Permian aquifers. A small pod of the Seymour aquifer is also present in southern Crosby County and northern Garza County (Figure 3.2). The PWS wells of concern are mainly completed in the Ogallala aquifer (one PWS well completed in the Edwards-Trinity High Plains aquifer). Contaminants of concern include fluoride, nitrate, arsenic, selenium, and uranium.

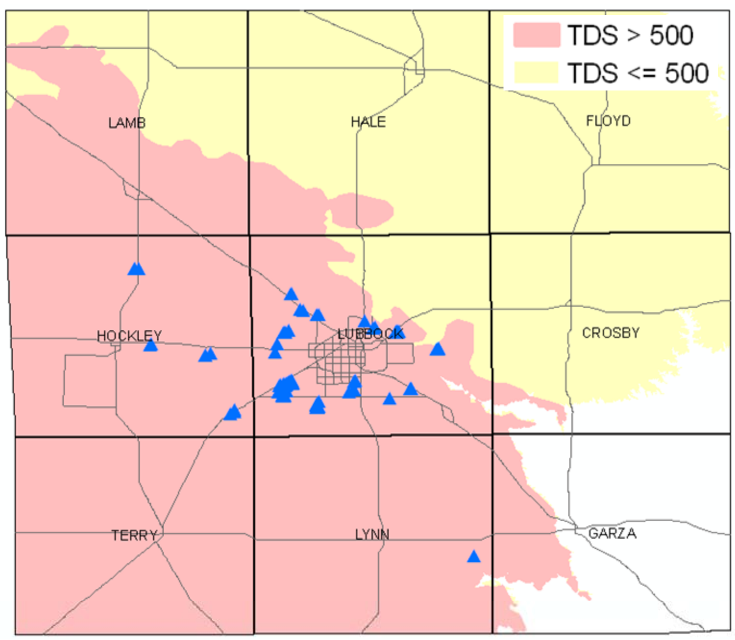
Figure 3.2 Major and Minor Aquifers in the Study Area



(1) Major aquifers include the Ogallala and Seymour aquifers, and (2) minor aquifers include the Edwards-Trinity High Plains and Dockum aquifers

Water quality in the Ogallala aquifer varies greatly between the north-east and south-west parts of the study area (Figure 3.3). Thus, two analysis zones were defined: Ogallala-North (TDS ≤500 mg/L), Ogallala-South (TDS >500 mg/L).

Figure 3.3 Water Quality Zones in the Study Area



Data in the analysis included information from three sources:

- Texas Water Development Board groundwater database available at: https://www.twdb.state.tx.us/DATA/waterwell/well_info.asp. The database includes information on well location, related aquifer, well depth, and groundwater quality information.

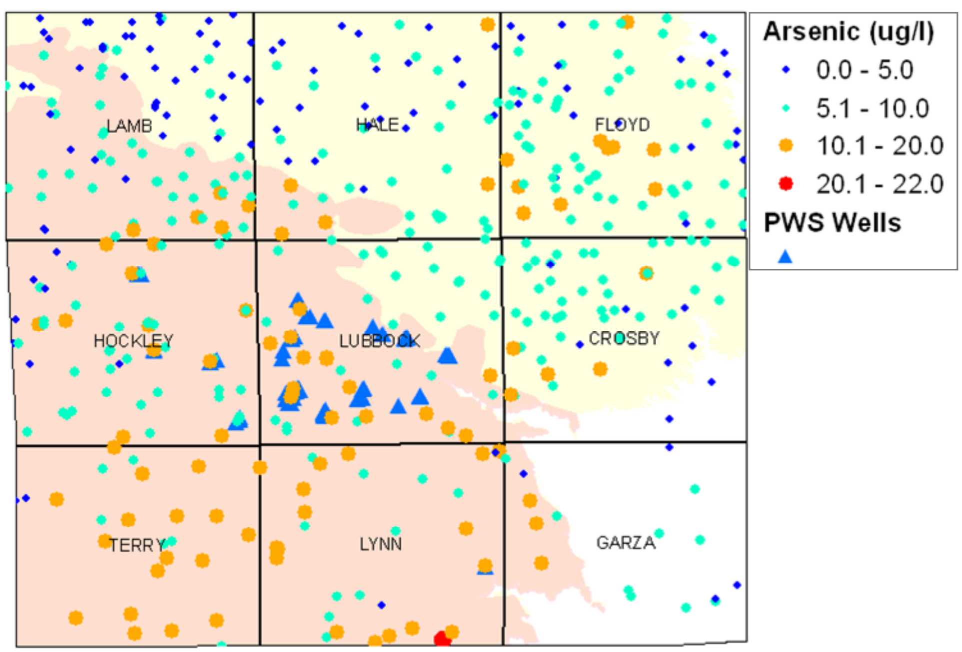
- Texas Commission on Environmental Quality Public Water Supply database (not publicly available). The database includes water quality data collected at PWSs in Texas, and information on the water sources such as location, depth, and related aquifers
- National Uranium Resource Evaluation (NURE) database available at: http://tin.er.usgs.gov/nure/water/. The NURE dataset includes groundwater quality data collected between 1975 and 1980. The database provides well locations, and depths with an array of analyzed chemical data. The NURE dataset covers only the eastern part of the study area.

3.2. CONTAMINANTS OF CONCERN IN THE STUDY AREA

ARSENIC

Arsenic concentrations exceed the MCL (10 µg/L) especially in the Ogallala-South area where 45 percent of the wells show arsenic above the MCL (Figure 3.4). In the Ogallala-North area only 8 percent of the wells have concentrations exceeding the arsenic MCL.

Figure 3.4 Arsenic Concentrations in the Ogallala Aquifer within the Study Area



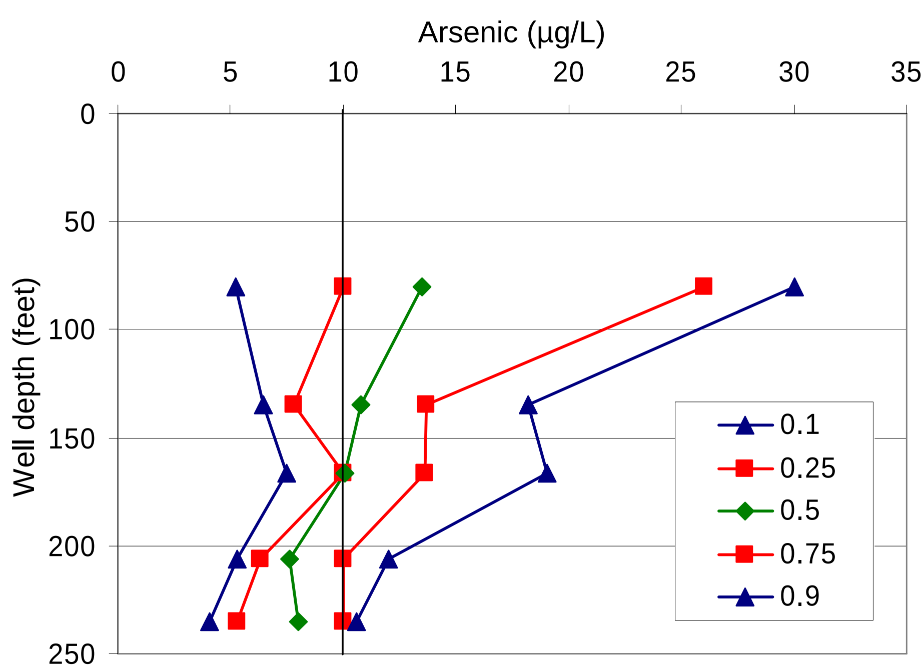
Data are from the TWDB database. The most recent sample for each well is shown. Table 3.1 gives the percentage of wells with arsenic exceeding the MCL in each of the major aquifers in the study area.

Table 3.1 Summary of Arsenic Concentrations by Aquifer

Aquifer	Total number of wells	Arsenic > 10 µg/L	
		Number of wells	Percentage
Ogallala-South	215	96	45%
Ogallala-North	222	17	8%
Edwards-Trinity (High Plains)	11	2	18%
Dockum	28	0	0%
Other	2	0	0%

In the Ogallala-South area where many wells have arsenic concentrations >10 µg/L, there is a stratification of arsenic concentrations with depth, particularly at the higher percentiles (Figure 3.5). Arsenic concentrations decrease with depth, which may suggest that tapping deeper water by deepening shallow wells or screening off shallower parts of certain wells may decrease arsenic concentrations and might provide a solution for wells where arsenic exceeds the MCL.

Figure 3.5 Stratification of Arsenic Concentrations with Depth in the Ogallala-South

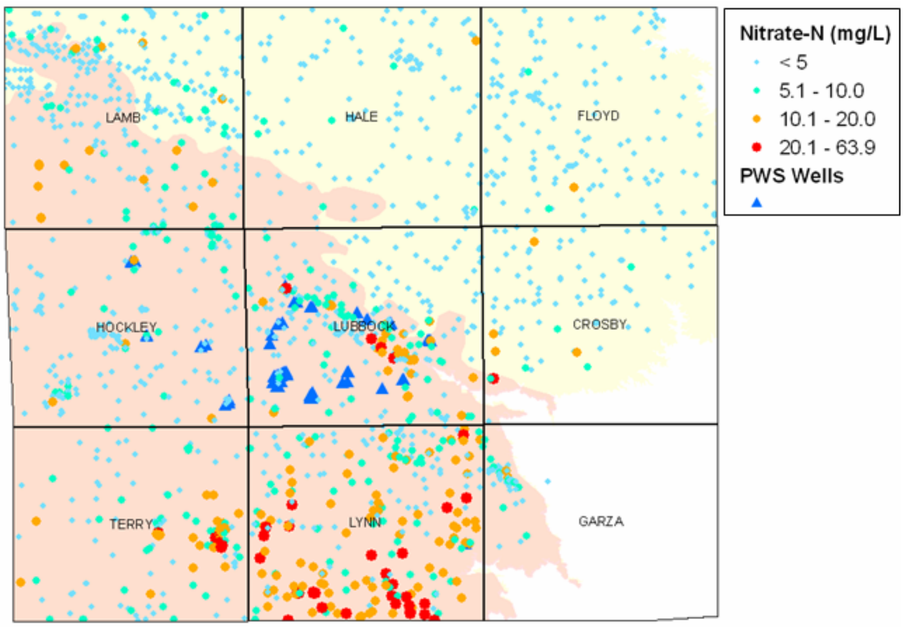


Arsenic concentrations are plotted as the 10th, 25th, 50th, 75th, and 90th percentiles and depths represent the median of 20th percentiles

NITRATE

Nitrate concentrations >10 mg/L nitrate-N (USEPA MCL) are abundant within the study area, especially in the Ogallala-South aquifer where 20 percent of the wells exceed the MCL (Figure 3.6). There is very little nitrate contamination in the Ogallala-North aquifer where only about 2 percent of the wells have nitrate concentrations exceeding the MCL.

Figure 3.6 Nitrate Concentrations in the Ogallala Aquifer within the Study Area



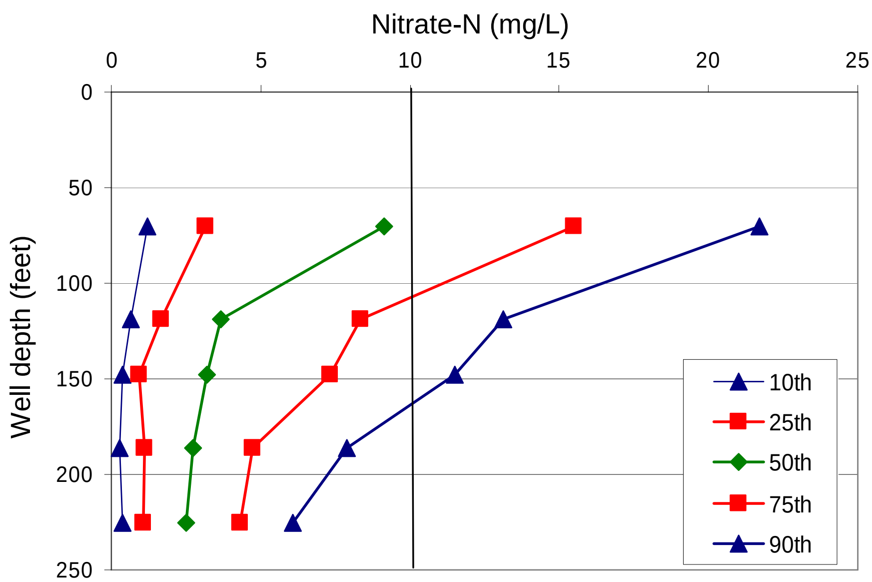
Data are from the TWDB database. The most recent sample for each well in the Ogallala aquifer is shown. Table 3.2 shows the percentage of wells with nitrate-N exceeding the MCL (10 mg/L).

Table 3.2 Summary of Nitrate Concentrations by Aquifer

Aquifer	Total number of wells	Nitrate > 10 mg/L	
		Number of wells	Percentage
Ogallala-South	1026	201	20%
Ogallala-North	580	12	2%
Edwards-Trinity (High Plains)	30	0	0%
Dockum	59	2	3%
Other	23	2	9%

In the Ogallala-South area where many wells have nitrate concentrations >10 mg/L, there is a clear stratification of nitrate-N concentrations with depth, particularly at the higher percentiles (Figure 3.7). Nitrate concentrations decrease with depth. This suggests that tapping deeper water by deepening shallow wells or screening off shallower parts of certain wells may decrease nitrate concentrations and might provide a solution for wells where nitrate exceeds the MCL.

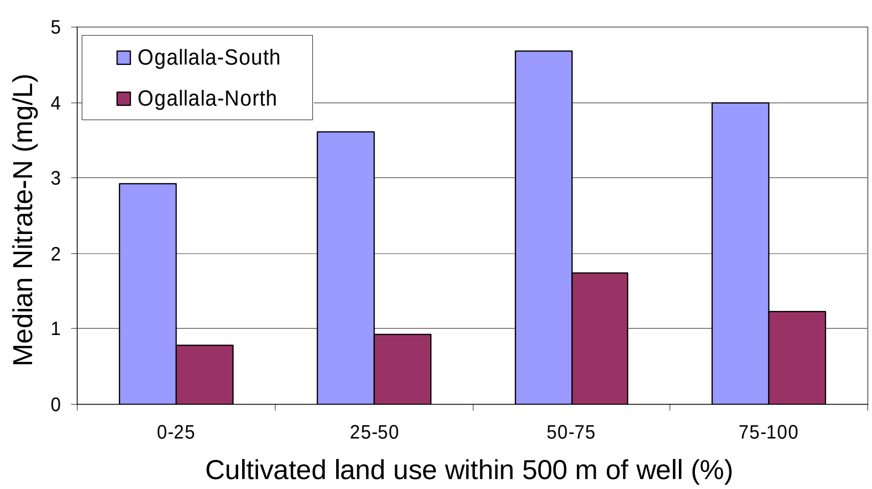
Figure 3.7 Stratification of Nitrate-N Concentrations with Depth in the Ogallala-South



Nitrate concentrations are plotted as the 10th, 25th, 50th, 75th, and 90th percentiles and depths represent the median of 20th percentiles.

Nitrate concentrations are correlated with land use in the study area (Figure 3.8). Median nitrate concentrations were compared with percentage of cultivated land within a 500 m radius around wells. Results indicate that nitrate-N concentrations generally increase with increasing cultivation.

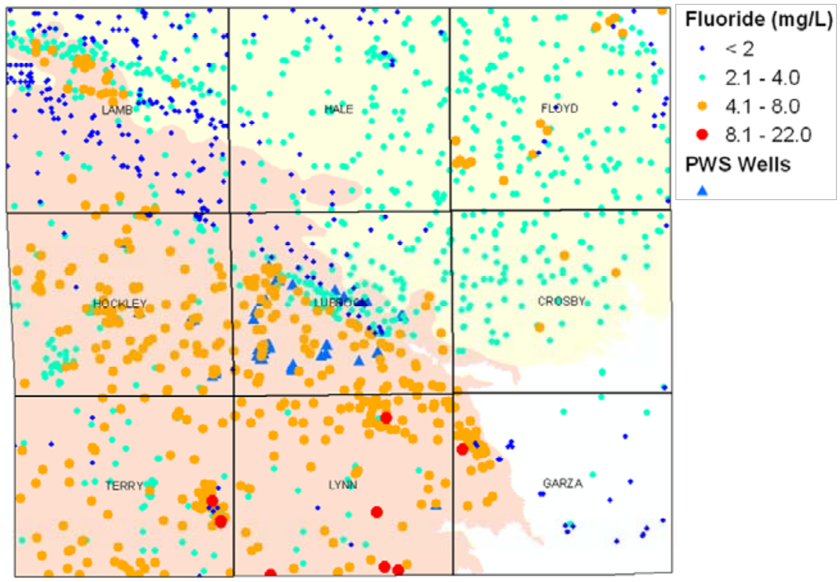
Figure 3.8 Relationship between Nitrate Concentrations and Cultivated Land



FLUORIDE

Fluoride concentrations exceeding the fluoride MCL (4 mg/L) are widespread in the Ogallala-South area (Figure 3.9, 51% of wells) and are low in the Ogallala-North area (3% of wells).

Figure 3.9 Spatial Distribution of Fluoride Concentrations in the Study Area



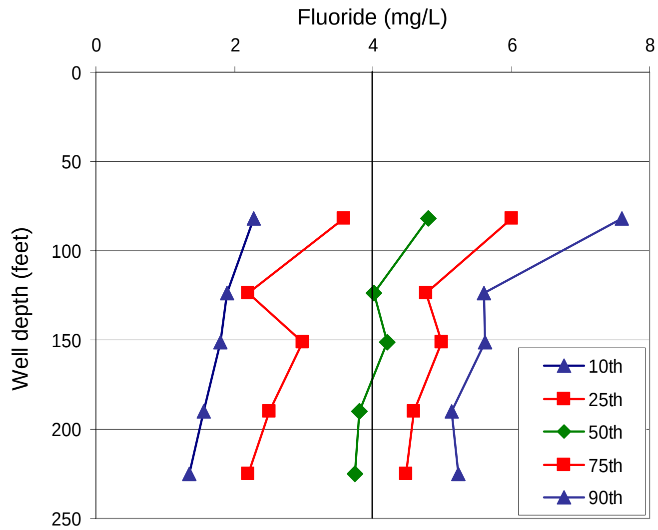
Data are from the TWDB database. The most recent sample for each well is shown. Table 3.3 shows the percentage of wells with fluoride exceeding the MCL (4 mg/L)) by aquifer.

Table 3.3 Summary of Fluoride Concentrations by Aquifer

Aquifer	Total number of wells	Fluoride ≥ 4 mg/L	
		Number of wells	Percentage
Ogallala-South	848	429	51%
Ogallala-North	576	17	3%
Edwards-Trinity (High Plains)	28	9	32%
Dockum	54	2	3%
Other	12	3	25%

In the Ogallala-South area where there are high rate of fluoride concentrations >4 mg/L, there is some stratification of fluoride concentrations with depth. Fluoride concentrations decrease with depth, particularly up to a depth of 125 feet (Figure 3.10). This suggests that tapping deeper water by deepening shallow wells or screening off the shallower parts of certain wells may decrease fluoride concentrations and might provide a solution for wells where fluoride concentrations exceed the MCL.

Figure 3.10 Stratification of Fluoride Concentrations with Depth in the Ogallala-South Area

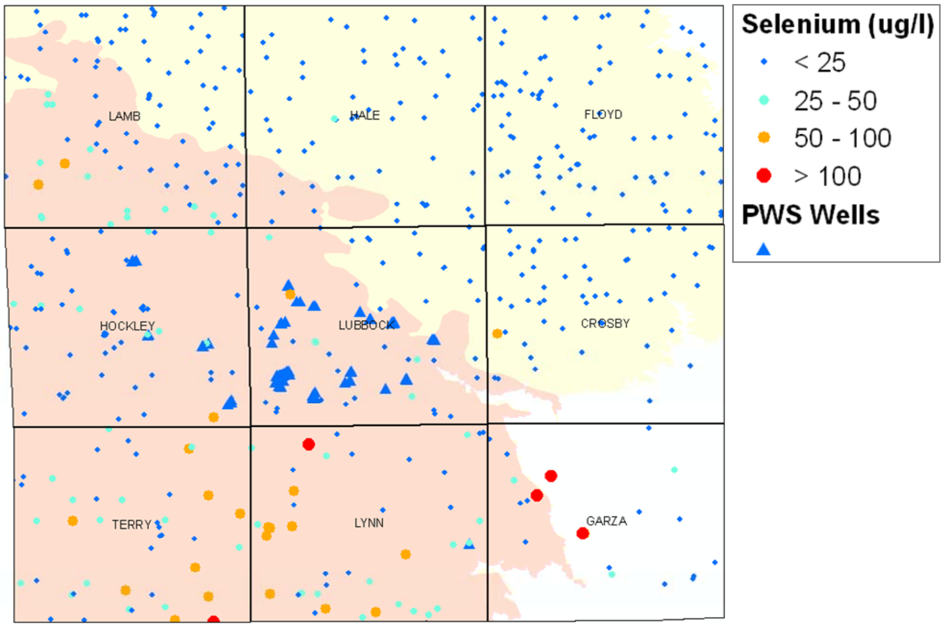


Fluoride concentrations are plotted as the 10th, 25th, 50th, 75th, and 90th percentiles and depths represent the median of 20th percentiles

SELENIUM

Selenium concentrations in the study area are generally below the MCL (50 µg/L). Concentrations of selenium are higher in the Ogallala-South area with 10 percent of wells exceeding the MCL, and in the Dockum aquifer where 15 percent of wells exceed the MCL. In the Ogallala-North and Edwards-Trinity (High Plains) aquifers, less than 1 percent of wells exceed the MCL for selenium. Figure 3.11 shows the distribution of selenium concentrations within the study area.

Figure 3.11 Spatial Distribution of Selenium Concentrations in the Study Area



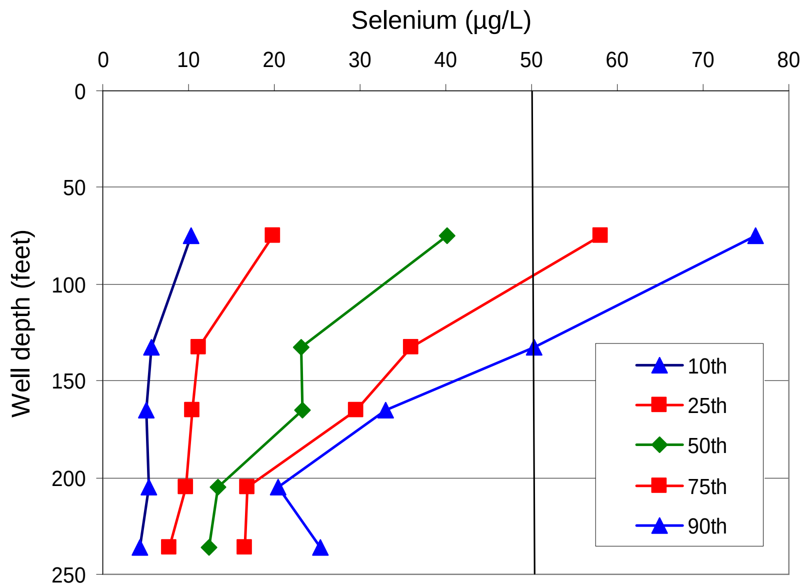
Data are from the TWDB database. The most recent sample for each well is shown. Table 3.4 shows the percentage of wells with selenium concentrations exceeding the selenium MCL (50 µg/L).

Table 3.4 Summary of Selenium Concentrations by Aquifer

Aquifer	Total number of wells	Selenium > 50 µg/L	
		Number of wells	Percentag e
Ogallala-South	225	22	10%
Ogallala-North	227	1	0.5%
Edwards-Trinity (High Plains)	11	0	0%
Dockum	33	5	15%
Other	2	0	0%

In the Ogallala-South area, where many wells have selenium concentrations >50 µg/L, there is a stratification of selenium concentrations with depth, particularly in the upper percentiles (Figure 3.12). Stratification of selenium is similar to that of nitrate and fluoride, with a decrease in selenium levels in the upper 200 feet (Figure 3.12). This suggests that tapping deeper water by deepening shallow wells or screening off the shallower parts of certain wells may decrease selenium concentrations and might provide a solution for wells where selenium exceeds the MCL.

Figure 3.12 Stratification of Selenium Concentrations with Depth in the Ogallala-South Area

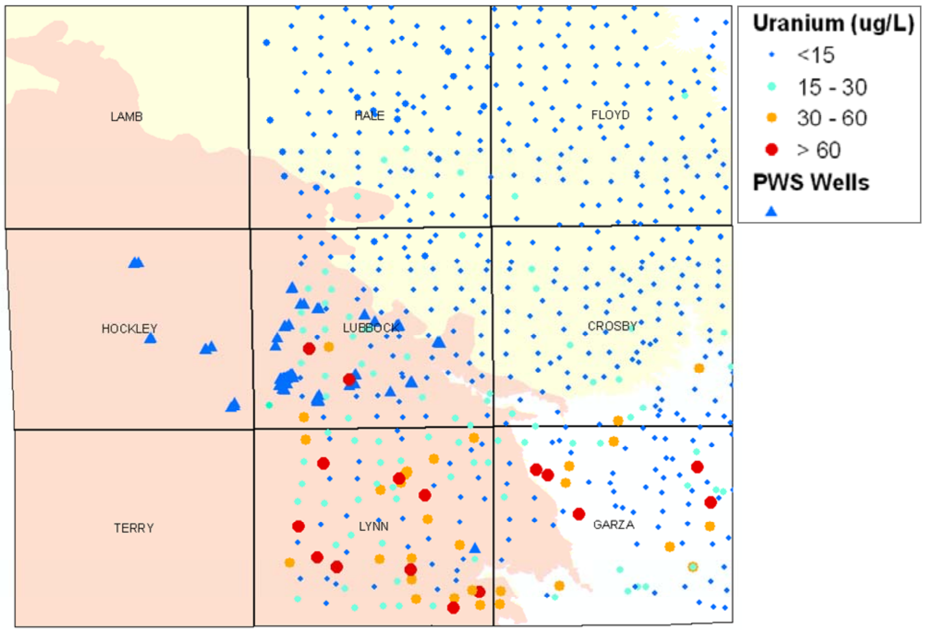


Selenium concentrations are plotted as the 10th, 25th, 50th, 75th, and 90th percentiles and depths represent the median of 20th percentiles

URANIUM

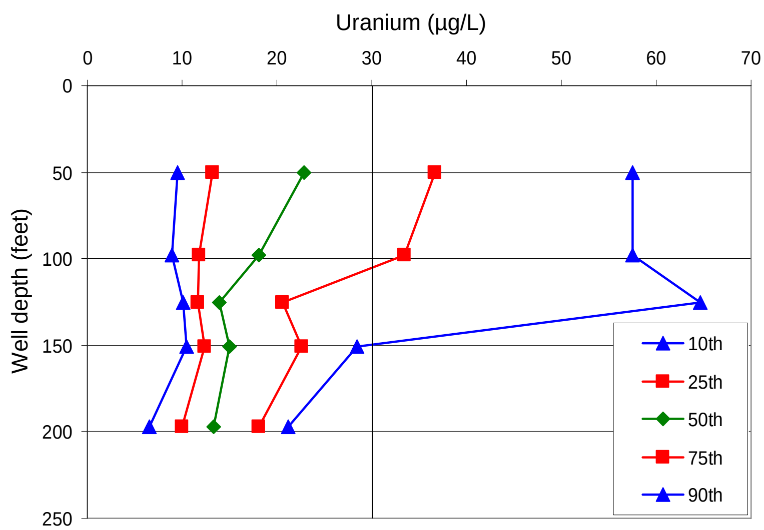
Uranium concentrations in the study area show distinct variation between the Ogallala-North and Ogallala-South areas. Concentrations of uranium are higher in the Ogallala-South area with 19 percent of wells exceeding the MCL (30 µg/L). In the Ogallala-North area there are no measurements that exceed the MCL for uranium (Figure 3.13). Data in the map are from the NURE database.

Figure 3.13 Spatial Distribution of Uranium Concentrations in the Study Area



In the Ogallala-South area where some wells show uranium concentrations greater than 30 µg/L, there is some stratification of uranium concentrations with depth, particularly in the upper percentiles (Figure 3.14). Depth stratification of uranium is similar to that of nitrate, fluoride, and selenium, with a decrease in uranium levels in the upper 150-200 feet. This suggests that tapping deeper water by deepening shallow wells or screening off the shallower parts of certain wells may decrease uranium concentrations and might provide a solution for wells where uranium exceeds the MCL.

Figure 3.14 Stratification of Uranium Concentrations with Depth in the Ogallala-South Area



Uranium concentrations are plotted as the 10th, 25th, 50th, 75th, and 90th percentiles and depths represent the median of 20th percentiles

3.3 REGIONAL GEOLOGY

The major aquifer in the study area is the High Plains or Ogallala aquifer. The main geologic unit that makes up the High Plains aquifer is the Ogallala Formation, which is late Tertiary (Miocene-Pliocene, about 4-12 million years) (Nativ 1988). The Ogallala formation consists of coarse fluvial sandstones and conglomerates that were deposited in paleovalleys in a mid-Tertiary erosional surface with eolian sand in intervening upland areas (Gustavson and Holliday 1985). The Ogallala-North area generally corresponds to a paleovalley where the saturated thickness of the aquifer is greater and the water table is deeper. In contrast, the Ogallala-South area generally corresponds to a paleoupland where the Ogallala Formation is thin, the aquifer thickness is low, and the water table is shallower. The top of the Ogallala Formation is marked by a resistant calcite layer termed the “caprock” caliche.

The Ogallala Formation is overlain by Quarternary-age (Pleistocene-Holocene) eolian, fluvial, and lacustrine sediments called the Blackwater Draw Formation (Holliday 1989). The texture of the formation ranges from sand and gravel along riverbeds and mostly clay in playa floors.

The Ogallala Formation is underlain by lower Cretaceous (Comanchean) strata in the southern High Plains. The top of the Cretaceous sediments is marked by an erosional surface that represents the end of the Laramide orogeny. Nonuniform erosion resulted in topographic relief on the Cretaceous beneath the Ogallala Formation. Cretaceous strata are absent beneath the thick Ogallala paleovalley fill deposits because they were removed by erosion. The Cretaceous sediments were deposited in a subsiding shelf environment and consist of (1) the Trinity Group (basal sandy, permeable Antlers Formation), (2) Fredericksburg Group (limy to shaly formations, including the Walnut, Comanche Peak, and Edwards Formation, as well as the Kiamichi Formation), and (3) the Washita Group (low-permeability, shaly sediments of Duck Creek Formation) (Nativ 1988). The sequence results in two main aquifer units: the Antlers Sandstone (also termed the Trinity or Paluxy sandstone, ~15 m thick) and the Edwards Limestone (~30 m thick). The term Edwards Trinity (High Plains) aquifer is generally used to describe these units (Ashworth 1991). The limestone decreases in thickness to the northwest and transitions into the Kiamichi Formation and Duck Creek Formation (predominantly shale).

The Ogallala Formation is underlain by the Triassic Dockum Group in much of the southern High Plains. The Dockum Group is exposed along the margins of the High Plains (~150 m thick). The uppermost sediments consist of red mudstones (termed red beds) that generally form an aquitard. Underlying units (Trujillo Sandstone [Upper Dockum] and Santa Rosa Sandstone [Lower Dockum]) are aquifers. Water quality in the Dockum is generally poor (Dutton and Simpkins 1986). The sediment of the Dockum was deposited in a continental fluvio-lacustrine environment that included streams, deltas, lakes, and mud flats (McGowen, *et al.* 1977) and included alternating arid and humid climatic conditions. The Triassic rocks are thickest in the Midland Basin (≤600 m).

3.4 DETAILED ASSESSMENT

The Roosevelt ISD PWS has four wells: G1520123A, G1520123B, G1520123C, and G1520123D. Wells G1520123A, G1520123B, and G1520123D are drilled to 150 feet and are designated as being within the Ogallala aquifer (121OGLL). Well G1520123C is 192 feet deep and is designated as being within an unknown aquifer. Wells G1520123B and G1520123C share an entry point in the water supply, and wells G1520123A and G1520123D are connected to separate entry points. This makes it possible to narrow down the source of contaminants in the system. Table 3.5 summarizes nitrate and arsenic concentrations measured at the Roosevelt ISD PWS.

Table 3.5 Nitrate and Arsenic Concentrations in the Roosevelt ISD PWS

Date	Nitrate-N (mg/L)	Arsenic (µg/L)	Well or wells sampled
1/21/1997	11.6	6.0	G1520123A, B, C and D
6/12/1997	9.92	-	G1520123A, B, C and D
12/10/1997	19.52	-	G1520123D
1/12/1998	17.48	-	G1520123D
5/4/1998	18.37	-	G1520123D
7/20/1998	18.58	-	G1520123D
1/7/1999	12.32	-	G1520123D
1/6/2000	18.9	-	G1520123D
1/6/2000	0.01	8	G1520123A, B, C and D
2/7/2001	20.08	-	G1520123D
5/15/2001	-	-	G1520123D
6/7/2001	18.83	-	G1520123D
8/20/2001	14.53	-	G1520123D
11/19/2001	15.42	-	G1520123D
2/11/2002	17.55	-	G1520123D
5/21/2002	19.39	-	G1520123D
8/7/2002	19.46	-	G1520123D
11/21/2002	12.16	-	G1520123D
3/13/2003	20.6	-	G1520123D
3/13/2003	13.89	16.4	G1520123A, B, C and D
5/28/2003	17	-	G1520123D
7/16/2003	-	-	G1520123D
9/4/2003	14.68	-	G1520123D
12/1/2003	12.33	9.4	G1520123D
1/28/2004	11.9	-	G1520123D
4/7/2004	16.63	-	G1520123D
9/23/2004	14.05	-	G1520123D
11/12/2004	0.25	-	G1520123D
1/26/2005	18.1	-	G1520123D
1/26/2005	-	10.1	G1520123D

Date	Nitrate-N (mg/L)	Arsenic (µg/L)	Well or wells sampled
4/28/2005	17.9	-	G1520123D
4/28/2005	-	13.3	G1520123D
7/20/2005	-	8.77	G1520123D
7/20/2005	19.3	-	G1520123D
11/1/2005	13.7	-	G1520123D
11/1/2005	-	10.4	G1520123D
1/24/2006	18.3	-	G1520123D
1/24/2006	-	9.11	G1520123D
4/20/2006	20.8	-	G1520123D
6/8/2006	-	11.7	G1520123D
7/20/2006	20	-	G1520123D
9/28/2006	-	8.04	G1520123D
10/24/2006	15.8	-	G1520123D
10/24/2006	-	8.04	G1520123D
1/18/2007	20.3	-	G1520123D
1/18/2007	-	9.72	G1520123D
4/19/2007	23.2	-	G1520123D
4/19/2007	-	6.94	G1520123D
12/10/1997	10.85	-	G1520123B and C
1/12/1998	11.28	-	G1520123B and C
5/4/1998	11.93	-	G1520123B and C
7/20/1998	12.87	-	G1520123B and C
1/7/1999	4.5	-	G1520123B and C
4/22/1999	12.32	-	G1520123B and C
8/5/1999	19.49	-	G1520123B and C
10/12/1999	19.54	-	G1520123B and C
1/6/2000	12.91	-	G1520123B and C
8/14/2000	13.25	-	G1520123B and C
10/16/2000	13.46	-	G1520123B and C
2/7/2001	-	-	G1520123B and C
5/15/2001	-	-	G1520123B and C
6/7/2001	13.21	-	G1520123B and C
9/12/2001	14.26	-	G1520123B and C
11/19/2001	13.88	-	G1520123B and C
2/11/2002	13.19	-	G1520123B and C
5/21/2002	13.67	-	G1520123B and C
8/7/2002	14.29	-	G1520123B and C
11/21/2002	13.23	-	G1520123B and C
3/13/2003	13.4	-	G1520123B and C
5/28/2003	14.04	-	G1520123B and C
7/16/2003	-	-	G1520123B and C
9/4/2003	14.93	-	G1520123B and C
12/1/2003	13.65	9.6	G1520123B and C

Date	Nitrate-N (mg/L)	Arsenic (µg/L)	Well or wells sampled
1/28/2004	12.46	-	G1520123B and C
4/7/2004	11.56	-	G1520123B and C
9/23/2004	14.67	-	G1520123B and C
11/12/2004	0.9	-	G1520123B and C
1/26/2005	11.9	-	G1520123B and C
1/26/2005	-	10.3	G1520123B and C
4/28/2005	13.4	-	G1520123B and C
4/28/2005	-	11.7	G1520123B and C
7/20/2005	-	8.6	G1520123B and C
7/20/2005	13.8	-	G1520123B and C
11/1/2005	13.5	-	G1520123B and C
11/1/2005	-	10.4	G1520123B and C
1/24/2006	11.7	-	G1520123B and C
1/24/2006	-	9.78	G1520123B and C
4/20/2006	14.1	-	G1520123B and C
6/8/2006	-	9.27	G1520123B and C
7/20/2006	14.2	-	G1520123B and C
10/24/2006	15.4	-	G1520123B and C
1/18/2007	15.6	-	G1520123B and C
1/18/2007	-	10.3	G1520123B and C
4/19/2007	13.8	-	G1520123B and C
4/19/2007	-	7.34	G1520123B and C
12/10/1997	3.75	-	G1520123A
1/12/1998	7.42	-	G1520123A
1/7/1999	10.69	-	G1520123A
1/6/2000	6.05	-	G1520123A
2/7/2001	-	-	G1520123A
5/15/2001	-	-	G1520123A
6/7/2001	10.42	-	G1520123A
8/20/2001	9.56	-	G1520123A
11/19/2001	8.66	-	G1520123A
2/11/2002	5.64	-	G1520123A
5/21/2002	7.22	-	G1520123A
8/7/2002	11.64	-	G1520123A
11/21/2002	6.82	-	G1520123A
3/13/2003	6.3	-	G1520123A
5/28/2003	9.3	-	G1520123A
7/16/2003	-	-	G1520123A
9/4/2003	9.14	-	G1520123A
12/1/2003	11.38	10	G1520123A
1/28/2004	6.91	-	G1520123A
4/7/2004	6.36	-	G1520123A
9/23/2004	9.88	-	G1520123A

Date	Nitrate-N (mg/L)	Arsenic (µg/L)	Well or wells sampled
11/12/2004	0.06	-	G1520123A
1/26/2005	-	13.5	G1520123A
1/26/2005	9.38	-	G1520123A
4/28/2005	12.2	-	G1520123A
4/28/2005	-	12.6	G1520123A
7/20/2005	-	8.34	G1520123A
7/20/2005	14.4	-	G1520123A
11/1/2005	12.1	-	G1520123A
11/1/2005	-	11.8	G1520123A
1/24/2006	-	10.9	G1520123A
1/24/2006	11.7	-	G1520123A
4/20/2006	12.7	-	G1520123A
4/20/2006	-	10.6	G1520123A
7/20/2006	14.8	-	G1520123A
7/20/2006	-	7.99	G1520123A
10/24/2006	15.7	-	G1520123A
10/24/2006	-	9.6	G1520123A
1/18/2007	12.9	-	G1520123A
1/18/2007	-	10.7	G1520123A
4/19/2007	12.4	-	G1520123A
4/19/2007	-	8.15	G1520123A

(data from the TCEQ database)

Of 101 known nitrate measurements, taken between 1997 and 2007, 80 exceeded the MCL for nitrate (10 mg/L). Of the 21 acceptable nitrate concentrations, 16 were sampled at the entry point associated with well G1520123A between 1997 and 2005. In general nitrate concentrations were somewhat lower at this entry point, although many of the measurements at the entry point were still above or close to the nitrate MCL (10 mg/L), of the 34 arsenic measurements made at the PWS between 1997 and 2007, 15 exceeded the arsenic MCL (10 µg/L). The spatial distribution of nitrate, arsenic, and fluoride concentrations measured within 5- and 10-km buffers of the PWS are shown in Figures 3.15, 3.16, and 3.17.

Figure 3.15 Nitrate Concentrations Within 5- and 10-Km Buffers of the Roosevelt ISD PWS Wells

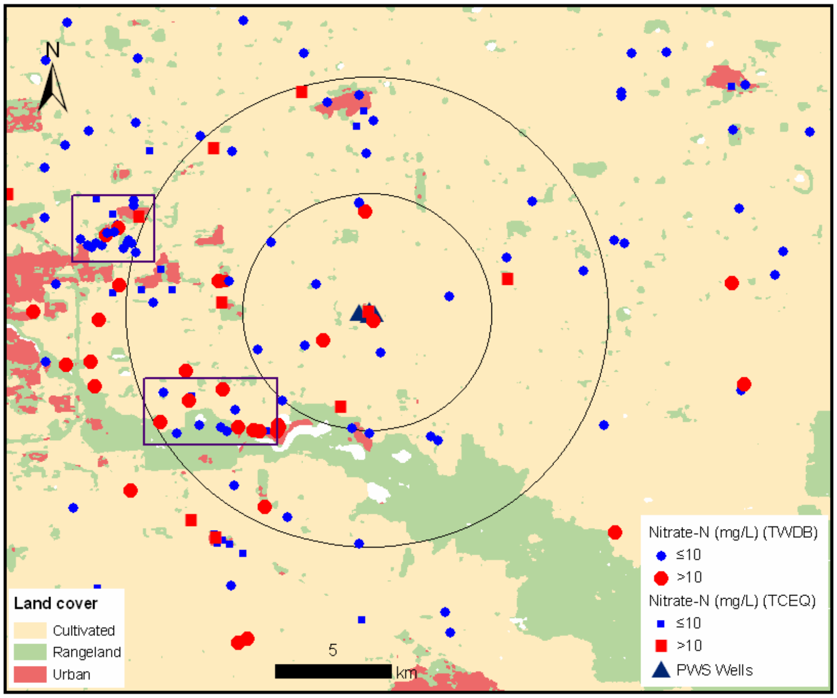


Figure 3.16 Arsenic Concentrations Within 5- and 10-Km Buffers of the Roosevelt ISD PWS Wells

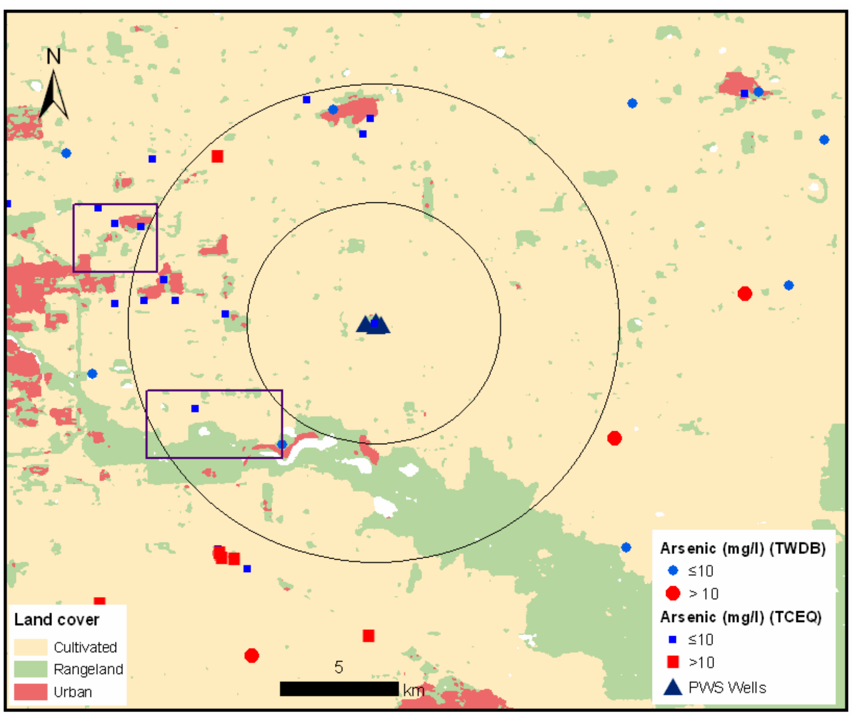
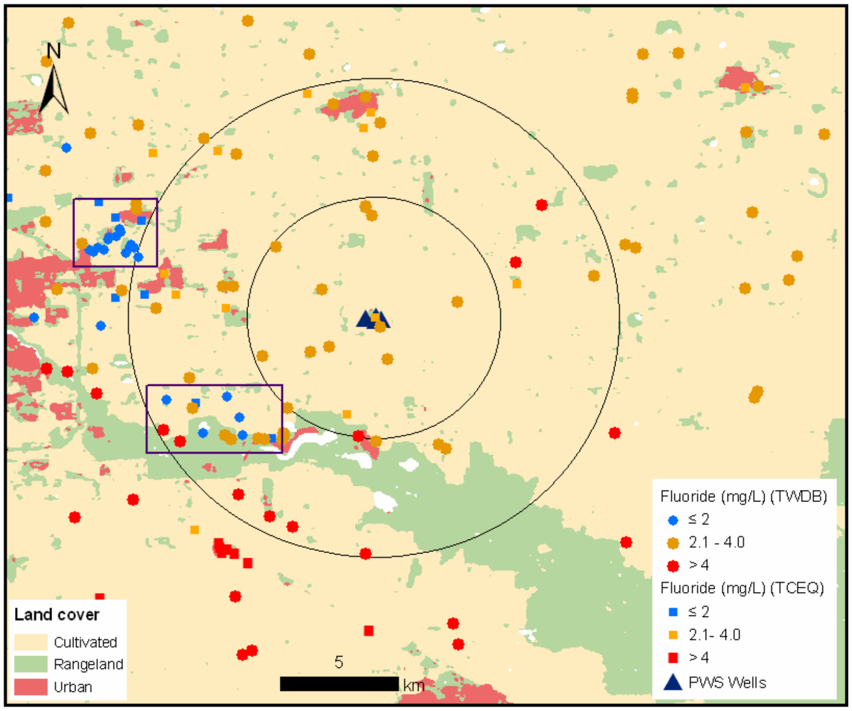


Figure 3.17 Fluoride Concentrations Within 5- and 10-Km Buffers of the Roosevelt ISD PWS Wells



Data are from the TCEQ and TWDB databases. Two types of samples were included in the analysis. Samples from the TCEQ database (shown as squares on the map) represent the most recent sample taken at a PWS, which can be raw samples from a single well or entry point samples that may combine water from multiple sources. Samples from the TWDB database are taken from single wells (shown as circles in the map). Where more than one measurement has been made in a well, the most recent concentration is shown.

Many of the samples taken within 10 km of the PWS wells have nitrate concentrations below the 10 mg/L MCL (Figure 3.15). However, most of these wells contain fluoride concentrations above the secondary MCL of 2 mg/L (Figure 3.17). Two groups of wells to the west and southwest of the Roosevelt ISD PWS show both nitrate below the MCL (10 mg/L) and fluoride concentrations below the secondary MCL (2 mg/L). The closest group (about 6 km southwest of the PWS) includes a number of wells and two PWS that show nitrate, arsenic, and fluoride below the MCLs. Table 3.6 gives information on the aquifer, well depth, water use, and the most recent concentrations measured at the wells.

Table 3.6 Characteristics of Wells Near the Roosevelt ISD PWS Wells that have Acceptable Levels of Nitrate

State or PWS well number	Aquifer	Well depth (ft)	Primary use	Nitrate -N (mg/L)	Fluoride (mg/L)	Arsenic (µg/L)	Selenium (µg/L)	Uranium (µg/L)
2326618	121OGLL	115	recreation	3.45	1.7	-	-	-
2327716	121OGLL	-	unused	3.06	1.6	-	-	-
2327402	121OGLL	115	irrigation	9.94	2.0	-	-	-
PWS 1520006	121OGLL	130-170	water supply	0.68	2.0	-	-	-
PWS 1520135	121OGLL	125	water supply	1.48	0.23	2.0	-	-

fluoride (data from the TCEQ and TWDB databases)

3.4.1 Summary of Alternative Groundwater Sources

One option is to obtain additional groundwater supplies from nearby wells. Data from the TWDB and TCEQ databases show many wells in the vicinity of the Roosevelt ISD PWS wells that have been shown to have nitrate levels below the MCL (10 mg/L). Most of these wells have fluoride concentrations above the secondary MCL and are not recommended as alternative sources. Two groups of wells are identified and the closer one is located about 6 km southwest of the Roosevelt ISD PWS. Nitrate and fluoride concentrations in the group of wells are below the nitrate MCL and below the secondary fluoride MCL (2 mg/L). The second group of wells is located about 10 km west of the PWS and includes a number of wells with nitrate and fluoride concentrations below the MCLs. Current levels of constituents should be measured before deciding whether to implement this option.

Regional analyses show that nitrate, fluoride, and arsenic tend to decrease with depth. Therefore, deepening one or more of the PWS wells and screening only the deeper portion of the wells might lower nitrate concentrations. However, there are not enough local data available to evaluate this option.

SECTION 4
ANALYSIS OF THE ROOSEVELT ISD PWS

4.1 DESCRIPTION OF EXISTING SYSTEM

4.1.1. Existing System

The Roosevelt ISD is shown in Figure 4.1. The Roosevelt ISD Water System is located north of Ransom Canyon, Texas at 1406 CR 3300, and 7 miles east of Lubbock. Berhl Robertson is the ISD Superintendent; Roy Turner is the Maintenance Director, and Dale Lampe is the system operator. Roosevelt ISD serves 1,600 people with 11 service connections.

Water for Roosevelt ISD is supplied by three 150 feet deep water wells: 1520123A, 1520123B, and 1520123D. Well 1520123A serves the high school through three pressure tanks. Well 1520123B serves the administration, elementary and junior high school buildings through tree pressure tanks, and well 1520123D serves through one pressure tank the teacher residences. Hypochlorination is provided after the wells and before storage. There are three separate pressure planes/systems within this facility. A fourth well is used to water the athletic fields and is not connected to the other systems.

The distribution system was constructed of PVC pipe, and is in good condition. There is a pipeline 4 miles away at Ransom Canyon that carries water from the City of Lubbock. Roosevelt ISD has sought water from the Town of Ransom Canyon for potable use. To achieve this, Roosevelt ISD proposes to construct a 6-inch supply line from Ransom Canyon to the Roosevelt ISD campus. This water will be discharge into a ground storage tank. This distribution system will consist of approximately 4,500 feet of 6-inch pipeline. An elevated storage tank with a capacity of 50,000 gallons will assist with maintaining system pressure and meeting fire flow requirements.

Between January 1997 and April 2005, arsenic concentrations ranged from 0.006 mg/L to 0.0164 mg/L at the Roosevelt ISD PWS. Nitrate concentrations ranged from 0.01 mg/L to 20.6 mg/L. The majority of measurements were above the 0.010 mg/L MCL for arsenic and 10 mg/L for nitrate. Therefore, the Roosevelt ISD faces compliance issues under the water quality standards for arsenic and nitrate.

Basic system information is as follows:

- Population served: 1,600
- Connections: 11
- Average daily flow: 0.048 million gallons per ay (mgd)
- Total production capacity: 0.192 mgd (estimated)

Basic system raw water quality data are as follows:

- Typical arsenic range: 0.006-0.0164 mg/L
- Typical nitrate range: <0.01-20.6 mg/L
- Typical TDS range: 885-984 mg/L
- Typical pH range: 7.3-7.7
- Typical calcium range: 83.5-150 mg/L
- Typical magnesium range: 63-97.8 mg/L
- Typical manganese: <0.008 mg/L
- Typical sodium range: 98-181 mg/L
- Typical chloride range: 204-219
- Typical bicarbonate (HCO_3) range: 365-404 mg/L
- Typical fluoride range: 2.3-3.3 mg/L
- Typical iron range: 0.01-0.036 mg/L

4.1.2 Capacity Assessment

The project team conducted a capacity assessment of the Roosevelt Independent School District water system on April 20, 2007. The results of this evaluation are separated into four categories: general assessment of capacity, positive aspects of capacity, capacity deficiencies, and capacity concerns. The general assessment of capacity describes the overall impression of FMT capability of the water system. The positive aspects of capacity describe the strengths of the system. These factors can provide the building blocks for the system to improve capacity deficiencies. The capacity deficiencies noted are those aspects that are creating a particular problem for the system related to long-term sustainability. Primarily, these problems are related to the system's ability to meet current or future compliance, ensure proper revenue to pay the expenses of running the system, and to ensure the proper operation of the system. The last category, capacity concerns, includes items that are not causing significant problems for the system at this time. However, the system may want to address them before they become problematic.

Because of the challenges facing very small water systems, it is increasingly important for them to develop the internal capacity to comply with all state and federal requirements for public drinking water systems. For example, it is especially important for very small water systems to develop long-term plans, set aside money in reserve accounts, and track system expenses and revenues because they cannot rely on increased growth and economies of scale to offset their costs. In addition, it is crucial for the owner, manager, and operator of a very small water system to understand the regulations and participate in appropriate trainings. Providing safe drinking water is the responsibility of every public water system, including those very small water systems that face increased challenges with compliance.

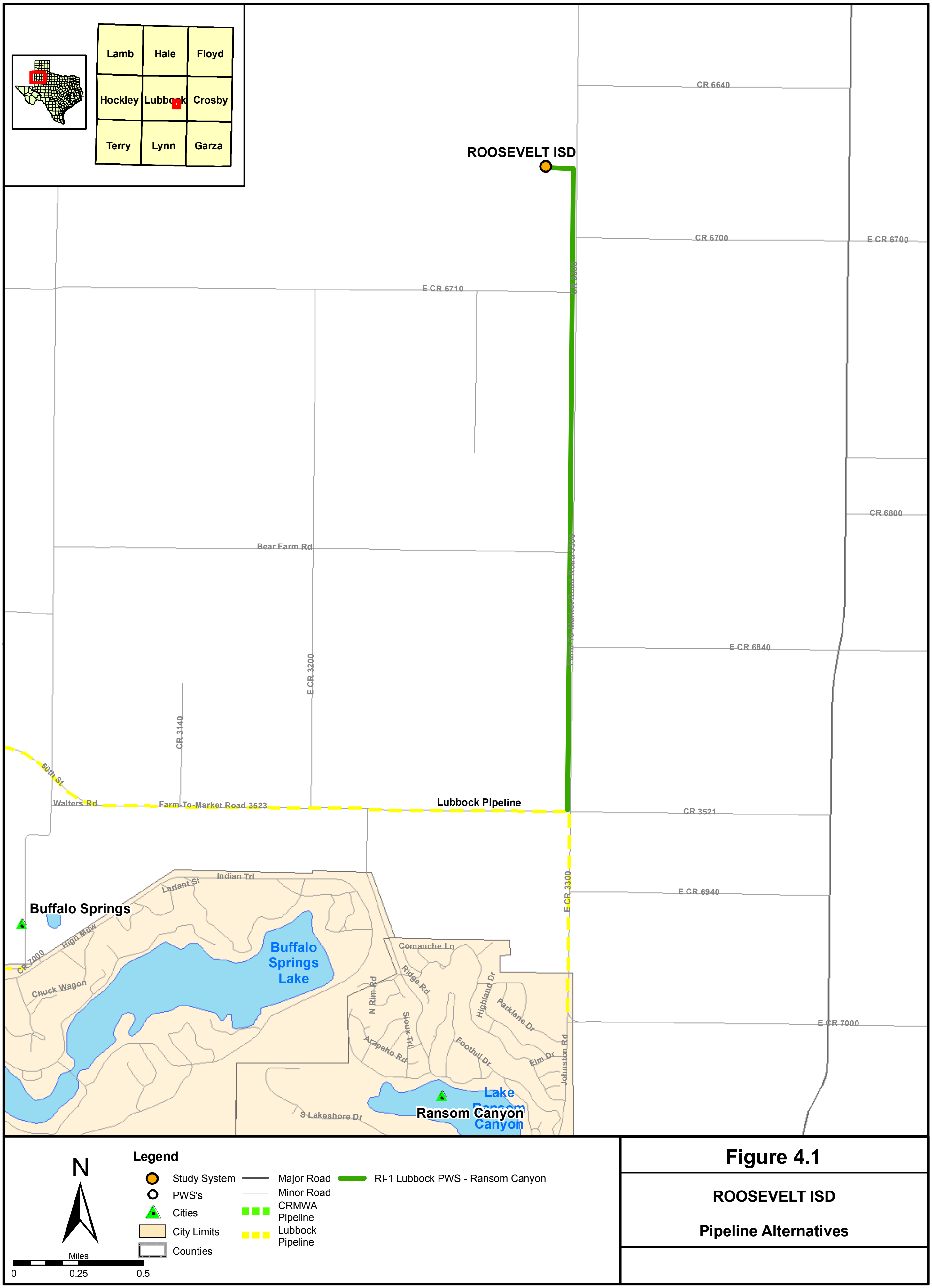


Figure 4.1

ROOSEVELT ISD

Pipeline Alternatives

The project team interviewed the following individuals:

- Berhl Robertson, Jr., Superintendent
- Dale Lampe, Water Operator

4.1.2.1 General Structure

The Roosevelt Independent School District water system serves about 1,600 students and staff with 11 service connections at an Administration Building, Elementary School, and High School. The District provides some housing for teachers, but these houses are on individual wells. The school is located about 7 miles east of Lubbock. The district is governed by a 7 member school board. The district receives approximately $6.5 million from state funding. In addition, property taxes generate about $2 million in revenue and the district receives between $600,000 and $1 million a year from federal programs. The annual budget is $9.5 million. All expenses for the water system are included in the maintenance budget and paid for out of the overall District revenues. The district is required to have a financial audit each year.

The staff for the water system consists of an operator and maintenance supervisor. Public notices are posted on the campus for teachers who are pregnant, however students are permitted to drink the water.

4.1.2.2 General Assessment of Capacity

Based on the team's assessment, this system has an adequate level of capacity. There are several positive FMT aspects of the water system. The only deficiency noted was the lack of compliance with the drinking water standards for nitrate.

4.1.2.3 Positive Aspects of Capacity

In assessing a system's overall capacity, it is important to look at all aspects – positive and negative. It is important for systems to understand those characteristics that are working well, so that those activities can be continued or strengthened. In addition, these positive aspects can assist the system in addressing the capacity deficiencies or concerns. The factors that were particularly important for Roosevelt ISD are listed below.

- **Knowledgeable and Dedicated Staff** – The water operator has been with the school for 30 years. While he is not certified, he is very knowledgeable about the system. The Maintenance Supervisor is certified and has been with the district for 3 years. The District Superintendent is committed to providing safe drinking water for the students and staff and has been working on options to address compliance issues.
- **Efforts toward Compliance** – The District has held discussions with the City of Lubbock to provide water from the City of Ransom Canyon, which has a line about 4 miles south of the school. The district believes that the nitrate problem is caused by a nitrate plume from the discharge of wastewater in the area. The total cost of the project is about $1 million. In order to have fire protection, the District will need to

obtain water from the City of Lubbock. The District would like for the City of Lubbock to finance the project, with repayment from the District. The Superintendent is hopeful that the City Council would approve the project in October of 2007 and construction could begin in 2008.

4.1.2.4 Capacity Deficiencies

The following capacity deficiencies were noted in conducting the assessment and seriously impact the ability of the water system to meet compliance with current and future regulations and to ensure long-term sustainability.

- **Lack of Compliance with Water Quality Standards –** The water system is not in compliance with water quality standards.
- **Lack of Long Term Capital Planning for Compliance and Sustainability** – There appears to be no long term plan in place to achieve and maintain compliance and to ensure the long-term sustainability of the water system. System needs appear to be assessed on a daily basis, rather than a multi-year basis. Without some type of planning process, the school district is not able to plan for the funding needed to make system improvements or add treatment processes. The district can also use the long-term planning process to help identify financing strategies to pay for the long-term needs.

4.1.2.5 Potential Capacity Concerns

The following items were concerns regarding capacity but no specific operational, managerial, or financial problems can be attributed to these items at this time. The system should address the items listed below to further improve FMT capabilities and to improve the system's long-term sustainability.

- **Lack of separate accounting for water system** - Without a separate budget for the water system, it is not possible to know how much of the maintenance budget is allocated for water operations. It is also difficult to estimate the financial impact of implementing a compliance alternative. There should be sufficient funds for the operation and maintenance, as well as a reserve fund for major equipment replacement. Insufficient funding may pose risks if it results in an inability to maintain and upgrade the facility or maintain sufficient stocks of spare parts, chemicals or equipment.

4.2 ALTERNATIVE WATER SOURCE DEVELOPMENT

4.2.1 Identification of Alternative Existing Public Water Supply Sources

Using data drawn from the TCEQ drinking water and TWDB groundwater well databases, the PWSs surrounding the Roosevelt ISD PWS were reviewed with regard to their reported drinking water quality and production capacity. PWSs that appeared to have water supplies with water quality issues or that purchase water were ruled out from evaluation as alternative sources, while those without identified water quality issues were investigated further. Owing to the large number of small (<1 mgd) water systems in the vicinity, small systems were only considered if they were established residential or non residential systems within 5 miles of Roosevelt ISD. Large systems or systems capable of producing greater than four times the daily volume produced by the study system were considered if they were within 15 miles of the study system. A distance of 15 miles was considered to be the upper limit of economic feasibility for constructing a new water line. Table 4.1 is a list of the selected PWSs based on these criteria for large and small PWSs within 15 miles of Roosevelt ISD. If it was determined that these PWSs had excess supply capacity and might be willing to sell the excess, or might be a suitable location for a new groundwater well, the system was taken forward for further consideration and identified with "EVALUATE FURTHER" in the comments column of Table 4.1.

Table 4.1 Selected Public Water Systems within 15 Miles of the Roosevelt ISD

PWS ID	PWS Name	Distance from Roosevelt ISD (miles)	Comments/Other Issues
1520204	PINKIES MINI MART 51	3.18	Small NonRes GW system. WQ issues: Nitrate
1520210	APPLES PIZZA DELI	3.31	Small NonRes GW system. WQ issues: Nitrate
1520056	RANSOM CANYON TOWN OF	3.51	Large GW system. Purchase water. EVALUATE FURTHER
1520006	LUBBOCK COUNTY WCID 1	3.88	Large GW system. Purchase water.
1520135	PINKIES MINI MART 53	4.52	Small NonRes GW system. WQ issues: Nitrates
1520128	COUNTY LINE BAR B Q	4.64	Small NonRes GW system. WQ issues: Nitrate
1520046	WILDWOOD MOBILE HOME VILLAGE	4.73	Large GW system. WQ issues: As, Nitrate
1520158	MILLER MOBILE HOME PARK	4.97	Large GW system. WQ issues: As
1520232	FULLER MOBILE HOME PARK	5.16	Large GW system. Marginal WQ issues: As
1520001	IDALOU CITY OF	5.61	Large GW system. WQ issues: As
1520080	KELSO WATER SYSTEM INC. 3	5.69	Large GW system. WQ issues: Nitrates
1520219	CHRISTIAN LIFE CENTER	6.14	Large NonRes GW system. WQ issues: Sulfate
1520148	LONE STAR MHP	6.23	Large GW system. Purchase water
1520072	TEXAS BOYS RANCH INC	6.49	Large NonRes GW system. Marginal WQ issues: Se
0540002	LORENZO CITY OF	8.01	Large GW system. Marginal WQ issue: As, Fl>2
1520149	WHORTON MOBILE HOME PARK	9.93	Large GW system. WQ issues: As, Fl
1520004	SLATON CITY OF	10.29	Large GW system. No WQ issues except Sulfate. EVALUATE FURTHER
1520079	TEXAS TECH NEW DEAL RESEARCH FRM	10.76	Large NonRes GW system. Marginal WQ issue: As, Fl>2
1520147	BECKER PUMP & PIPE WATER SUPPLY	11.06	Large NonRes GW system. WQ issues: As, Fl
1520009	BIG Q MOBILE HOME ESTATES	11.46	Large GW system. WQ issues: As, Fl, Combined Uranium
1520142	COUNTRY SQUIRE MHP 1	12.39	Large GW system. WQ issues: As, Fl, Nitrate
1520236	PRATERS FOODS INC	12.63	Large NonRes GW system. WQ issues: As, Fl
1520155	COUNTRY SQUIRE MHP 2	12.94	Large GW system. WQ issues: As, Fl
1520015	NEW DEAL CITY OF	12.97	Large GW system. WQ issues: Nitrate Purchase water
1520242	LUBBOCK STOCKYARD	13.08	Large NonRes GW system. WQ issues: Fl
1520122	LUBBOCK COOPER ISD	13.47	Large NonRes GW system. WQ issues: As, Fl
1520247	COUNTRY VIEW MHP	13.53	Large GW system. WQ issues: As, Fl, Combined Uranium
1520159	NORTH UNIVERSITY ESTATES	14	Large GW system. WQ issues: Nitrate

After the PWSs in Table 4.1 with water quality problems were eliminated from further consideration, the remaining PWSs were screened by proximity to Roosevelt ISD and sufficient total production capacity for selling or sharing water. Based on the initial screening summarized in Table 4.1 above, one alternative was selected for further evaluation. This alternative is summarized in Table 4.2 and entails obtaining water from the City of Lubbock via a pipeline running between Ransom Canyon/Buffalo Springs and the southeast side of Lubbock. The Canadian River Municipal Water Authority (CRMWA) is the primary water source to the City of Lubbock; and so a description of the CRMWA is included along with a description of the City of Lubbock PWS following Table 4.2. A second option of receiving water from the City of Slaton was reviewed; however, it was not further considered since the City of Lubbock is much closer and both cities utilize the same water source as member cities of the CRMWA.

Table 4.2 Public Water Systems Within the Vicinity of the Roosevelt ISD PWS Selected for Further Evaluation

PWS ID	PWS Name	Pop	Conn	Total Production (mgd)	Ave Daily Usage (mgd)	Approx. Dist. from Roosevelt ISD	Comments/Other Issues
1520002	Lubbock PWS via Ransom Canyon	222,473	81,059	136.077	40.263	3.5 miles	Large SW/GW system that does have excess capacity. Connection would be made at the pipeline running between the City of Lubbock and Ransom Canyon/Buffalo Springs. The primary source of water for the City of Lubbock in the eastern portion of their distribution system is CRMWA.
1520004	Slaton City of	6,800	2,297	1.334	0.843	10.3 miles	Large SW/GW system supplied with water from CRMWA. Since primary source of water for both Slaton and the City of Lubbock is the CRMWA, will consider the closer connection, the City of Lubbock via pipeline between Lubbock and Ransom Canyon.

4.2.1.1 City of Lubbock Water System

The City of Lubbock PWS produces an average of 38 to 40 mgd for the City of Lubbock and five surrounding small municipalities. The system is capable of meeting a peak demand of over 90 mgd. In addition to treating water for the City of Lubbock distribution system, the Lubbock water treatment plant treats about 6 mgd on average for the six CRMWA member cities receiving treated water from the City of Lubbock.

The City of Lubbock receives water from two sources, the CRMWA and from the Bailey County well field. Additional details on the CRMWA are provided in a separate description. As a member of the 11-City agreement with the CRMWA, the City of Lubbock is responsible for treating raw water from the Lake Meredith/Roberts County well field located 160 miles north of Lubbock. A CRMWA aqueduct distributes the treated water to six other PWSs: Levelland, Brownfield, Slaton, Tahoka, O'Donnell, and Lamesa. In 2006, the water from CRMWA constituted about 76 percent of the water used by the City of Lubbock. The other 24 percent comes from a well field in Bailey County located 60 miles northwest of Lubbock. The city has water rights to 82,000 surface acres at the Bailey County well field. The water produced by the Bailey County well field is chlorinated before it enters the pipeline leading to Lubbock. As the water reaches Lubbock, it enters directly into the distribution system predominantly in the northwest section of Lubbock. It should be noted that the City of Lubbock normally utilizes their total annual water allocation from CRMWA and if Lubbock needs additional water, their supply is supplemented with water from the Bailey County well field which consists of 150 wells capable of producing 50 mgd total (pipeline is limited to 40 mgd). In 2006, the City of Lubbock pumped an average of 9.3 mgd from the Bailey County well field. However, most of this water was pumped during the summer months with the pipeline near peak capacity at various times.

In addition to the population of Lubbock, five cities are connected to the City of Lubbock distribution system. Shallowater and Reese Redevelopment are located northwest and west of Lubbock and receive water predominantly originating in Bailey County. Buffalo Springs and Ransom Canyon are located east of Lubbock and receive water mostly originating from Lake Meredith/Roberts County well field. A fifth city, Littlefield, located northwest of the City has an emergency water line connected to the Bailey County pipeline. The decision to add these five cities to the City of Lubbock water supply was made by the Lubbock City Council.

Future plans for the City of Lubbock water supply system call for the construction of infrastructure to obtain water from Lake Alan Henry located 65 miles southeast of Lubbock. The project is still in the preliminary engineering phase. The amount of water available from this system will be staged into the existing Lubbock system over several years to match Lubbock's needs. The system is estimated to be operating in 2012.

4.2.1.2 Canadian River Municipal Water Authority

The CRMWA has contracts to provide water to 11 member cities in west Texas including Amarillo, Borger, Brownfield, Lamesa, Levelland, Lubbock, O'Donnell, Pampa, Plainview, Slaton, and Tahoka. A pipeline ranging in size from 8 feet to 1.5 feet is used to convey untreated water approximately 160 miles from Lake Meredith and a well field in Roberts County (40 miles northeast of Lake Meredith) to the Lubbock water treatment plant. Along the pipeline route, four cities (Amarillo, Borger, Pampa and Plainview) receive their allocated water supply and each of these four cities treats their own water. The rest of the raw water for the other seven member cities of the CRMWA is treated at the City of Lubbock water treatment plant. The treated water is pumped into the City of Lubbock distribution system and to the other six member cities. The raw water line flows by gravity from Amarillo to the Lubbock treatment plant. The treated water leaving the City of Lubbock water treatment plant flows by gravity in the east leg pipeline to Lamesa, however the water in the west leg to Levelland and Brownfield is pumped.

The current volume of water delivered annually by the CRMWA to the member cities is 85,000 acre-feet (35,000 acre-feet from Lake Meredith and 50,000 acre-feet from the well field in Roberts County). The available water volume is set by the CRMWA and may fluctuate during the year, but the volume is based on the water levels in the well field and in the lake. The allocation for each member city is based on a contracted percentage of the available volume. The City of Lubbock is under contract to receive 41.6 mgd from the

CRMWA, and the City of Lubbock water treatment plant treats an additional 5.4 mgd for the other six member cities. When the CRMWA program was established in the 1960s, the system was designed to accommodate the 11 member cities at the time and there were no plans to add additional member cities.

If a member city has excess water, that particular city can decide to sell that water to a non-member PWS. If the non-member city would receive the water directly from a member city's distribution system, then the CRMWA would not be involved. However, if a non-member is requesting to receive the water (essentially a portion of a member city's allocation) via a direct line from the CRMWA line, then the non-member city must get approval from the CRMWA and the 11 member cities. The non-member PWS would be responsible for financing the installation of the pipeline to connect to the CRMWA treated water line from Lubbock. The CRMWA would be involved throughout the process of a non-member PWS applying for, securing access to, and eventually receiving water through the CRMWA system.

4.2.2 Potential for New Groundwater Sources

4.2.2.1 Installing New Compliant Wells

Developing new wells or well fields is recommended, provided good quality groundwater available in sufficient quantity can be identified. Since a number of water systems in the area have water quality problems, it should be possible to share in the cost and effort of identifying compliant groundwater and constructing well fields.

Installation of a new well in the vicinity of the system intake point is likely to be an attractive option provided compliant groundwater can be found, since the PWS is already familiar with operation of a water well. As a result, existing nearby wells with good water quality should be investigated. Re-sampling and test pumping would be required to verify and determine the quality and quantity of water at those wells.

The use of existing wells should probably be limited to use as indicators of groundwater quality and availability. If a new groundwater source is to be developed, it is recommended that a new well or wells be installed instead of using existing wells. This would ensure well characteristics are known and meet standards for drinking water wells.

Some of the alternatives suggest new wells be drilled in areas where existing wells have acceptable water quality. In developing the cost estimates, Parsons assumed that the aquifer in these areas would produce the required amount of water with only one well. Site investigations and geological research, which are beyond the scope of this study, could indicate whether the aquifer at a particular site and depth would provide the amount of water needed or if more than one well would need to be drilled in separate areas.

4.2.2.2 Results of Groundwater Availability Modeling

Regional groundwater withdrawal in the Texas High Plains region is extensive and likely to remain near current levels over the next decades. In Lubbock County, where the PWS is located, groundwater is available from two sources, the relatively shallow Ogallala aquifer, and the underlying Edwards-Trinity (High Plains) aquifer. The Ogallala provides drinking water to most of the communities in the Texas panhandle, as well as irrigation water. The Edwards-Trinity (High Plains) is a lower yield aquifer used almost exclusively as an irrigation water source. Supply wells for the Roosevelt ISD water system and its vicinity withdraw water primarily from the southern Ogallala aquifer. Within a 10-mile radius of the system, a few active irrigation wells are completed in the Edwards-Trinity (High Plains) aquifer.

The Ogallala is the largest aquifer in the United States. The aquifer outcrop underlies much of the Texas High Plains region and eastern New Mexico, and extends eastward beyond Lubbock County. The Ogallala provides significantly more water for users than any other aquifer in the state, and is used primarily for irrigation. The aquifer saturated thickness ranges up to an approximate depth of 600 feet; supply wells have an average yield of approximately 500 gallons per minute (gpm), but higher yields, up to 2,000 gpm, are found in previously eroded drainage channels filled with coarse-grained sediments (TWDB 2007a). Water level declines in excess of 300 feet have occurred in several aquifer areas over the last 50 to 60 years; the rate of decline, however, has slowed in recent years and water levels have risen in a few areas (TWDB 2007a). The Texas Water Plan anticipates 24 percent depletion in the Ogallala supply over the next decades, from 5,000,097 acre-feet per year estimated in 2000 to 3,785,409 acre-feet per year in 2050.

A GAM developed for the Ogallala aquifer simulated historical conditions and provided long-term groundwater projections (Blandford, *et al.* 2003). Predictive simulations using the GAM model indicated that, if estimated future withdrawals are realized, aquifer water levels could decline to a point at which significant regions currently practicing irrigated agriculture could be essentially dewatered by 2050. The model predicted the most critical conditions for Cochran, Hockley, Lubbock, Yoakum, Terry, and Gaines Counties where the simulated drawdown could exceed 100 feet. For Lubbock County, the simulated drawdown by the year 2050 would be within a typical 50 to 100 feet range (Blandford, *et al.* 2003). The Ogallala aquifer GAM was not run for the PWS because anticipated use would represent a minor addition to regional withdrawal conditions, beyond the spatial resolution of the GAM model.

The Edwards-Trinity (High Plains) aquifer underlies the Ogallala in the south-central section of the Texas panhandle. Two distinct aquifer zones are utilized as irrigation water sources. One zone occurs in the basal sand and sandstone deposits of the Antlers Sands Formation (Trinity Group), and is usually under artesian pressure. The other water-bearing zone occurs primarily in joints, solution cavities, and bedding planes in limestone of the Fredericksburg Group. Wells completed in the Edwards-Trinity aquifer have typical yields from 50 to 200 gpm, and are usually also completed in the overlying Ogallala aquifer (TWDB 2007b). Extensive aquifer utilization has caused water-level declines, up to 30 feet, in some areas. A GAM model providing long-term groundwater projections for the Edwards-Trinity (High Plains) aquifer is under development (TWDB 2007c).

A limited number of active wells utilizing the Edwards-Trinity (High Plains) aquifer as an irrigation water source are located within a 10-mile radius of the Roosevelt ISD water system. Those wells are supplied by both the Antlers Sands formation of the Trinity Group, and the Edwards and Comanche Peak formations of the Fredericksburg Group.

4.2.3 Potential for New Surface Water Sources

There is a low potential for development of new surface water sources for the PWS system as indicated by limited water availability within the river basin. The Roosevelt ISD water system is located in the upper Brazos Basin where current surface water availability is expected to decrease up to 17 percent over the next 50 years according to the 2002 Texas Water Plan (from approximately from 1,423,071 acre-feet per year to 1,177,277 acre-feet per year during drought conditions).

In the vicinity of the Roosevelt ISD water system, there is no availability of surface water for new uses. The TCEQ availability map for the Brazos Basin indicates that in the site vicinity, and within the entire Lubbock County, unappropriated flows for new uses are typically available up to 50 percent of the time. This supply is inadequate as the TCEQ requires 100 percent supply availability for a PWS.

4.2.4 Options for Detailed Consideration

The initial review of alternative sources of water results in the following options for more-detailed consideration:

1. Lubbock Public Water System - Ransom Canyon. A pipeline would be constructed from a pipeline running between Ransom Canyon and Buffalo Springs (Alternative RI-1).

2. New Wells at 10, 5, and 1 mile. Installing a new well within 10, 5, or 1 mile of the Roosevelt ISD PWS would produce compliant water in place of the water produced by the existing active well. A pipeline and pump station would be constructed to transfer the water to the Roosevelt ISD PWS (Alternatives RI-2, RI-3, and RI-4).

4.3 CENTRAL TREATMENT OPTIONS

Centralized treatment of the well water is identified as a potential option. Reverse osmosis and EDR treatment could all be potentially applicable. The central RO treatment alternative is RI-5 and the central EDR treatment alternative is RI-6.

4.4 ALTERNATIVE DEVELOPMENT AND ANALYSIS

A number of potential alternatives for compliance with the MCL for arsenic and nitrate have been identified. Each of the potential alternatives is described in the following subsections. It should be noted that the cost information given is the capital cost and change in O&M costs associated with implementing the particular alternative. Appendix C contains cost estimates for the compliance alternatives. These compliance alternatives represent a range of possibilities, and a number of them are likely not feasible. However, all have been presented to provide a complete picture of the range of alternatives considered. It is anticipated that a PWS will be able to use the information contained herein to select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation.

4.4.1 Alternative RI-1: Purchase Water from the Lubbock PWS to Ransom Canyon

This alternative involves purchasing potable water from the City of Lubbock via a pipeline running between Ransom Canyon and Buffalo Springs. The City of Lubbock currently has sufficient excess capacity for this alternative to be feasible, although current City policy only allows drinking water to be provided to areas annexed by the City. It is assumed that Roosevelt ISD would obtain all its water from the City of Lubbock.

This alternative would require constructing a pipeline from the water line running between Ransom Canyon and Buffalo Springs to a new 30,000-gallon storage tank for the Roosevelt ISD system. A 30,000-gallon feed tank would be required at a point adjacent to the main distribution water line. A pump station would also be required to overcome pipe friction and the elevation differences between the feed tank and Roosevelt ISD. The pump station would include two 6.5 horsepower pumps, including one standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for Roosevelt ISD.

The required pipeline would be 6-inches in diameter and follow Shepard Road from the connection to a new storage tank at Roosevelt ISD. The length of pipe required would be approximately 2.6 miles long.

The estimated capital cost for this alternative includes constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the purchase price for the treated water minus the cost related to current operation of the Roosevelt ISD wells, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is $1.0 million, and the estimated annual O&M cost is $72,600. If the purchased water was used for blending rather than for the full water supply, the annual O&M cost for this alternative could be reduced because of reduced pumping costs and reduced water purchase costs. However, additional costs would be incurred for equipment to ensure proper blending, and additional monitoring to ensure the finished water is compliant.

The reliability of adequate amounts of compliant water under this alternative should be good. City of Lubbock provides treated surface water on a large scale, facilitating adequate O&M resources. From the perspective of Roosevelt ISD, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood. If the decision was made to perform blending then the operational complexity would increase.

The feasibility of this alternative is dependent reaching an agreement with the City of Lubbock for purchase of water.

4.4.2 Alternative RI-2: New Well at 10 Miles

This alternative consists of installing one new well within 10 miles of the Roosevelt ISD that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 300-foot well, a new pump station with a 30,000-gallon feed tank near the new well, an additional pump station and feed tank along the pipeline, a pipeline from the new well/feed tank to a new 30,000-gallon storage tank with two service pumps installed within a pump house near the existing intake point for the Roosevelt ISD system. The pump stations would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be approximately 10 miles long, and would be 6 inches in diameter and discharge to a new 30,000-gallon storage tank at the Roosevelt ISD. The pump station would include two pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes installing the well, constructing the pipeline, the pump stations, the storage tanks, service pumps, and pump house. The estimated O&M cost for this alternative includes O&M for the pipeline and pump station. The estimated capital cost for this alternative is $3.4 million, and the estimated annual O&M cost for this alternative is $56,600.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. For operation, this alternative would be similar to the existing system. Roosevelt ISD personnel have experience with O&M of wells, pipelines and pumps.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is likely that an alternate groundwater source would not be found on land owned by Roosevelt ISD, so landowner cooperation would likely be required.

4.4.3 Alternative RI-3: New Well at 5 Miles

This alternative consists of installing one new well within 5 miles of the Roosevelt ISD that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 300-foot well, a new pump station with a 30,000-gallon feed tank near the new well, and a pipeline from the new well/feed tank to a new 30,000-gallon storage tank with two service pumps installed within a pump house near the existing intake point for the Roosevelt ISD system. The pump station and feed tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline would be 6 inches in diameter, assumed to be approximately 5 miles long, and would discharge to a new storage tank at the Roosevelt ISD PWS. The pump station near the well would include two transfer pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes installing the well, and constructing the pipeline, the pump station, the storage tanks, service pumps, and pump house. The estimated O&M cost for this alternative includes O&M for the pipeline and pump station. The estimated capital cost for this alternative is $1.86 million, and the estimated annual O&M cost for this alternative is $37,400.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. For operation, this alternative would be similar to the existing system. Roosevelt ISD personnel have experience with O&M of wells, pipelines and pumps.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is likely an alternate groundwater source would not be found on land owned by Roosevelt ISD, so landowner cooperation would likely be required.

4.4.4 Alternative RI-4: New Well at 1 Mile

This alternative consists of installing one new well within 1 mile of the Roosevelt ISD that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 300-foot well, a pipeline from the new well to a new 30,000-gallon storage with two service pumps installed within a pump house near the existing intake point for the Roosevelt ISD system. A pump station and storage tank would not be necessary to overcome pipe friction and changes in land elevation for only 1 mile, the well pump would be sufficient. For this alternative, the pipeline would be 6 inches in diameter, assumed to be approximately 1 mile long, and would discharge to a new storage tank at the Roosevelt ISD PWS. The new storage tank would include two service pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes installing the well, constructing the pipeline, the storage tanks, service pumps, and pump house. The estimated O&M cost for this alternative includes O&M for the pipeline. The estimated capital cost for this alternative is $526,900, and the estimated annual O&M cost for this alternative is $18,500.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. For operation, this alternative would be similar to the existing system. Roosevelt ISD personnel have experience with O&M of wells, pipelines and pumps.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is possible an alternate groundwater source would not be found on land owned by Roosevelt ISD, so landowner cooperation may be required

4.4.5 Alternative RI-5: Central RO Treatment

This system would continue to pump water from the existing wells and would treat the water through an RO system prior to distribution. For this option, 60 percent of the raw water would be treated and blended with untreated water to obtain compliant water. The RO process concentrates impurities in the reject stream which would require disposal. It is estimated the RO reject generation would be approximately 6,450 gallons per day (gpd) when the system is operated at the average daily flow rate of 0.043 mgd.

This alternative consists of constructing the RO treatment plant near the existing wells. The plant is composed of a 700 square foot building with a paved driveway; a skid with the pre-constructed RO plant; two transfer pumps, a 30,000-gallon tank for storing the treated water; and a 400,000 gal earthen pond for storage of RO reject before disposal by hauling. The treated water would be chlorinated and stored in the new treated water tank prior to being pumped into the distribution system. The existing pressure tanks would continue to be used to feed the distribution system. The entire facility is fenced.

The estimated capital cost for this alternative is $937,900, and the estimated annual O&M cost is $81,000.

The reliability of this alternative is good, since RO treatment is a common and well-understood treatment technology. However, O&M efforts required for the central RO treatment plant may be significant, and O&M personnel would require training with RO. The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.6 Alternative RI-6: Central EDR Treatment

The system would continue to pump water from the existing wells, and would treat the water through an EDR system prior to distribution. For this option the EDR would treat the full flow without bypass as the EDR operation can be tailored for desired removal efficiency. It is estimated the EDR concentrate generation would be approximately 4,700 gpd when the system is operated at the average daily flow rate of 0.043 mgd. The EDR concentrate would be discharged to a new pond.

This alternative consists of constructing the EDR treatment plant near the existing wells. The plant is composed of a 700 square foot building with a paved driveway; a skid with the pre-constructed EDR system; two transfer pumps; a 30,000-gallon tank for storing the treated water; and a 400,000 gal earthen pond for storage of EDR reject for disposal by hauling. The treated water would be chlorinated and stored in the new treated water tank prior to being pumped into the distribution system. The existing pressure tanks would continue to be used to pressure the distribution system. The entire facility would be fenced.

The estimated capital cost for this alternative is $1.12 million and the estimated annual O&M cost is $75,500.

The reliability of this alternative is good, since EDR treatment is a common and well-understood treatment technology. However, O&M efforts required for the central EDR treatment plant may be significant, and O&M personnel would require training with EDR. The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.7 Summary of Alternatives

Table 4.3 provides a summary of the costs of each alternative for Roosevelt ISD PWS.

Table 4.3 Summary of Compliance Alternatives for Roosevelt ISD PWS

Alt No.	Alternative Description	Major Components	Capital Cost[1]	Annual O&M Cost[1]	Total Annualized Cost[2]	Reliability	System Impact	Remarks
RI-1	Purchase water from the City of Lubbock	-1 Storage tanks - 1 Pump stations - 2.6-mile pipeline	$1,000,900	$72,600	$159,800	Good	N	Agreement must be successfully negotiated with the City of Lubbock and pipeline easements must be obtained. Blending may be possible. Costs could possibly be shared with small systems along pipeline route.
RI-2	Install new compliant well at 10 miles	- New well - 1 Storage tanks - 2 Pump stations - 10-mile pipeline	$3,396,400	$56,600	$352,700	Good	N	May be difficult to find well with good water quality and pipeline easements must be obtained. Costs could possibly be shared with small systems along pipeline route.
RI-3	Install new compliant well at 5 miles	- New well - 1 Storage tanks - 1 Pump station - 5-mile pipeline	$1,857,200	$37,400	$199,300	Good	N	May be difficult to find well with good water quality and pipeline easements must be obtained. Costs could possibly be shared with small systems along pipeline route.
RI-4	Install new compliant well at 1 mile	- New well - Storage tank - 1-mile pipeline	$526,900	$18,500	$64,400	Good	N	May be difficult to find well with good water quality and pipeline easements must be obtained.
RI-5	Continue operation of Roosevelt ISD well field with central RO treatment	- Central RO treatment plant	$937,900	$81,000	$162,800	Good	T	Costs could possibly be shared with nearby small systems.
RI-6	Continue operation of Roosevelt ISD well field with central EDR treatment	- Central EDR treatment plant	$1,122,100	$75,500	$173,300	Good	T	Costs could possibly be shared with nearby small systems.

Notes: *N – No significant increase required in technical or management capability*
T – Implementation of alternative will require increase in technical capability
M – Implementation of alternative will require increase in management capability
1 – See cost breakdown in Appendix C
2 – 20-year return period and 6 percent interest

4.5 DEVELOPMENT AND EVALUATION OF A REGIONAL SOLUTION

A concept for a regional solution to provide compliant drinking water to PWSs near Lubbock and surrounding counties was developed and evaluated to investigate whether a large-scale regional approach might be more cost-effective than each PWS seeking its own solution. The development and evaluation of the Lubbock Area Regional Solutions is described in Appendix E. It was found that a regional solution to serving non-compliant PWSs in the Lubbock area presents a potentially viable solution to an existing problem. A regional system could be implemented within a cost-per-connection range of $59/month ($711/year) to $189/month ($2,266/year), with the actual cost depending on the source and costs of capital funds needed to build a regional system.

4.6 COST OF SERVICE AND FUNDING ANALYSIS

To evaluate the financial impact of implementing the compliance alternatives, a 30-year financial planning model was developed. This model can be found in Appendix D. Since the model was developed for water systems that collect revenue from paying customers for water usage, it had to be adapted for Roosevelt ISD whose water system costs are funded by property taxes and State funds. Data for such models are typically derived from established budgets, audited financial reports, published water tariffs, and consumption data. Roosevelt ISD did not provide a financial report for fiscal year 2006, and does not track expenses for the water system separately.

Roosevelt ISD is a facility with 11 service connections serving an educational campus/complex with a population of approximately 1,600. Since the Roosevelt ISD is an educational campus there are no revenues from the sale of water. Information that was available to complete the financial analysis included estimated expenses for the water system from Roosevelt ISD personnel, water production capacity data for the Roosevelt ISD from the TCEQ website, and estimated water usage based on a per capita usage rate of 30-gallon per day.

This analysis will need to be performed in a more detailed fashion and applied to alternatives that are deemed attractive and worthy of more detailed evaluation. A more detailed analysis should include additional factors such as:

- Cost escalation,
- Price elasticity effects where increased rates may result in lower water consumption,
- Costs for other system upgrades and rehabilitation needed to maintain compliant operation.

4.6.1 Financial Plan Development

Since financial records for Roosevelt ISD were not available and no revenues are generated from the sale of water, the following assumptions were made to derive estimates for input into the financial planning model. These assumptions were:

1) Water system expenses are $15,000

2) 2006 revenues equal 2006 expenses for operation of the water system.

3) The existing potable water system is paid for and has been fully depreciated

4) A nominal fee per student/teacher for water use was assigned in order to simulate a revenue stream.

5) The average water usage rate per student was assumed to be 30-gallons per day, over a 9-month period (length of the school year)

A consumption rate of 30 gpd per student was assumed which translates into 8,213 gpd per student based on a 9-month school year. Even though school is only in session 5 days per week, a 30-day month was assumed to account for sporting events and other activities held at the school. This amounts to a total of 13,140,000 gallons of water usage per year.

The Roosevelt ISD has a population of 1,600. While students/teachers do not pay for the water they consume, a base rate of $9.50 per person was established which accounts for $15,200 of the water system revenues. This arbitrary value results in a theoretical revenue equal to the $15,000 in operating expenses. These values were used in the financial planning model.

While these assumptions are arbitrary, they help to frame costs of the water system operation and allow impacts of the incremental costs of the various alternatives to be evaluated.

4.6.2 Current Financial Condition

4.6.2.1 Cash Flow Needs

Cash flow needs could not be evaluated for the Roosevelt ISD because the system provides water to the school campus without cost. The school budget covers the operation of the water system. However, since it was assumed that theoretical water revenues are equal to the operating expenses, any capital improvements to the water system would require additional funding.

4.6.2.2 Ratio Analysis

Current Ratio

The Current Ratio for the Roosevelt ISD water system could not be determined due to lack of necessary financial data to determine this ratio.

Debt to Net Worth Ratio

A Debt-to-Net-Worth Ratio also could not be determined owing to lack of the necessary financial data to determine this ratio.

Operating Ratio

Because of the lack of complete separate financial data specifically related to the Roosevelt ISD water system, the Operating Ratio could not be accurately determined.

4.6.3 Financial Plan Results

Each compliance alternative for the Roosevelt ISD was evaluated, with emphasis on the impact on affordability, and the overall increase in water rates necessary to pay for the improvements. Each alternative was examined under the various funding options described in Section 2.4.

Results of the financial impact analysis are provided in Table 4.4 and Figure 4.2. Table 4.4 presents rate impacts assuming that any deficiencies in reserve accounts are funded immediately in the year following the occurrence of the deficiency, which would cause the first few years' water rates to be higher than they would be if the reserve account was built-up over a longer period of time. Figure 4.2 provides a bar chart that, in terms of the yearly billing to an average student (684 gallons/month consumption, over a 9-month period), shows the following:

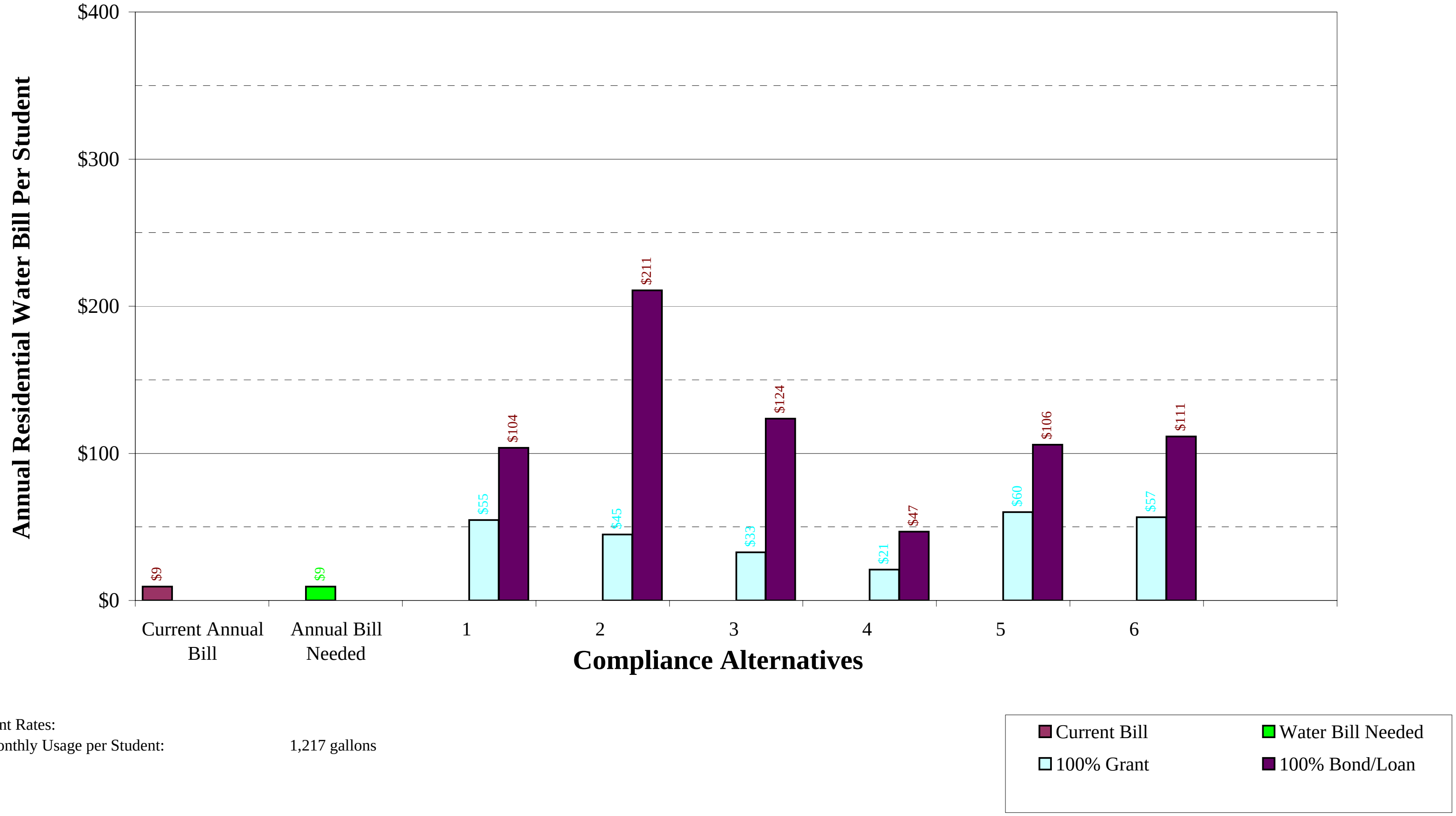
- Current annual average bill,
- Projected annual average bill including rate increase, if needed, to match existing expenditures, and
- Projected annual bill including rate increases needed to fund implementation of a compliance alternative (this does not include funding for reserve accounts).

The two bars shown for each compliance alternative represent the rate changes necessary for revenues to match total expenditures assuming 100 percent grant funding and 100 percent loan/bond funding. Most funding options will fall between 100 percent grant and 100 percent loan/bond funding, with the exception of 100 percent revenue financing. Establishing or increasing reserve accounts would require an increase in rates. If existing reserves are insufficient to fund a compliance alternative, rates would need to be raised before implementing the compliance alternative. This would allow for accumulation of sufficient reserves to avoid larger but temporary rate increases during the years the compliance alternative was being implemented.

Table 4.4 Roosevelt ISD - Financial Impact per Student

Alternative	Description		All Revenue	100% Grant	75% Grant	50% Grant	SRF	Bond
1	Purchase Water from Lubbock-Ransom Canyon	Max % of HH Income	2.0%	0.3%	0.4%	0.4%	0.5%	0.6%
		Max % Rate Increase Compared to Current	7097%	958%	1217%	1476%	1869%	1994%
		Average Water Bill Required by Alternative	$ 613.74	$ 86.86	$ 108.57	$ 130.27	$ 163.20	$ 173.68
2	New Well at 10 Miles	Max % of HH Income	6.3%	0.2%	0.5%	0.7%	1.1%	1.2%
		Max % Rate Increase Compared to Current	22834%	747%	1626%	2505%	3837%	4262%
		Average Water Bill Required by Alternative	$ 1,957.32	$ 69.81	$ 143.46	$ 217.11	$ 328.84	$ 364.42
3	New Well at 5 Miles	Max % of HH Income	3.4%	0.2%	0.3%	0.4%	0.6%	0.7%
		Max % Rate Increase Compared to Current	12527%	493%	974%	1454%	2183%	2415%
		Average Water Bill Required by Alternative	$ 1,077.97	$ 49.30	$ 89.57	$ 129.85	$ 190.94	$ 210.39
4	New Well at 1 Mile	Max % of HH Income	1.0%	0.1%	0.1%	0.2%	0.2%	0.2%
		Max % Rate Increase Compared to Current	3604%	243%	379%	515%	722%	788%
		Average Water Bill Required by Alternative	$ 316.66	$ 29.05	$ 40.48	$ 51.90	$ 69.23	$ 74.75
5	Central Treatment - Reverse Osmosis	Max % of HH Income	1.9%	0.3%	0.4%	0.5%	0.6%	0.6%
		Max % Rate Increase Compared to Current	6736%	1070%	1312%	1555%	1923%	2040%
		Average Water Bill Required by Alternative	$ 582.74	$ 95.86	$ 116.20	$ 136.54	$ 167.39	$ 177.22
6	Central Treatment - Electro-dialysis Reversal	Max % of HH Income	2.2%	0.3%	0.4%	0.5%	0.6%	0.6%
		Max % Rate Increase Compared to Current	7917%	997%	1287%	1577%	2017%	2158%
		Average Water Bill Required by Alternative	$ 683.69	$ 89.94	$ 114.27	$ 138.61	$ 175.52	$ 187.27

Figure 4-2 Roosevelt ISD - Alternative Cost Summary



Current Rates:
Monthly Usage per Student: 1,217 gallons

SECTION 5
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APPENDIX A
PWS INTERVIEW FORM

CAPACITY DEVELOPMENT ASSESSMENT FORM

*Prepared By*___________________________________ *Date*___________________________

Section 1. Public Water System Information

1. PWS ID # 2. Water System Name

3. County

4. Owner Address

Tele. E-mail

Fax Message

5. Admin Address

Tele. E-mail

Fax Message

6. Operator Address

Tele. E-mail

Fax Message

7. Population Served 8. No. of Service Connections

9. Ownership Type 10. Metered (Yes or No)

11. Source Type

12. Total PWS Annual Water Used

13. Number of Water Quality Violations (Prior 36 months)

Total Coliform Chemical/Radiological

Monitoring (CCR, Public Notification, etc.) Treatment Technique, D/DBP

A. Basic Information

1. Name of Water System:

2. Name of Person Interviewed:

3. Position:

4. Number of years at job:

5. Number of years experience with drinking water systems:

6. Percent of time (day or week) on drinking water system activities, with current position (how much time is dedicated exclusively to the water system, not wastewater, solid waste or other activities):

7. Certified Water Operator (Yes or No):

   If Yes,
   7a. Certification Level (water):

   7b. How long have you been certified?

8. Describe your water system related duties on a typical day.

B. Organization and Structure

1. Describe the organizational structure of the Utility. Please provide an organizational chart. (Looking to find out the governance structure (who reports to whom), whether or not there is a utility board, if the water system answers to public works or city council, etc.)

2. If not already covered in Question 1, to whom do you report?

3. Do all of the positions have a written job description?

   3a. If yes, is it available to employees?

   3b. May we see a copy?

C. Personnel

1. What is the current staffing level (include all personnel who spend more than 10% of their time working on the water system)?

2. Are there any vacant positions? How long have the positions been vacant?

3. In your opinion, is the current staffing level adequate? If not adequate, what are the issues or staffing needs (how many and what positions)?

4. What is the rate of employee turnover for management and operators? What are the major issues involved in the turnover (e.g., operator pay, working conditions, hours)?

5. Is the system staffed 24 hours a day? How is this handled (on-site or on-call)? Is there an alarm system to call an operator if an emergency occurs after hours?

D. Communication

1. Does the utility have a mission statement? If yes, what is it?

2. Does the utility have water quality goals? What are they?

3. How are your work priorities set?

4. How are work tasks delegated to staff?

5. Does the utility have regular staff meetings? How often? Who attends?

6. Are there separate management meetings? If so, describe.

7. Do management personnel ever visit the treatment facility? If yes, how often?

8. Is there effective communication between utility management and state regulators (e.g., NMED)?

9. Describe communication between utility and customers.

E. Planning and Funding

1. Describe the rate structure for the utility.

2. Is there a written rate structure, such as a rate ordinance? May we see it?

    2a. What is the average rate for 6,000 gallons of water?

3. How often are the rates reviewed?

4. What process is used to set or revise the rates?

5. In general, how often are the new rates set?

6. Is there an operating budget for the water utility? Is it separate from other activities, such as wastewater, other utilities, or general city funds?

7. Who develops the budget, how is it developed and how often is a new budget created or the old budget updated?

8. How is the budget approved or adopted?

9. In the last 5 years, how many budget shortfalls have there been (i.e., didn't collect enough money to cover expenses)? What caused the shortfall (e.g., unpaid bills, an emergency repair, weather conditions)?

    9a. How are budget shortfalls handled?

10. In the last 5 years how many years have there been budget surpluses (i.e., collected revenues exceeded expenses?

    10a. How are budget surpluses handled (i.e., what is done with the money)?

11. Does the utility have a line-item in the budget for emergencies or some kind of emergency reserve account?

12. How do you plan and pay for short-term system needs?

13. How do you plan and pay for long- term system needs?

14. How are major water system capital improvements funded? Does the utility have a written capital improvements plan?

15. How is the facility planning for future growth (either new hook-ups or expansion into new areas)?

16. Does the utility have and maintain an annual financial report? Is it presented to policy makers?

17. Has an independent financial audit been conducted of the utility finances? If so, how often? When was the last one?

18. Will the system consider any type of regionalization with any other PWS, such as system interconnection, purchasing water, sharing operator, emergency water connection, sharing bookkeeper/billing or other?

F. Policies, Procedures, and Programs

1. Are there written operational procedures? Do the employees use them?

2. Who in the utility department has spending authorization? What is the process for obtaining needed equipment or supplies, including who approves expenditures?

3. Does the utility have a source water protection program? What are the major components of the program?

4. Are managers and operators familiar with current SDWA regulations?

5. How do the managers and operators hear about new or proposed regulations, such as arsenic, DBP, Groundwater Rule? Are there any new regulations that will be of particular concern to the utility?

6. What are the typical customer complaints that the utility receives?

7. Approximately how many complaints are there per month?

8. How are customer complaints handled? Are they recorded?

9. (If not specifically addressed in Question 7) If the complaint is of a water quality nature, how are these types of complaints handled?

10. Does the utility maintain an updated list of critical customers?

11. Is there a cross-connection control plan for the utility? Is it written? Who enforces the plan's requirements?

12. Does the utility have a written water conservation plan?

13. Has there been a water audit of the system? If yes, what were the results?

14. (If not specifically answered in 11 above) What is the estimated percentage for loss to leakage for the system?

15. Are you, or is the utility itself, a member of any trade organizations, such as AWWA or Rural Water Association? Are you an active member (i.e., attend regular meetings or participate in a leadership role)? Do you find this membership helpful? If yes, in what ways does it help you?

G. Operations and Maintenance

1. How is decision-making authority split between operations and management for the following items:

    a. Process Control

    b. Purchases of supplies or small equipment

    c. Compliance sampling/reporting

    d. Staff scheduling

2. Describe your utility's preventative maintenance program.

3. Do the operators have the ability to make changes or modify the preventative maintenance program?

4. How does management prioritize the repair or replacement of utility assets? Do the operators play a role in this prioritization process?

5. Does the utility keep an inventory of spare parts?

6. Where does staff have to go to buy supplies/minor equipment? How often?

    6a. How do you handle supplies that are critical, but not in close proximity (for example if chlorine is not available in the immediate area or if the components for a critical pump are not in the area)

7. Describe the system's disinfection process. Have you had any problems in the last few years with the disinfection system?

    7a. Who has the ability to adjust the disinfection process?

8. How often is the disinfectant residual checked and where is it checked?

    8a. Is there an official policy on checking residuals or is it up to the operators?

9. Does the utility have an O & M manual? Does the staff use it?

10. Are the operators trained on safety issues? How are they trained and how often?

11. Describe how on-going training is handled for operators and other staff. How do you hear about appropriate trainings? Who suggests the trainings – the managers or the operators? How often do operators, managers, or other staff go to training? Who are the typical trainers used and where are the trainings usually held?

12. In your opinion is the level of your on-going training adequate?

13. In your opinion is the level of on-going training for other staff members, particularly the operators, adequate?

14. Does the facility have mapping of the water utility components? Is it used on any routine basis by the operators or management? If so, how is it used? If not, what is the process used for locating utility components?

15. In the last sanitary survey, were any deficiencies noted? If yes, were they corrected?

16. How often are storage tanks inspected? Who does the inspection?

    16a. Have you experienced any problems with the storage tanks?

H. SDWA Compliance

1. Has the system had any violations (monitoring or MCL) in the past 3 years? If so, describe.

2. How were the violations handled?

3. Does the system properly publish public notifications when notified of a violation?

4. Is the system currently in violation of any SDWA or state regulatory requirements, including failure to pay fees, fines, or other administrative type requirements?

5. Does the utility prepare and distribute a Consumer Confidence Report (CCR)? Is it done every year? What type of response does the utility get to the CCR from customers?

I. Emergency Planning

1. Does the system have a written emergency plan to handle emergencies such as water outages, weather issues, loss of power, loss of major equipment, etc?

2. When was the last time the plan was updated?

3. Do all employees know where the plan is? Do they follow it?

4. Describe the last emergency the facility faced and how it was handled.

Attachment A

A. Technical Capacity Assessment Questions

1. Based on available information of water rights on record and water pumped has the system exceeded its water rights in the past year? YES ☐ NO ☐

   In any of the past 5 years? YES ☐ NO ☐ How many times? _____

2. Does the system have the proper level of certified operator? *(Use questions a – c to answer.)*
   YES ☐ NO ☐

   a. What is the Classification Level of the system by NMED? _____

   b. Does the system have one or more certified operator(s)? [20 NMAC 7.4.20]

   YES ☐ NO ☐

   c. If YES, provide the number of operators at each New Mexico Certification Level. [20 NMAC 7.4.12]

   _____NM Small System _____Class 2

   _____NM Small System Advanced _____Class 3

   _____Class 1 _____Class 4

3. Did the system correct any sanitary deficiency noted on the most recent sanitary survey within 6 months of receiving that information? [20 NMAC 7.20.504]

   YES ☐ NO ☐ No Deficiencies ☐

   What was the type of deficiency? *(Check all that are applicable.)*

   Source ☐ Storage ☐

   Treatment ☐ Distribution ☐

   Other _____

   From the system's perspective, were there any other deficiencies that were not noted on the sanitary survey? Please describe.

4. Will the system's current treatment process meet known future regulations?

   Radionuclides YES ☐ NO ☐ Doesn't Apply ☐

   Arsenic YES ☐ NO ☐ Doesn't Apply ☐

   Stage 1 Disinfectants and Disinfection By-Product (DBP)

   YES ☐ NO ☐ Doesn't Apply ☐

   Surface Water Treatment Rule YES ☐ NO ☐ Doesn't Apply ☐

5. Does the system have a current site plan/map? [20 NMAC 7.10.302 A.1.]

   YES ☐ NO ☐

6. Has the system had a water supply outage in the prior 24 months?

YES ☐ NO ☐

What were the causes of the outage(s)? *(Include number of outages for each cause.)*

Drought _____ Limited Supply _____

System Failure _____ Other _____

7. Has the system ever had a water audit or a leak evaluation?

YES ☐ NO ☐ Don't Know ☐

If YES, please complete the following table.

Type of Investigation	Date Done	Water Loss (%)	What approach or technology was used to complete the investigation?	Was any follow-up done? If so, describe

8. Have all drinking water projects received NMED review and approval? [20 NMAC 7.10.201]

YES ☐ NO ☐

If NO, what types of projects have not received NMED review and approval.

Source ☐ Storage ☐

Treatment ☐ Distribution ☐

Other ☐ _____

9. What are the typical customer complaints that the utility receives?

10. Approximately how many complaints are there per month? _____

11. How are customer complaints handled? Are they recorded?

12. What is the age and composition of the distribution system? ***(Collect this information from the Sanitary Survey)***

Pipe Material	Approximate Age	Percentage of the system	Comments
			Sanitary Survey Distribution System Records Attached

13. Are there any dead end lines in the system?

YES ☐ NO ☐

14. Does the system have a flushing program?

YES ☐ NO ☐

If YES, please describe.

15. Are there any pressure problems within the system?

YES ☐ NO ☐

If YES, please describe.

16. Does the system disinfect the finished water?

YES ☐ NO ☐

If yes, which disinfectant product is used? ______

Interviewer Comments on Technical Capacity:

B. Managerial Capacity Assessment Questions

17. Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan?

YES ☐ NO ☐

If YES, has the plan been submitted to Local Government Division?

YES ☐ NO ☐

18. Does the system have written operating procedures?

YES ☐ NO ☐

19. Does the system have written job descriptions for all staff?

YES ☐ NO ☐

20. Does the system have:

A preventative maintenance plan?
YES ☐ NO ☐
A source water protection plan?
YES ☐ NO ☐ N/A ☐
An emergency plan?
YES ☐ NO ☐
A cross-connection control program?
YES ☐ NO ☐
An emergency source?
YES ☐ NO ☐
System security measures?
YES ☐ NO ☐

21. Does the system report and maintain records in accordance with the drinking water regulations concerning:

Water quality violations
YES ☐ NO ☐

Public notification
YES ☐ NO ☐

Sampling exemptions
YES ☐ NO ☐

22. Please describe how the above records are maintained:

23. Describe the management structure for the water system, including board and operations staff. Please include examples of duties, if possible.

24. Please describe type and quantity of training or continuing education for staff identified above.

25. Describe last major project undertaken by the water system, including the following: project in detail, positive aspects, negative aspects, the way in which the project was funded, any necessary rate increases, the public response to the project, whether the project is complete or not, and any other pertinent information.

26. Does the system have any debt? YES ☐ NO ☐

If yes, is the system current with all debt payments?
YES ☐ NO ☐

If no, describe the applicable funding agency and the default.

27. Is the system currently contemplating or actively seeking funding for any project?
YES ☐ NO ☐

If yes, from which agency and how much?

Describe the project?

Is the system receiving assistance from any agency or organization in its efforts?

28. Will the system consider any type of regionalization with other PWS? *(Check YES if the system has already regionalized.)*

YES ☐ NO ☐

If YES, what type of regionalization has been implemented/considered/discussed? *(Check all that apply.)*

- System interconnection ☐
- Sharing operator ☐
- Sharing bookkeeper ☐
- Purchasing water ☐
- Emergency water connection ☐
- Other: _____

29. Does the system have any of the following? *(Check all that apply.)*

- Water Conservation Policy/Ordinance ☐
- Current Drought Plan ☐
- Water Use Restrictions ☐
- Water Supply Emergency Plan ☐

Interviewer Comments on Managerial Capacity:

C. Financial Capacity Assessment

30. Does the system have a budget?

YES ☐ NO ☐

If YES, what type of budget?

Operating Budget ☐

Capital Budget ☐

31. Have the system revenues covered expenses and debt service for the past 5 years?

YES ☐ NO ☐

If NO, how many years has the system had a shortfall? _____

32. Does the system have a written/adopted rate structure?

YES ☐ NO ☐

33. What was the date of the last rate increase? _____

34. Are rates reviewed annually?

YES ☐ NO ☐

IF YES, what was the date of the last review? _____

35. Did the rate review show that the rates covered the following expenses? *(Check all that apply.)*

Operation & Maintenance ☐

Infrastructure Repair & replacement ☐

Staffing ☐

Emergency/Reserve fund ☐

Debt payment ☐

36. Is the rate collection above 90% of the customers?

YES ☐ NO ☐

37. Is there a cut-off policy for customers who are in arrears with their bill or for illegal connections?

YES ☐ NO ☐

If yes, is this policy implemented?

38. What is the residential water rate for 6,000 gallons of usage in one month. _____

39. In the past 12 months, how many customers have had accounts frozen or dropped for non-payment? _____

[Convert to % of active connections

Less than 1% ☐ 1% - 3% ☐ 4% - 5% ☐ 6% - 10% ☐

11% - 20% ☐ 21% - 50% ☐ Greater than 50% ☐]

40. The following questions refer to the process of obtaining needed equipment and supplies.

a. Can the water system operator buy or obtain supplies or equipment when they are needed?

YES ☐ NO ☐

b. Is the process simple or burdensome to the employees?

c. Can supplies or equipment be obtained quickly during an emergency?

YES ☐ NO ☐

d. Has the water system operator ever experienced a situation in which he/she couldn't purchase the needed supplies?

YES ☐ NO ☐

e. Does the system maintain some type of spare parts inventory?

YES ☐ NO ☐

If yes, please describe.

41. Has the system ever had a financial audit?

YES ☐ NO ☐

If YES, what is the date of the most recent audit? ______

42. Has the system ever had its electricity or phone turned off due to non-payment? Please describe.

Interviewer Comments on Financial Assessment:

43. What do you think the system capabilities are now and what are the issues you feel your system will be facing in the future? In addition, are there any specific needs, such as types of training that you would like to see addressed by NMED or its contractors?

APPENDIX B
COST BASIS

This section presents the basis for unit costs used to develop the conceptual cost estimates for the compliance alternatives. Cost estimates are conceptual in nature (+50%/-30%), and are intended to make comparisons between compliance options and to provide a preliminary indication of possible rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. Capital cost includes an allowance for engineering and construction management. It is assumed that adequate electrical power is available near the site. The cost estimates specifically do not include costs for the following:

- Obtaining land or easements.
- Surveying.
- Mobilization/demobilization for construction.
- Insurance and bonds

In general, unit costs are based on recent construction bids for similar work in the area when possible, consultations with vendors or other suppliers, published construction and O&M cost data, and USEPA cost guidance. Unit costs used for the cost estimates are summarized in Table B.1.

Unit costs for pipeline components are based on 2007 RS Means Site Work & Landscape Cost Data. The number of borings and encasements and open cuts and encasements is estimated by counting the road, highway, railroad, stream, and river crossings for a conceptual routing of the pipeline. The number of air release valves is estimated by examining the land surface profile along the conceptual pipeline route. It is assumed that gate valves and flush valves would be installed, on average, every 5,000 feet along the pipeline. Pipeline cost estimates are based on the use of C-900 PVC pipe. Other pipe materials could be considered for more detailed development of attractive alternatives.

Pump station unit costs are based on experience with similar installations. The cost estimate for the pump stations include two pumps, station piping and valves, station electrical and instrumentation, minor site improvement, installation of a concrete pad, fence and building, and tools. The number of pump stations is based on calculations of pressure losses in the proposed pipeline for each alternative. Back-flow prevention is required in cases where pressure losses are negligible, and pump stations are not needed. Construction cost of a storage tank is based on consultations with vendors and 2007 RS Means Site Work & Landscape Cost Data.

Labor costs are estimated based on 2007 RS Means Site Work & Landscape Cost Data specific to the Lubbock County region.

Electrical power cost is estimated to be $0.043 per kWH, as supplied by Xcel Energy. The annual cost for power to a pump station is calculated based on the pumping head and volume, and includes 11,800 kWH for pump building heating, cooling, and lighting, as recommended in USEPA's *Standardized Costs for Water Supply Distribution Systems* (USEPA 1992).

In addition to the cost of electricity, pump stations have other maintenance costs. These costs cover: materials for minor repairs to keep the pumps operating; purchase of a maintenance vehicle, fuel costs, and vehicle maintenance costs; utilities; office supplies, small tools and equipment; and miscellaneous materials such as safety, clothing, chemicals, and paint. The non-power O&M costs are estimated based on the USEPA's, *Standardized Costs for Water Supply Distribution Systems* (USEPA 1992), which provides cost curves for O&M components. Costs from the 1992 report are adjusted to 2007 dollars based on the ENR construction cost index.

Pipeline maintenance costs include routine cleaning and flushing, as well as minor repairs to lines. The unit rate for pipeline maintenance is calculated based on the USEPA technical report, *Innovative and Alternate Technology Assessment Manual MCD 53* (USEPA 1980). Costs from the 1980 report are adjusted to 2007 dollars based on the ENR construction cost index.

Storage tank maintenance costs include cleaning and renewal of interior lining and exterior coating. Unit costs for storage tank O&M are based on USEPA's *Standardized Costs for Water Supply Distribution Systems* (USEPA 1992). Costs from the 1992 report are adjusted to 2007 dollars based on the ENR construction cost index.

Central treatment plant costs, for both adsorption and coagulation/filtration, include pricing for buildings, utilities, and site work. Costs are based on pricing given in the various 2007 RS Means Cost Data references, as well as prices obtained from similar work on other projects. Pricing for treatment equipment was obtained from vendors.

Well installation costs are based on quotations from drillers for installation of similar depth wells in the area. Well installation costs include drilling, a well pump, electrical and instrumentation installation, well finishing, piping, and water quality testing. O&M costs for water wells include power, materials, and labor.

Table B.1
Summary of General Data
Roosevelt ISD
1520123
General PWS Information

Service Population	1,600	**Number of Connections**	11
Total PWS Daily Water Usage	0.048 (mgd)	**Source**	Site visit list

Unit Cost Data

General Items	**Unit**	**Unit Cost**
Treated water purchase cost	*See alternative*	
Water purchase cost (trucked)	$/1,000 gals	$ 2.61
Contingency	20%	n/a
Engineering & Constr. Management	25%	n/a
Procurement/admin (POU/POE)	20%	n/a

Pipeline Unit Costs	**Unit**	**Unit Cost**
PVC water line, Class 200, 06"	LF	$ 32
Bore and encasement, 10"	LF	$ 240
Open cut and encasement, 10"	LF	$ 105
Gate valve and box, 06"	EA	$ 915
Air valve	EA	$ 2,000
Flush valve	EA	$ 1,000
Metal detectable tape	LF	$ 2
Bore and encasement, length	Feet	200
Open cut and encasement, length	Feet	50

Pump Station Unit Costs	**Unit**	**Unit Cost**
Pump	EA	$ 8,000
Pump Station Piping, 06"	EA	$ 815
Gate valve, 06"	EA	$ 915
Check valve, 06"	EA	$ 915
Electrical/Instrumentation	EA	$ 10,000
Site work	EA	$ 2,500
Building pad	EA	$ 5,000
Pump Building	EA	$ 10,000
Fence	EA	$ 6,000
Tools	EA	$ 1,000

Well Installation Unit Costs	**Unit**	**Unit Cost**
Well installation	*See alternative*	
Water quality testing	EA	$ 1,250
Well pump	EA	$ 10,000
Well electrical/instrumentation	EA	$ 5,500
Well cover and base	EA	$ 3,000
Piping	EA	$ 3,000
30,000 gal storage / feed tank	EA	$ 45,000
Electrical Power	$/kWH	$ 0.043
Building Power	kWH	11,800
Labor	$/hr	$ 68
Materials	EA	$ 1,500
Transmission main O&M	$/mile	$ 250
Tank O&M	EA	$ 1,000

POU/POE Unit Costs		
POU treatment unit purchase	EA	$ 600
POU treatment unit installation	EA	$ 150
POE treatment unit purchase	EA	$ 5,000
POE - pad and shed, per unit	EA	$ 2,000
POE - piping connection, per unit	EA	$ 1,000
POE - electrical hook-up, per unit	EA	$ 1,000
POU Treatment O&M, per unit	$/year	$ 225
POE Treatment O&M, per unit	$/year	$ 1,500
Treatment analysis	$/year	$ 200
POU/POE labor support	$/hr	$ 50

Dispenser/Bottled Water Unit Costs		
POE-Treatment unit purchase	EA	$ 7,000
POE-Treatment unit installation	EA	$ 5,000
Treatment unit O&M	EA	$ 2,000
Administrative labor	hr	$ 40
Bottled water cost (inc. delivery)	gallon	$ 1
Water use, per capita per day	gpcd	1.0
Bottled water program materials	EA	$ 5,000
10,000 gal storage / feed tank	EA	$ 20,000
Site improvements	EA	$ 3,000
Potable water truck	EA	$ 75,000
Water analysis, per sample	EA	$ 200
Potable water truck O&M costs	$/mile	$ 2

Central Treatment Unit Costs	**Unit**	**Unit Cost**
General		
Site preparation	acre	$ 4,000
Slab	CY	$ 1,000
Building	SF	$ 60
Building electrical	SF	$ 8
Building plumbing	SF	$ 8
Heating and ventilation	SF	$ 7
Fence	LF	$ 15
Paving	SF	$ 2
Reject pond, excavation	CYD	$ 3
Reject pond, compacted fill	CYD	$ 7
Reject pond, lining	SF	$ 0.50
Reject pond, vegetation	SY	$ 1
Reject pond, access road	LF	$ 30
Reject water haulage truck	EA	$ 100,000
Chlorination point	EA	$ 2,000
Building power	$/kWH	$ 0.043
Equipment power	$/kWH	$ 0.043
Labor, O&M	hr	$ 40
Analyses	test	$ 200
Reverse Osmosis		
Electrical	JOB	$ 60,000
Piping	JOB	$ 30,000
RO package plant	UNIT	$ 235,000
Feed pump	EA	$ 8,000
Permeate tank	gal	$ 3
RO materials	year	$ 6,000
RO chemicals	year	$ 3,000
Backwash disposal mileage cost	miles	$ 1.00
Backwash disposal fee	1,000 gal/yr	$ 5
EDR		
Electrical	JOB	$ 60,000
Piping	JOB	$ 30,000
Product storage tank	gal	$ 3
EDR package plant	UNIT	$ 350,000
Feed pump	EA	$ 8,000
Transfer pump (5hp)	EA	$ 6,000
EDR materials	year	$ 6,000
EDR chemicals	year	$ 3,000
Backwash disposal mileage cost	miles	$ 1
Backwash disposal fee	1,000 gal/yr	$ 5

APPENDIX C
COMPLIANCE ALTERNATIVE CONCEPTUAL COST ESTIMATES

This appendix presents the conceptual cost estimates developed for the compliance alternatives. The conceptual cost estimates are given in Tables C.1 through C.6. The cost estimates are conceptual in nature (+50%/-30%), and are intended for making comparisons between compliance options and to provide a preliminary indication of possible water rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation.

Table C.1

PWS Name ***Roosevelt ISD***
Alternative Name ***Purchase Water from Lubbock-Ransom Canyon***
Alternative Number ***RI-1***

Distance from Alternative to PWS (along pipe)	2.59	miles
Total PWS annual water usage	17.520	MG
Treated water purchase cost	$ 2.61	per 1,000 gals
Pump Stations needed w/ 1 feed tank each	1	
On site storage tanks / pump sets needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	-	n/a	n/a	n/a
Number of Crossings, open cut	2	n/a	n/a	n/a
PVC water line, Class 200, 06"	13,675	LF	$ 32	$ 437,606
Bore and encasement, 10"	-	LF	$ 240	$ -
Open cut and encasement, 10"	100	LF	$ 105	$ 10,500
Gate valve and box, 06"	3	EA	$ 915	$ 2,503
Air valve	3	EA	$ 2,000	$ 6,000
Flush valve	3	EA	$ 1,000	$ 2,735
Metal detectable tape	13,675	LF	$ 2	$ 27,350
Subtotal				**$ 486,694**
Pump Station(s) Installation				
Pump	4	EA	$ 8,000	$ 32,000
Pump Station Piping, 06"	2	EA	$ 815	$ 1,630
Gate valve, 06"	8	EA	$ 915	$ 7,320
Check valve, 06"	4	EA	$ 915	$ 3,660
Electrical/Instrumentation	2	EA	$ 10,000	$ 20,000
Site work	2	EA	$ 2,500	$ 5,000
Building pad	2	EA	$ 5,000	$ 10,000
Pump Building	2	EA	$ 10,000	$ 20,000
Fence	2	EA	$ 6,000	$ 12,000
Tools	2	EA	$ 1,000	$ 2,000
30,000 gal storage / feed tank	2	EA	$ 45,000	$ 90,000
Subtotal				**$ 203,610**
Subtotal of Component Costs				**$ 690,304**
Contingency	20%			$ 138,061
Design & Constr Management	25%			$ 172,576
TOTAL CAPITAL COSTS				**$ 1,000,941**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	2.59	mile	$ 250	$ 648
Subtotal				**$ 648**
Water Purchase Cost				
From PWS	17,520	1,000 gal	$ 2.61	$ 45,727
Subtotal				**$ 45,727**
Pump Station(s) O&M				
Building Power	23,600	kWH	$ 0.043	$ 1,015
Pump Power	7,258	kWH	$ 0.043	$ 312
Materials	2	EA	$ 1,500	$ 3,000
Labor	730	Hrs	$ 40	$ 29,200
Tank O&M	2	EA	$ 1,000	$ 2,000
Subtotal				**$ 35,527**
O&M Credit for Existing Well Closure				
Pump power	14,514	kWH	$ 0.043	$ (624)
Well O&M matl	1	EA	$ 1,500	$ (1,500)
Well O&M labor	180	Hrs	$ 40	$ (7,200)
Subtotal				**$ (9,324)**
TOTAL ANNUAL O&M COSTS				**$ 72,577**

Table C.2

PWS Name *Roosevelt ISD*
Alternative Name *New Well at 10 Miles*
Alternative Number *RI-2*

Distance from PWS to new well location	10.0	miles
Estimated well depth	300	feet
Number of wells required	1	
Well installation cost (location specific)	$145	per foot
Pump Stations needed w/ 1 feed tank each	2	
On site storage tanks / pump sets needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	1	n/a	n/a	n/a
Number of Crossings, open cut	16	n/a	n/a	n/a
PVC water line, Class 200, 06"	52,800	LF	$ 32	$ 1,689,600
Bore and encasement, 10"	200	LF	$ 240	$ 48,000
Open cut and encasement, 10"	800	LF	$ 105	$ 84,000
Gate valve and box, 06"	11	EA	$ 915	$ 9,662
Air valve	11	EA	$ 2,000	$ 22,000
Flush valve	11	EA	$ 1,000	$ 10,560
Metal detectable tape	52,800	LF	$ 2	$ 105,600
Subtotal				**$ 1,969,422**
Pump Station(s) Installation				
Pump	6	EA	$ 8,000	$ 48,000
Pump Station Piping, 06"	3	EA	$ 815	$ 2,445
Gate valve, 06"	12	EA	$ 915	$ 10,980
Check valve, 06"	6	EA	$ 915	$ 5,490
Electrical/Instrumentation	3	EA	$ 10,000	$ 30,000
Site work	3	EA	$ 2,500	$ 7,500
Building pad	3	EA	$ 5,000	$ 15,000
Pump Building	3	EA	$ 10,000	$ 30,000
Fence	3	EA	$ 6,000	$ 18,000
Tools	3	EA	$ 1,000	$ 3,000
30,000 gal storage / feed tank	3	EA	$ 45,000	$ 135,000
Subtotal				**$ 305,415**
Well Installation				
Well installation	300	LF	$ 145	$ 43,500
Water quality testing	2	EA	$ 1,250	$ 2,500
Well pump	1	EA	$ 10,000	$ 10,000
Well electrical/instrumentation	1	EA	$ 5,500	$ 5,500
Well cover and base	1	EA	$ 3,000	$ 3,000
Piping	1	EA	$ 3,000	$ 3,000
Subtotal				**$ 67,500**
Subtotal of Component Costs				**$ 2,342,337**
Contingency	20%			$ 468,467
Design & Constr Management	25%			$ 585,584
TOTAL CAPITAL COSTS				**$ 3,396,389**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	10.0	mile	$ 250	$ 2,500
Subtotal				**$ 2,500**
Pump Station(s) O&M				
Building Power	35,400	kWH	$ 0.043	$ 1,522
Pump Power	15,648	kWH	$ 0.043	$ 673
Materials	3	EA	$ 1,500	$ 4,500
Labor	1,095	Hrs	$ 40	$ 43,800
Tank O&M	3	EA	$ 1,000	$ 3,000
Subtotal				**$ 53,495**
Well O&M				
Pump power	28,980	kWH	$ 0.043	$ 1,246
Well O&M matl	1	EA	$ 1,500	$ 1,500
Well O&M labor	180	Hrs	$ 40	$ 7,200
Subtotal				**$ 9,946**
O&M Credit for Existing Well Closure				
Pump power	14,514	kWH	$ 0.043	$ (624)
Well O&M matl	1	EA	$ 1,500	$ (1,500)
Well O&M labor	180	Hrs	$ 40	$ (7,200)
Subtotal				**$ (9,324)**
TOTAL ANNUAL O&M COSTS				**$ 56,617**

Table C.3

PWS Name ***Roosevelt ISD***
Alternative Name ***New Well at 5 Miles***
Alternative Number ***RI-3***

Distance from PWS to new well location	5.0	miles
Estimated well depth	300	feet
Number of wells required	1	
Well installation cost (location specific)	$145	per foot
Pump Stations needed w/ 1 feed tank each	1	
On site storage tanks / pump sets needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	1	n/a	n/a	n/a
Number of Crossings, open cut	8	n/a	n/a	n/a
PVC water line, Class 200, 06"	26,400	LF	$ 32	$ 844,800
Bore and encasement, 10"	200	LF	$ 240	$ 48,000
Open cut and encasement, 10"	400	LF	$ 105	$ 42,000
Gate valve and box, 06"	5	EA	$ 915	$ 4,831
Air valve	6	EA	$ 2,000	$ 12,000
Flush valve	5	EA	$ 1,000	$ 5,280
Metal detectable tape	26,400	LF	$ 2	$ 52,800
Subtotal				**$ 1,009,711**
Pump Station(s) Installation				
Pump	4	EA	$ 8,000	$ 32,000
Pump Station Piping, 06"	2	EA	$ 815	$ 1,630
Gate valve, 06"	8	EA	$ 915	$ 7,320
Check valve, 06"	4	EA	$ 915	$ 3,660
Electrical/Instrumentation	2	EA	$ 10,000	$ 20,000
Site work	2	EA	$ 2,500	$ 5,000
Building pad	2	EA	$ 5,000	$ 10,000
Pump Building	2	EA	$ 10,000	$ 20,000
Fence	2	EA	$ 6,000	$ 12,000
Tools	2	EA	$ 1,000	$ 2,000
30,000 gal storage / feed tank	2	EA	$ 45,000	$ 90,000
Subtotal				**$ 203,610**
Well Installation				
Well installation	300	LF	$ 145	$ 43,500
Water quality testing	2	EA	$ 1,250	$ 2,500
Well pump	1	EA	$ 10,000	$ 10,000
Well electrical/instrumentation	1	EA	$ 5,500	$ 5,500
Well cover and base	1	EA	$ 3,000	$ 3,000
Piping	1	EA	$ 3,000	$ 3,000
Subtotal				**$ 67,500**
Subtotal of Component Costs				**$ 1,280,821**
Contingency	20%			$ 256,164
Design & Constr Management	25%			$ 320,205
TOTAL CAPITAL COSTS				**$ 1,857,191**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	5.0	mile	$ 250	$ 1,250
Subtotal				**$ 1,250**
Pump Station(s) O&M				
Building Power	23,600	kWH	$ 0.043	$ 1,015
Pump Power	7,824	kWH	$ 0.043	$ 336
Materials	2	EA	$ 1,500	$ 3,000
Labor	730	Hrs	$ 40	$ 29,200
Tank O&M	2	EA	$ 1,000	$ 2,000
Subtotal				**$ 35,551**
Well O&M				
Pump power	28,980	kWH	$ 0.043	$ 1,246
Well O&M matl	1	EA	$ 1,500	$ 1,500
Well O&M labor	180	Hrs	$ 40	$ 7,200
Subtotal				**$ 9,946**
O&M Credit for Existing Well Closure				
Pump power	14,514	kWH	$ 0.043	$ (624)
Well O&M matl	1	EA	$ 1,500	$ (1,500)
Well O&M labor	180	Hrs	$ 40	$ (7,200)
Subtotal				**$ (9,324)**
TOTAL ANNUAL O&M COSTS				**$ 37,423**

Table C.4

PWS Name ***Roosevelt ISD***
Alternative Name ***New Well at 1 Mile***
Alternative Number ***RI-4***

Distance from PWS to new well location	1.0	miles
Estimated well depth	300	feet
Number of wells required	1	
Well installation cost (location specific)	$145	per foot
Pump Stations needed w/ 1 feed tank each	0	
On site storage tanks / pump sets needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	-	n/a	n/a	n/a
Number of Crossings, open cut	2	n/a	n/a	n/a
PVC water line, Class 200, 06"	5,280	LF	$ 32	$ 168,960
Bore and encasement, 10"	-	LF	$ 240	$ -
Open cut and encasement, 10"	100	LF	$ 105	$ 10,500
Gate valve and box, 06"	1	EA	$ 915	$ 966
Air valve	1	EA	$ 2,000	$ 2,000
Flush valve	1	EA	$ 1,000	$ 1,056
Metal detectable tape	5,280	LF	$ 2	$ 10,560
Subtotal				**$ 194,042**
Pump Station(s) Installation				
Pump	2	EA	$ 8,000	$ 16,000
Pump Station Piping, 06"	1	EA	$ 815	$ 815
Gate valve, 06"	4	EA	$ 915	$ 3,660
Check valve, 06"	2	EA	$ 915	$ 1,830
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,500	$ 2,500
Building pad	1	EA	$ 5,000	$ 5,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 6,000	$ 6,000
Tools	1	EA	$ 1,000	$ 1,000
30,000 gal storage / feed tank	1	EA	$ 45,000	$ 45,000
Subtotal				**$ 101,805**
Well Installation				
Well installation	300	LF	$ 145	$ 43,500
Water quality testing	2	EA	$ 1,250	$ 2,500
Well pump	1	EA	$ 10,000	$ 10,000
Well electrical/instrumentation	1	EA	$ 5,500	$ 5,500
Well cover and base	1	EA	$ 3,000	$ 3,000
Piping	1	EA	$ 3,000	$ 3,000
Subtotal				**$ 67,500**
Subtotal of Component Costs				**$ 363,347**
Contingency	20%			$ 72,669
Design & Constr Management	25%			$ 90,837
TOTAL CAPITAL COSTS				**$ 526,853**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	1.0	mile	$ 250	$ 250
Subtotal				**$ 250**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.043	$ 507
Pump Power	-	kWH	$ 0.043	$ -
Materials	1	EA	$ 1,500	$ 1,500
Labor	365	Hrs	$ 40	$ 14,600
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 17,607**
Well O&M				
Pump power	28,980	kWH	$ 0.043	$ 1,246
Well O&M matl	1	EA	$ 1,500	$ 1,500
Well O&M labor	180	Hrs	$ 40	$ 7,200
Subtotal				**$ 9,946**
O&M Credit for Existing Well Closure				
Pump power	14,514	kWH	$ 0.043	$ (624)
Well O&M matl	1	EA	$ 1,500	$ (1,500)
Well O&M labor	180	Hrs	$ 40	$ (7,200)
Subtotal				**$ (9,324)**
TOTAL ANNUAL O&M COSTS				**$ 18,479**

Table C.5

PWS Name *Roosevelt ISD*
Alternative Name *Central Treatment - Reverse Osmosis*
Alternative Number *RI-5*

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Reverse Osmosis Unit Purchase/Installation				
Site preparation	0.60	acre	$ 4,000	$ 2,400
Slab	20	CY	$ 1,000	$ 20,000
Building	700	SF	$ 60	$ 42,000
Building electrical	700	SF	$ 8	$ 5,600
Building plumbing	700	SF	$ 8	$ 5,600
Heating and ventilation	700	SF	$ 7	$ 4,900
Fence	1,000	LF	$ 15	$ 15,000
Paving	3,000	SF	$ 2	$ 6,000
Electrical	1	JOB	$ 60,000	$ 60,000
Piping	1	JOB	$ 30,000	$ 30,000
Reverse osmosis package including:				
High pressure pumps - 15hp				
Cartridge filters and vessels				
RO membranes and vessels				
Control system				
Chemical feed systems				
Freight cost				
Vendor start-up services	1	UNIT	$ 235,000	$ 235,000
Feed pumps	2	EA	$ 8,000	$ 16,000
Permeate tank	30,000	gal	$ 3	$ 90,000
Reject pond:				
Excavation	1,500	CYD	$ 3	$ 4,500
Compacted fill	1,250	CYD	$ 7	$ 8,750
Lining	21,750	SF	$ 0.50	$ 10,875
Vegetation	2,500	SY	$ 1	$ 2,500
Access road	625	LF	$ 30	$ 18,750
Subtotal of Design/Construction Costs				**$ 577,875**
Contingency	20%			$ 115,575
Design & Constr Management	25%			$ 144,469
Reject water haulage truck	1	EA	$ 100,000	$ 100,000
TOTAL CAPITAL COSTS				**$ 937,919**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Reverse Osmosis Unit O&M				
Building Power	12,000	kwh/yr	$ 0.043	$ 516
Equipment power	29,500	kwh/yr	$ 0.043	$ 1,269
Labor	1,000	hrs/yr	$ 40	$ 40,000
Materials	1	year	$ 6,000	$ 6,000
Chemicals	1	year	$ 3,000	$ 3,000
Analyses	24	test	$ 200	$ 4,800
Subtotal				**$ 55,585**
Backwash Disposal				
Disposal truck mileage	12,265	miles	$ 1	$ 12,265
Backwash disposal fee	2,630	kgal/yr	$ 5	$ 13,150
Subtotal				**$ 25,415**
TOTAL ANNUAL O&M COSTS				**$ 81,000**

Table C.6

PWS Name ***Roosevelt ISD***
Alternative Name ***Central Treatment - Electro-dialysis Reversal***
Alternative Number ***RI-6***

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
EDR Unit Purchase/Installation				
Site preparation	0.60	acre	$ 4,000	$ 2,400
Slab	20	CY	$ 1,000	$ 20,000
Building	700	SF	$ 60	$ 42,000
Building electrical	700	SF	$ 8	$ 5,600
Building plumbing	700	SF	$ 8	$ 5,600
Heating and ventilation	700	SF	$ 7	$ 4,900
Fence	1,000	LF	$ 15	$ 15,000
Paving	3,000	SF	$ 2	$ 6,000
Electrical	1	JOB	$ 60,000	$ 60,000
Piping	1	JOB	$ 30,000	$ 30,000
Product storage tank	30,000	gal	$ 3	$ 90,000
Feed pump	2	EA	$ 8,000	$ 16,000
Transfer pump (5hp)	2	EA	$ 6,000	$ 12,000
EDR package including:				
Feed and concentrate pumps				
Cartridge filters and vessels				
EDR membrane stacks				
Electrical module				
Chemical feed systems				
Freight cost				
Vendor start-up services	1	UNIT	$ 350,000	$ 350,000
Reject pond:				
Excavation	1,500	CYD	$ 3	$ 4,500
Compacted fill	1,250	CYD	$ 7	$ 8,750
Lining	21,750	SF	$ 0.50	$ 10,875
Vegetation	2,500	SY	$ 1	$ 2,500
Access road	625	LF	$ 30	$ 18,750
Subtotal of Design/Construction Costs				**$ 704,875**
Contingency	20%			$ 140,975
Design & Constr Management	25%			$ 176,219
Reject water haulage truck	1	EA	$ 100,000	$ 100,000
TOTAL CAPITAL COSTS				**$ 1,122,069**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
EDR Unit O&M				
Building Power	12,000	kwh/yr	$ 0.043	$ 516
Equipment power	53,600	kwh/yr	$ 0.043	$ 2,305
Labor	1,000	hrs/yr	$ 40	$ 40,000
Materials	1	year	$ 6,000	$ 6,000
Chemicals	1	year	$ 3,000	$ 3,000
Analyses	24	test	$ 200	$ 4,800
Subtotal				**$ 56,621**
Backwash Disposal				
Disposal truck mileage	9,100	miles	$ 1	$ 9,100
Backwash disposal fee	1,948	kgal/yr	$ 5	$ 9,740
Subtotal				**$ 18,840**
TOTAL ANNUAL O&M COSTS				**$ 75,461**

APPENDIX D
EXAMPLE FINANCIAL MODEL

Step 1

Water System: Roosevelt ISD

Step 2

Click Here to Update Verification and Raw

Water System	Roosevelt ISD
Alternative Description	New Well at 1 Mile

Sum of Amount — Year / Funding Alternative

Group	Type	2008 100% Grant	2008 Bond	2009 100% Grant	2009 Bond	2010 100% Grant	2010 Bond	2011 100% Grant	2011 Bond	2012 100% Grant	2012 Bond	2013 100% Grant	2013 Bond	2014 100% Grant	2014 Bond	2015 100% Grant	2015 Bond
Capital Expenditures	Capital Expenditures-Funded from Bonds	$ -	$ 526,853	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Grants	$ 526,853	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Revenue/Reserves	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from SRF Loans	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Capital Expenditures Sum		$ 526,853	$ 526,853	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service	Revenue Bonds	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214
	State Revolving Funds	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service Sum		$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214
Operating Expenditures	Other Operating Expenditures 1	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000
	O&M Associated with Alternative			$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479
Operating Expenditures Sum		$ 15,000	$ 15,000	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479
Residential Operating Revenu	Residential Tier2 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier3 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier4 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Unmetered Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier 1 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Base Annual Rate	$ 15,120	$ 15,120	$ 15,120	$ 55,974	$ 33,119	$ 115,547	$ 51,478	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266
Residential Operating Revenues Sum		$ 15,120	$ 15,120	$ 15,120	$ 55,974	$ 33,119	$ 115,547	$ 51,478	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266

Group	Type	2016 100% Grant	2016 Bond	2017 100% Grant	2017 Bond	2018 100% Grant	2018 Bond	2019 100% Grant	2019 Bond	2020 100% Grant	2020 Bond	2021 100% Grant	2021 Bond	2022 100% Grant	2022 Bond	2023 100% Grant	2023 Bond
Capital Expenditures	Capital Expenditures-Funded from Bonds	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Grants	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Revenue/Reserves	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from SRF Loans	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Capital Expenditures Sum		$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service	Revenue Bonds	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214
	State Revolving Funds	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service Sum		$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214	$ -	$ 41,214
Operating Expenditures	Other Operating Expenditures 1	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000	$ 15,000
	O&M Associated with Alternative	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479	$ 18,479
Operating Expenditures Sum		$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479	$ 33,479
Residential Operating Revenu	Residential Tier2 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier3 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier4 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Unmetered Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier 1 Annual Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Base Annual Rate	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266
Residential Operating Revenues Sum		$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266

Location_Name	Roosevelt ISD
Alt_Desc	New Well at 1 Mile

Current_Year Funding_Alt

Data	2008 100% Grant	2008 Bond	2009 100% Grant	2009 Bond	2010 100% Grant	2010 Bond	2011 100% Grant	2011 Bond	2012 100% Grant	2012 Bond	2013 100% Grant	2013 Bond	2014 100% Grant	2014 Bond	2015 100% Grant	2015 Bond
Sum of Beginning_Cash_Bal	$ 240	$ 240	$ 360	$ (40,854)	$ (17,999)	$ (59,573)	$ (18,359)	$ (18,719)	$ (360)	$ 40,854	$ 17,999	$ 100,427	$ 36,358	$ 160,000	$ 54,717	$ 219,573
Sum of Total_Expenditures	$ 541,853	$ 583,067	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693
Sum of Total_Receipts	$ 541,973	$ 541,973	$ 15,120	$ 55,974	$ 33,119	$ 115,547	$ 51,478	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266
Sum of Net_Cash_Flow	$ 120	$ (41,094)	$ (18,359)	$ (18,719)	$ (360)	$ 40,854	$ 17,999	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573
Sum of Ending_Cash_Bal	$ 360	$ (40,854)	$ (17,999)	$ (59,573)	$ (18,359)	$ (18,719)	$ (360)	$ 40,854	$ 17,999	$ 100,427	$ 36,358	$ 160,000	$ 54,717	$ 219,573	$ 73,076	$ 279,146
Sum of Working_Cap	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Repl_Resv	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Total_Reqd_Resv	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Net_Avail_Bal	$ 360	$ (40,854)	$ (17,999)	$ (59,573)	$ (18,359)	$ (18,719)	$ (360)	$ 40,854	$ 17,999	$ 100,427	$ 36,358	$ 160,000	$ 54,717	$ 219,573	$ 73,076	$ 279,146
Sum of Add_Resv_Needed	$ -	$ (40,854)	$ (17,999)	$ (59,573)	$ (18,359)	$ (18,719)	$ (360)	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Rate_Inc_Needed	0%	270%	119%	106%	55%	16%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Sum of Percent_Rate_Increase	0%	0%	0%	270%	119%	664%	240%	788%	243%	788%	243%	788%	243%	788%	243%	788%

Data	2016 100% Grant	2016 Bond	2017 100% Grant	2017 Bond	2018 100% Grant	2018 Bond	2019 100% Grant	2019 Bond	2020 100% Grant	2020 Bond	2021 100% Grant	2021 Bond	2022 100% Grant	2022 Bond	2023 100% Grant	2023 Bond
Sum of Beginning_Cash_Bal	$ 73,076	$ 279,146	$ 91,435	$ 338,719	$ 109,794	$ 398,292	$ 128,153	$ 457,865	$ 146,512	$ 517,438	$ 164,871	$ 577,011	$ 183,230	$ 636,584	$ 201,589	$ 696,157
Sum of Total_Expenditures	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693	$ 33,479	$ 74,693
Sum of Total_Receipts	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266	$ 51,838	$ 134,266
Sum of Net_Cash_Flow	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573	$ 18,359	$ 59,573
Sum of Ending_Cash_Bal	$ 91,435	$ 338,719	$ 109,794	$ 398,292	$ 128,153	$ 457,865	$ 146,512	$ 517,438	$ 164,871	$ 577,011	$ 183,230	$ 636,584	$ 201,589	$ 696,157	$ 219,948	$ 755,730
Sum of Working_Cap	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Repl_Resv	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Total_Reqd_Resv	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Net_Avail_Bal	$ 91,435	$ 338,719	$ 109,794	$ 398,292	$ 128,153	$ 457,865	$ 146,512	$ 517,438	$ 164,871	$ 577,011	$ 183,230	$ 636,584	$ 201,589	$ 696,157	$ 219,948	$ 755,730
Sum of Add_Resv_Needed	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Sum of Rate_Inc_Needed	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Sum of Percent_Rate_Increase	243%	788%	243%	788%	243%	788%	243%	788%	243%	788%	243%	788%	243%	788%	243%	788%

APPENDIX E
CONCEPTUAL ANALYSIS OF INCREASING COMPLIANT DRINKING WATER

E.1 INTRODUCTION

E.1.1 OVERVIEW OF DRINKING WATER QUALITY IN REGION

There are many PWSs in the Lubbock area that do not have compliant drinking water due to elevated concentrations of naturally occurring contaminants in the area groundwater. Largely, this is a result of the generally poor water quality associated with the Ogallala-South Formation that is the water source for most of these systems (see Chapter 3 of the report to which this is appended). The common groundwater contaminants in the Ogallala-South Formation include arsenic, selenium, fluoride, nitrate, and uranium.

According to the TCEQ Water Utility Database, there are nearly 24,000 people in the Lubbock area who are served by active residential PWSs that do not currently have compliant drinking water. The majority of this population can be found in the area just outside the City of Lubbock, and also to the south of the city. The total area population with noncompliant drinking water is likely greater than 24,000, since only populations served by active PWSs are included in this estimate. There is additional populations that currently obtain drinking water from private wells or are served by PWSs that have too few connections to be considered active PWSs in the TCEQ Water Utility Database. Additionally, while the issue of noncompliant drinking water affects these area residents directly, the lack of good quality drinking water may restrict growth in the entire Lubbock area.

This appendix presents a conceptual analysis of a possible regional solution to the drinking water compliance issue in the Lubbock area. The purpose of this analysis is to investigate whether a large-scale regional approach to provide compliant drinking water might be more cost-effective than each PWS seeking its own solution. The objective of the analysis is to provide an indication of whether there is sufficient potential benefit to a regional approach to warrant further study. The conceptual analysis presented here is based on a single scenario and does not attempt to evaluate or rank a range of different solutions. For purposes of this report, this single scenario is referred to as the Lubbock Area Regional Solution (LARS).

To improve readability, the tables and figures for this appendix appear in Section E.6.

E.1.2 EVALUATION OF PWS DRINKING WATER QUALITY

Drinking water quality for the PWSs in the eight counties included in and around Lubbock was evaluated using TCEQ PWS drinking water quality data to identify PWSs that had potential water quality compliance issues. There are a number of PWSs that do not serve residential populations, such as restaurants, businesses, *etc*. Since this analysis is focused on residential systems, these commercial systems were excluded from the analysis. Additionally, systems listed as "inactive" were also excluded because it was not easy to determine whether they were listed as inactive because of small size, or are truly inactive.

Once the active residential PWSs were identified, they were screened for the common contaminants in the area: arsenic, selenium, fluoride, nitrate, and uranium. Systems with concentrations of the identified contaminants greater than MCLs were deemed to have noncompliant water. It is important to note that this screening was not an official compliance determination, and a system's compliance status determined from the screening may not coincide with a system's actual compliance status. Discrepancies may result from the data available not being current, the use of simplified algorithms to give an indication of compliance, *etc*.

The PWSs identified with potential water quality compliance issues are shown in Table E.1, along with numbers of connections, the population served, and average daily consumption. For the LARS, the area has been divided into three separate subareas named LARS–Lubbock, LARS-Lamesa, and LARS-Brownfield. The PWSs, population, connections, and average daily consumptions for these subareas are shown in Tables E.2, E.3, and E.4. These systems are also shown in Figure E.1. As can be seen on the figure, these systems are generally located near Lubbock and south of Lubbock.

E.1.3 EXISTING DRINKING WATER SUPPLIES AND INFRASTRUCTURE

PWSs in the area typically obtain drinking water from wells, purchase water from the City of Lubbock, or obtain water from the Canadian River Municipal Water Authority (CRMWA), either as one of the 11 member cities or as customers of a member city. The City of Lubbock is a member city of the CRMWA and has the largest water system in the area. As well as getting water from the CRMWA, Lubbock obtains water from its own well field in Bailey County. The CRMWA provides surface water and groundwater via a pipeline from the north to a water treatment plant located at and operated by Lubbock, from which point the treated water is distributed via transmission mains to the seven member cities west and south of Lubbock. There are existing CRMWA pipelines that extend to the southeast and west and southwest from Lubbock. The approximate location and extent of these lines are shown in Figure E.1.

The CRMWA production is fully committed to the 11 member cities. In addition, the transmission mains from Lubbock to the other seven member cities are at capacity during the summer months. Therefore, the LARS scenario proposed here uses new wells for the water source and if existing pipeline infrastructure is used for water transmission, allowances are made to account for any pipeline capacity used.

E.2 DESCRIPTION OF THE LARS

Since existing water supplies and infrastructure do not have sufficient capacity available, and the existing infrastructure does not cover the entire area projected to be served by the LARS, the LARS needs to provide both a water source and a means of conveyance. To accomplish this, the LARS includes several groundwater treatment plants located near clusters of PWSs with water quality problems. The locations of these treatment plants include one near the existing water treatment plant in Lubbock, one at Lamesa, and one at Brownfield (Figure E.2).

In addition to the groundwater treatment plants, new well fields would also be required to feed the groundwater treatment plants. The assumed water quality used to design each groundwater treatment plant is based on water quality data for PWSs near the proposed plant location. Groundwater treatment will be achieved using RO technology because, of the two technologies best suited for treating contaminants generally found in the water of the Ogallala-South aquifer (RO and EDR), RO is typically the most economical option.

The plant at Lubbock would tie into the Lubbock distribution system. The water would be passed through the Lubbock distribution system, and pipelines would be run from the Lubbock distribution system to the noncompliant PWSs around Lubbock. The location of the treatment plant, required new pipelines, and potential customers for the Lubbock component of the LARS are shown on Figure E.3.

The plant at Lamesa could tie into the Lubbock distribution system at Lamesa or could be independent. If tied into the Lamesa system, it could supplement Lamesa's system to allow the non-compliant PWSs upstream of Lamesa to withdraw water without impacting existing customers between Lamesa and Lubbock. If not tied in, the system could serve PWSs outside the Lamesa area. The location of the treatment plant, required new pipelines, and potential customers for the Lamesa component of the LARS are shown on Figure E.4.

The plant at Brownfield could tie into the Brownfield distribution system at Brownfield or could be independent. If tied into the Brownfield system, it could supplement Lubbock's system to allow the non-compliant PWSs upstream of Brownfield to withdraw water without impacting existing customers between Brownfield and Lubbock. If not tied in, the system could serve PWSs outside the Brownfield area. The location of the treatment plant, required new pipelines, and potential customers for the Brownfield component of the LARS are shown on Figure E.5.

Pipelines could be built to connect the CRMWA lines to the other noncompliant PWSs. In this way, the Lamesa and Brownfield groundwater treatment plants could provide enough drinking water to meet the demands of the systems at the ends of the CRMWA lines to offset water that would be taken out by noncompliant PWSs along the existing CRMWA lines. Connecting pipelines for the groundwater treatment plants and noncompliant PWSs to the existing City of Lubbock and CRMWA pipe systems reduces the need for added infrastructure to implement the regional solution, and would provide operational flexibility.

E.3 ESTIMATED COSTS

Costs to implement the LARS were estimated. This includes costs for new wells, pipelines, pump stations, and treatment plants. A conceptual design was developed for the main infrastructure components, and was used as the basis for estimating capital and O&M costs. The estimated capital and O&M costs for the major infrastructure components are summarized in Table E.5. The annualized costs of these components are also shown in Table E.5, using a 6 percent discount rate and a 20-year period. Details of the capital costs for the three subareas are included in Tables E.6, E.7, and E.8.

Table E-9 presents an estimate of the cost of service to the LARS customers. If the customers were to bear the total capital and operating costs of the systems for their subarea or the system as a whole, the approximate monthly cost per connection would be as follows:

LARS-Lubbock:	$111/month	$1,336/year	4% of MHI
LARS-Lamesa:	$277/month	$3,327/year	9% of MHI
LARS-Brownfield:	$226/month	$2,716/year	8% of MHI
Combined:	$189/month	$2,266/year	6% of MHI

If the systems would be able to get 100 percent grant funding for the capital costs of constructing the system, the approximate monthly cost per connection would be as follows:

LARS-Lubbock:	$42/month	$509/year	1% of MHI
LARS-Lamesa:	$53/month	$630/year	2% of MHI
LARS-Brownfield:	$72/month	$866/year	2% of MHI
Combined:	$59/month	$711/year	2% of MHI

This then forms the approximate range of the cost of service for the customers (per connection) of a regional solution.

Increasing the coverage of the regional solution to include populations served by inactive PWSs or those that have private wells could have the effect of reducing treatment costs on a per gallon basis, but increasing the cost for distribution piping. Likewise, other sources of water with associated quality aspects would affect the cost, including surface water sources, better groundwater sources, and the use of reclaimed water, either for supplemental potable or non-potable uses. A more detailed assessment would be required to determine whether the overall effect would be an increase or decrease on the cost to the customers.

E.5 CONCLUSION

A regional solution to serving non-compliant PWSs in the Lubbock area presents a potentially viable solution to an existing problem. If suitable groundwater can be found, a regional system could be implemented within a cost per connection range of $59/month to $189/month, with the actual cost depending on the source and costs of capital funds needed to build a regional system.

A Community Development Block Grant is one possible source of funding the capital costs for the regional solution. Community Development Block Grants are discussed further in Attachment E1.

Table E.1

Active Residential Public Water Systems with Potential Water Quality Problems

Lubbock Area Regional Solution

PWS ID #	PWS Name	Population	Connections	Avg. Daily Consumption (mgd)	County
0170010	BORDEN COUNTY WATER SYSTEM	102	102	0.010	BORDEN
0580011	ACKERLY WATER SUPPLY CORP	230	125	0.115	DAWSON
0580013	WELCH WATER SUPPLY CORP	312	123	0.057	DAWSON
0580025	KLONDIKE HIGH SCHOOL	250	16	0.025	DAWSON
0830001	SEAGRAVES CITY OF	2400	974	0.473	GAINES
0830011	LOOP WATER SUPPLY CORP	350	117	0.053	GAINES
0830012	SEMINOLE CITY OF	6456	2641	1.531	GAINES
0850002	SOUTHLAND ISD	193	4	0.019	GARZA
1100004	ROPESVILLE CITY OF	517	196	0.094	HOCKLEY
1100010	SMYER CITY OF	480	180	0.051	HOCKLEY
1100011	WHITHARRAL WATER SUPPLY CORP	275	82	0.043	HOCKLEY
1100030	OPDYKE WEST WATER SUPPLY	140	63	0.018	HOCKLEY
1520005	WOLFFORTH CITY OF	3000	1150	0.439	LUBBOCK
1520009	BIG Q MOBILE HOME ESTATES	200	70	0.013	LUBBOCK
1520025	BUSTERS MOBILE HOME PARK	20	8	0.002	LUBBOCK
1520026	FAMILY COMMUNITY CENTER MHP	88	40	0.011	LUBBOCK
1520027	WAGON WHEEL MOBILE VILLAGE HOME PR	30	21	0.003	LUBBOCK
1520036	GREEN MOBILE HOME PARK	50	28	0.004	LUBBOCK
1520039	PECAN GROVE MOBILE HOME PARK	100	50	0.008	LUBBOCK
1520062	PLOTT ACRES	201	63	0.019	LUBBOCK
1520067	114TH STREET MOBILE HOME PARK	96	43	0.009	LUBBOCK
1520080	FRANKLIN WATER SERVICE COMPANY	152	64	0.011	LUBBOCK
1520094	TOWN NORTH VILLAGE WATER SYSTEM	330	117	0.031	LUBBOCK
1520106	COX ADDITION WATER SYSTEM	133	40	0.014	LUBBOCK
1520122	LUBBOCK COOPER ISD	1900	14	0.190	LUBBOCK
1520123	ROOSEVELT ISD	1600	11	0.048	LUBBOCK
1520149	WHORTON MOBILE HOME PARK	75	26	0.008	LUBBOCK
1520152	TOWN NORTH ESTATES	227	67	0.015	LUBBOCK
1520154	CHARLIE BROWNS LEARNING CENTER	47	3	0.005	LUBBOCK
1520155	COUNTRY SQUIRE MHP 2	75	16	0.008	LUBBOCK
1520156	ELM GROVE MOBILE HOME PARK	24	20	0.002	LUBBOCK
1520158	MILLER MOBILE HOME PARK	60	33	0.005	LUBBOCK
1520185	LUBBOCK RV PARK	133	100	0.009	LUBBOCK
1520188	CASEY ESTATES WATER	312	104	0.026	LUBBOCK
1520192	TERRELLS MOBILE HOME PARK	50	22	0.005	LUBBOCK
1520198	VALLEY ESTATES	70	36	0.007	LUBBOCK
1520199	WOLFFORTH PLACE	460	123	0.041	LUBBOCK
1520211	TEXIN ENTERPRISES	27	9	0.002	LUBBOCK
1520217	SOUTHWEST GARDEN WATER	375	125	0.028	LUBBOCK
1520223	PAUL COBB WATER SYSTEM	30	18	0.003	LUBBOCK
1520225	FAY BEN MOBILE HOME PARK	90	55	0.007	LUBBOCK
1520241	MANAGED CARE CENTER	40	5	0.003	LUBBOCK
1520247	COUNTRY VIEW MHP	67	24	0.007	LUBBOCK
1530001	ODONNELL CITY OF	1100	392	0.139	LYNN
1530004	NEW HOME CITY OF	280	125	0.055	LYNN
1530005	GRASSLAND WATER SUPPLY CORP	80	30	0.008	LYNN
2230002	MEADOW CITY OF	547	230	0.138	TERRY
2230003	WELLMAN PUBLIC WATER SYSTEM	236	95	0.046	TERRY
	TOTALS	**24,010**	**8,000**	**3.856**	

Table E.2
Public Water Systems associated with LARS-Lubbock Treatment Plant

PWS ID #	PWS Name	Population	Connections	Avg. Daily Consumption (mgd)	County
0850002	SOUTHLAND ISD	193	4	0.019	GARZA
1100010	SMYER CITY OF	480	180	0.051	HOCKLEY
1100011	WHITHARRAL WATER SUPPLY CORP	275	82	0.043	HOCKLEY
1100030	OPDYKE WEST WATER SUPPLY	140	63	0.018	HOCKLEY
1520005	WOLFFORTH CITY OF	3000	1150	0.439	LUBBOCK
1520009	BIG Q MOBILE HOME ESTATES	200	70	0.013	LUBBOCK
1520025	BUSTERS MOBILE HOME PARK	20	8	0.002	LUBBOCK
1520026	FAMILY COMMUNITY CENTER MHP	88	40	0.011	LUBBOCK
1520027	WAGON WHEEL MOBILE VILLAGE HOME PR	30	21	0.003	LUBBOCK
1520036	GREEN MOBILE HOME PARK	50	28	0.004	LUBBOCK
1520039	PECAN GROVE MOBILE HOME PARK	100	50	0.008	LUBBOCK
1520062	PLOTT ACRES	201	63	0.019	LUBBOCK
1520067	114TH STREET MOBILE HOME PARK	96	43	0.009	LUBBOCK
1520080	FRANKLIN WATER SERVICE COMPANY	152	64	0.011	LUBBOCK
1520094	TOWN NORTH VILLAGE WATER SYSTEM	330	117	0.031	LUBBOCK
1520106	COX ADDITION WATER SYSTEM	133	40	0.014	LUBBOCK
1520122	LUBBOCK COOPER ISD	1900	14	0.190	LUBBOCK
1520123	ROOSEVELT ISD	1600	11	0.048	LUBBOCK
1520149	WHORTON MOBILE HOME PARK	75	26	0.008	LUBBOCK
1520152	TOWN NORTH ESTATES	227	67	0.015	LUBBOCK
1520154	CHARLIE BROWNS LEARNING CENTER	47	3	0.005	LUBBOCK
1520155	COUNTRY SQUIRE MHP 2	75	16	0.008	LUBBOCK
1520156	ELM GROVE MOBILE HOME PARK	24	20	0.002	LUBBOCK
1520158	MILLER MOBILE HOME PARK	60	33	0.005	LUBBOCK
1520185	LUBBOCK RV PARK	133	100	0.009	LUBBOCK
1520188	CASEY ESTATES WATER	312	104	0.026	LUBBOCK
1520192	TERRELLS MOBILE HOME PARK	50	22	0.005	LUBBOCK
1520198	VALLEY ESTATES	70	36	0.007	LUBBOCK
1520199	WOLFFORTH PLACE	460	123	0.041	LUBBOCK
1520211	TEXIN ENTERPRISES	27	9	0.002	LUBBOCK
1520217	SOUTHWEST GARDEN WATER	375	125	0.028	LUBBOCK
1520223	PAUL COBB WATER SYSTEM	30	18	0.003	LUBBOCK
1520225	FAY BEN MOBILE HOME PARK	90	55	0.007	LUBBOCK
1520241	MANAGED CARE CENTER	40	5	0.003	LUBBOCK
1520247	COUNTRY VIEW MHP	67	24	0.007	LUBBOCK
1530004	NEW HOME CITY OF	280	125	0.055	LYNN
	TOTALS	**11,430**	**2,959**	**1.167**	

Table E.3
Public Water Systems associated with LARS-Lamesa Treatment Plant

PWS ID #	PWS Name	Population	Connections	Avg. Daily Consumption (mgd)	County
0170010	BORDEN COUNTY WATER SYSTEM	102	102	0.010	BORDEN
0580011	ACKERLY WATER SUPPLY CORP	230	125	0.115	DAWSON
0580013	WELCH WATER SUPPLY CORP	312	123	0.057	DAWSON
0580025	KLONDIKE HIGH SCHOOL	250	16	0.025	DAWSON
1530001	ODONNELL CITY OF	1100	392	0.139	LYNN
1530005	GRASSLAND WATER SUPPLY CORP	80	30	0.008	LYNN
	TOTALS	**2,074**	**788**	**0.354**	

Table E.4
Public Water Systems associated with LARS-Brownfield Treatment Plant

PWS ID #	PWS Name	Population	Connections	Avg. Daily Consumption (mgd)	County
0830001	SEAGRAVES CITY OF	2400	974	0.473	GAINES
0830011	LOOP WATER SUPPLY CORP	350	117	0.053	GAINES
0830012	SEMINOLE CITY OF	6456	2641	1.531	GAINES
1100004	ROPESVILLE CITY OF	517	196	0.094	HOCKLEY
2230002	MEADOW CITY OF	547	230	0.138	TERRY
2230003	WELLMAN PUBLIC WATER SYSTEM	236	95	0.046	TERRY
	TOTALS	**10,506**	**4,253**	**2.335**	

Table E.5

Summary of Cost Components

Lubbock Area Regional Solution (LARS)

Cost Item	Capital	O&M	Annualized 20 yr, 6%
LARS - Lamesa			
Wells	$ 783,000	$ 78,578	$ 146,844
Treatment Plant	$ 3,271,200	$ 308,989	$ 594,187
Pipeline and Pump Stations	$ 20,323,892	$ 108,939	$ 1,880,869
Subtotal	**$ 24,378,092**	**$ 496,506**	**$ 2,621,899**
LARS - Brownfield			
Wells	$ 5,383,125	$ 540,224	$ 1,009,550
Treatment Plant	$ 14,734,900	$ 1,563,235	$ 2,847,891
Pipeline and Pump Stations	$ 70,140,452	$ 1,578,779	$ 7,693,944
Subtotal	**$ 90,258,477**	**$ 3,682,239**	**$ 11,551,384**
LARS - Lubbock			
Wells	$ 2,740,500	$ 275,023	$ 513,952
Treatment Plant	$ 7,397,900	$ 816,460	$ 1,461,443
Pipeline and Pump Stations	$ 17,931,065	$ 415,323	$ 1,978,635
Subtotal	**$ 28,069,465**	**$ 1,506,807**	**$ 3,954,030**
TOTAL	**$ 142,706,034**	**$ 5,685,551**	**$ 18,127,314**

Table E.6

Lubbock Area Regional Solution - Treatment Plant at Lubbock

Summary of Cost Components

Item	Quantity	Unit	Capital	O&M
Wells				
New wells	28	EA	$ 1,890,000	$ 275,023
Contingency	20%		$ 378,000	
Design & Constr Management	25%		$ 472,500	
Subtotal			**$ 2,740,500**	**$ 275,023**
Treatment				
RO Treatment Plant	1	EA	$ 5,102,000	$ 816,460
Contingency	20%		$ 1,020,400	
Design & Constr Management	25%		$ 1,275,500	
Subtotal			**$ 7,397,900**	**$ 816,460**
Pipeline				
4" Pipeline w/complete installation	49.07	Miles	$ 8,636,689	$ 11,450
6" Pipeline w/complete installation	3.66	Miles	$ 642,002	$ 849
10" Pipeline w/complete installation	2.17	Miles	$ 612,761	$ 542
Contingency	20%		$ 1,978,290	
Design & Constr Management	25%		$ 2,472,863	
Subtotal			**$ 14,342,605**	**$ 12,841**
Pump Stations				
Pump Stations	13	EA	$ 2,474,800	$ 402,482
Contingency	20%		$ 494,960	
Design & Constr Management	25%		$ 618,700	
Subtotal			**$ 3,588,460**	**$ 402,482**
TOTAL COSTS			**$ 28,069,465**	**$ 1,506,807**

Table E.7

Lubbock Area Regional Solution - Treatment Plant at Lamesa

Summary of Cost Components

Item	Quantity	Unit	Capital	O&M
Wells				
New wells	8	EA	$ 540,000	$ 78,578
Contingency	20%		$ 108,000	
Design & Constr Management	25%		$ 135,000	
Subtotal			**$ 783,000**	**$ 78,578**
Treatment				
RO Treatment Plant	1	EA	$ 2,256,000	$ 308,989
Contingency	20%		$ 451,200	
Design & Constr Management	25%		$ 564,000	
Subtotal			**$ 3,271,200**	**$ 308,989**
Pipeline				
4" Pipeline w/complete installation	33.30	Miles	$ 5,484,498	$ 8,326
6" Pipeline w/complete installation	15.15	Miles	$ 2,966,562	$ 3,787
8" Pipeline w/complete installation	22.89	Miles	$ 5,203,212	$ 5,722
Contingency	20%		$ 2,730,854	
Design & Constr Management	25%		$ 3,413,568	
Subtotal			**$ 19,798,695**	**$ 17,835**
Pump Stations				
Pump Stations	5	EA	$ 362,205	$ 91,104
Contingency	20%		$ 72,441	
Design & Constr Management	25%		$ 90,551	
Subtotal			**$ 525,197**	**$ 91,104**
TOTAL COSTS			**$ 24,378,092**	**$ 496,506**

Table E.8
Lubbock Area Regional Solution - Treatment Plant at Brownfield
Summary of Cost Components

Item	Quantity	Unit	Capital	O&M
Wells				
New wells	55	EA	$ 3,712,500	$ 540,224
Contingency	20%		$ 742,500	
Design & Constr Management	25%		$ 928,125	
Subtotal			**$ 5,383,125**	**$ 540,224**
Treatment				
RO Treatment Plant	1	EA	$ 10,162,000	$ 1,563,235
Contingency	20%		$ 2,032,400	
Design & Constr Management	25%		$ 2,540,500	
Subtotal			**$ 14,734,900**	**$ 1,563,235**
Pipeline				
4" Pipeline w/complete installation	3.43	Miles	$ 543,272	$ 857
6" Pipeline w/complete installation	16.36	Miles	$ 3,206,887	$ 4,090
8" Pipeline w/complete installation	1.01	Miles	$ 284,268	$ 251
24" Pipeline w/complete installation	16.66	Miles	$ 15,300,032	$ 4,166
30" Pipeline w/complete installation	24.72	Miles	$ 28,023,581	$ 6,180
Contingency	20%		$ 9,471,608	
Design & Constr Management	25%		$ 11,839,510	
Subtotal			**$ 68,669,159**	**$ 15,544**
Pump Stations				
Pump Stations	6	EA	$ 1,014,685	$ 137,212
Contingency	20%		$ 202,937	
Design & Constr Management	25%		$ 253,671	
Subtotal			**$ 1,471,293**	**$ 137,212**
TOTAL COSTS			**$ 90,258,477**	**$ 2,256,215**

Table E.9
Lubbock Area Regional Solution (LARS)
Cost of Service

Component	Lubbock	Lamesa	Brownfield	Combined
Capital Cost	$ 28,069,465	$ 24,378,092	$ 90,258,477	$ 142,706,034
Annual O&M	$ 1,506,807	$ 496,506	$ 3,682,239	$ 5,685,551
Annualized 20 yr., 6%	$ 3,954,030	$ 2,621,899	$ 11,551,384	$ 18,127,314
Population	11,430	2,074	10,506	$ 24,010
Connections	2,959	788	4,253	$ 8,000
Annualized/Population	$ 345.93	$ 1,264.18	$ 1,099.50	$ 754.99
Annualized/Connection	$ 1,336.27	$ 3,327.28	$ 2,716.06	$ 2,265.91
Annualized/Connection as % of MHI*	4%	9%	8%	6%
Annualized/Connection/Month	**$ 111.36**	**$ 277.27**	**$ 226.34**	**$ 188.83**
Annual O&M/Population	$ 131.83	$ 239.40	$ 350.49	$ 236.80
Annual O&M/Connection	$ 509.23	$ 630.08	$ 865.80	$ 710.69
Annual O&M/Connection as % of MHI*	1%	2%	2%	2%
Annual O&M/Connection/Month	**$ 42.44**	**$ 52.51**	**$ 72.15**	**$ 59.22**

* Percentage of MHI calculated based on the MHI for Lubbock County of $35,189.

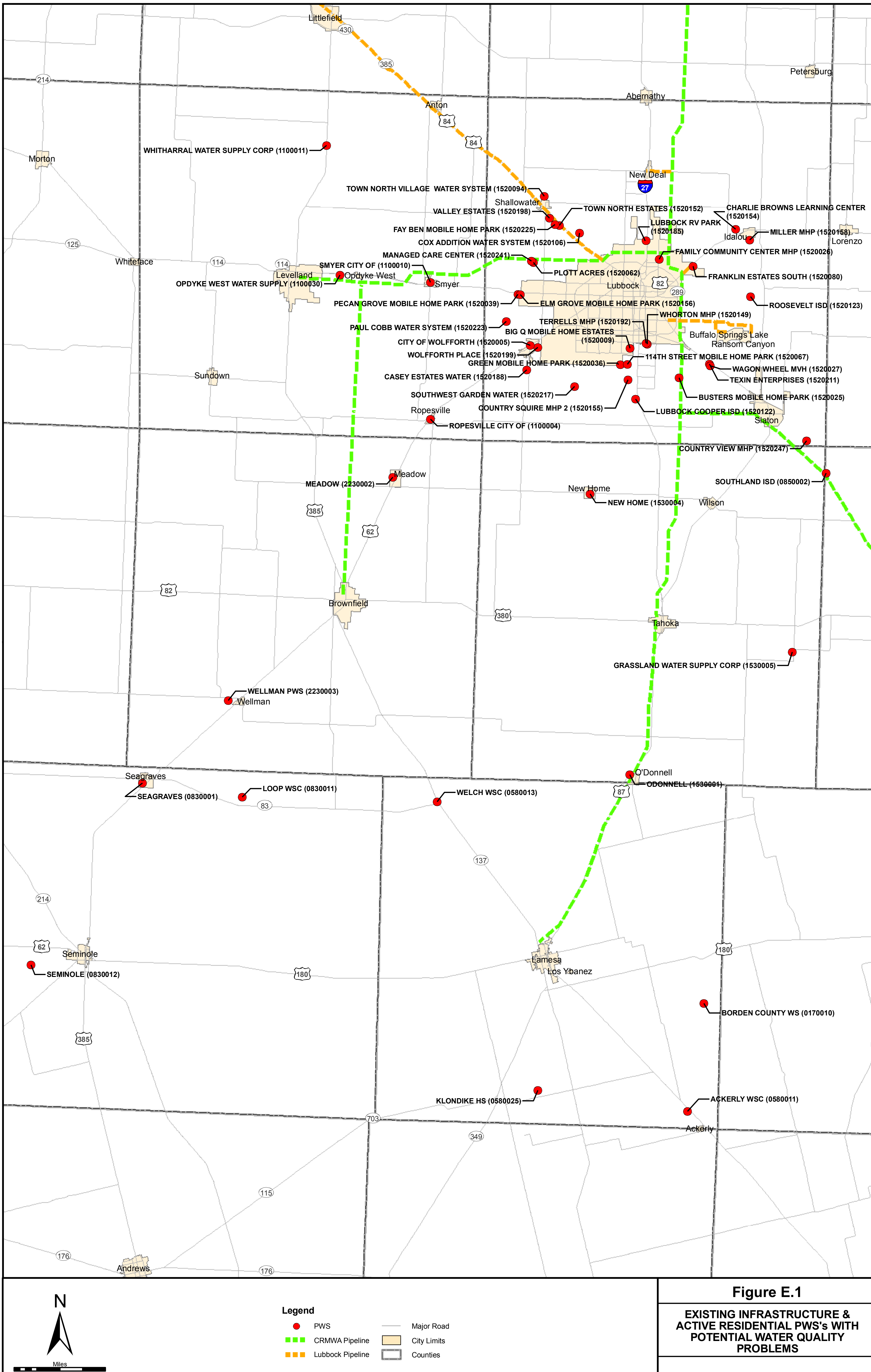

Figure E.1

EXISTING INFRASTRUCTURE & ACTIVE RESIDENTIAL PWS's WITH POTENTIAL WATER QUALITY PROBLEMS

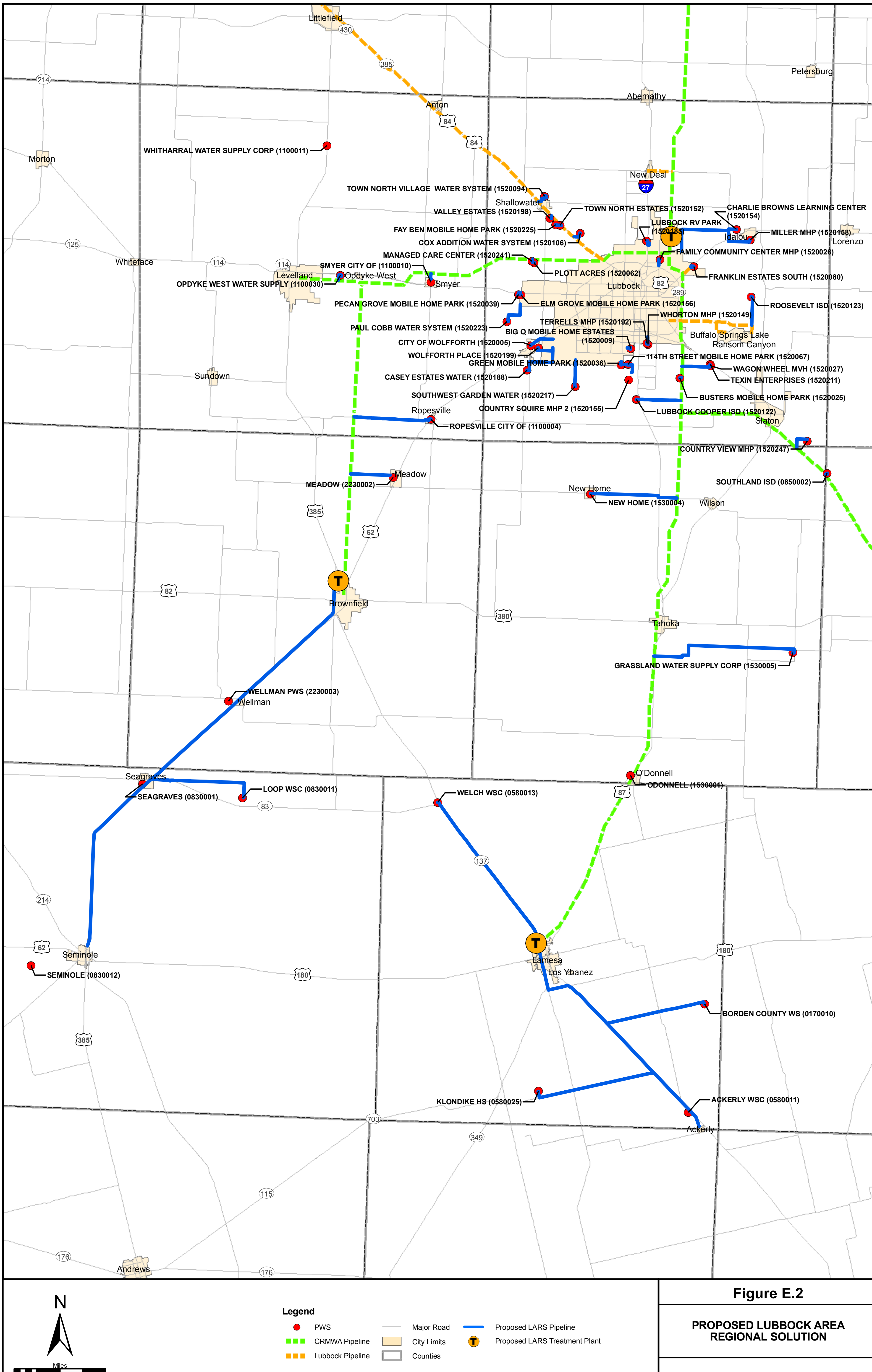

Figure E.2

PROPOSED LUBBOCK AREA REGIONAL SOLUTION

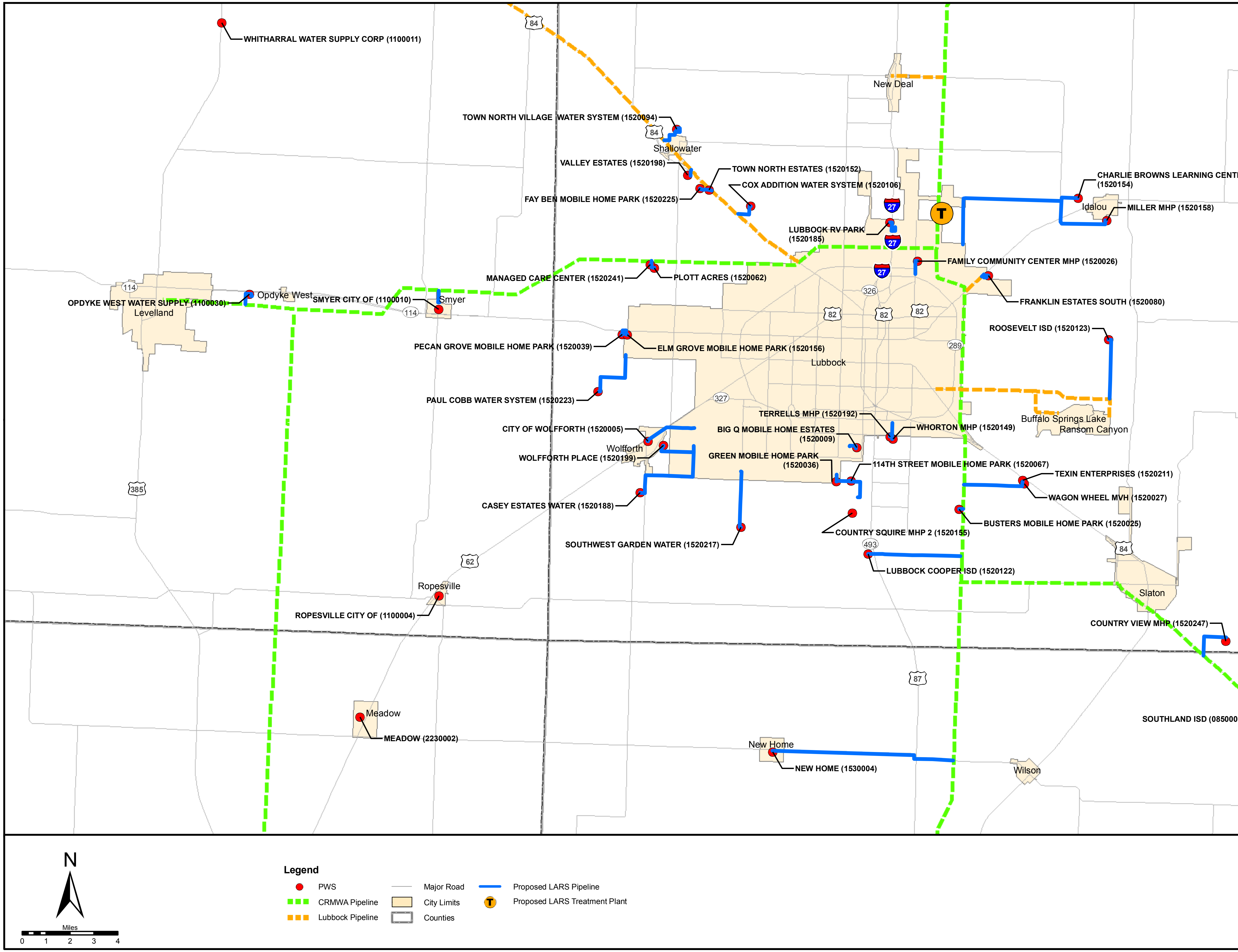

Figure E.3

LUBBOCK PLANT & ASSOCIATED PWS's

Lubbock Area Regional Solution

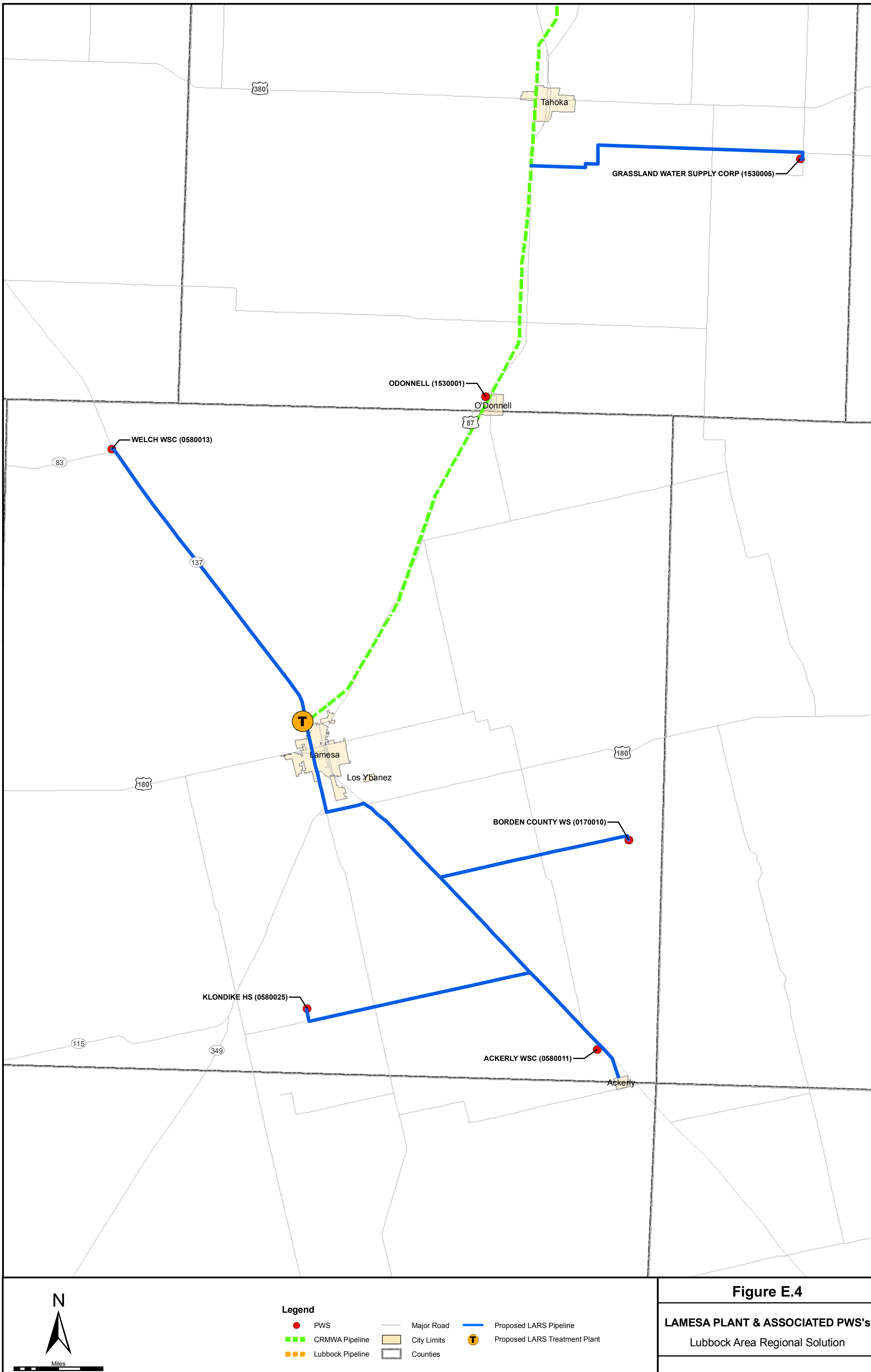

Figure E.4

LAMESA PLANT & ASSOCIATED PWS's

Lubbock Area Regional Solution

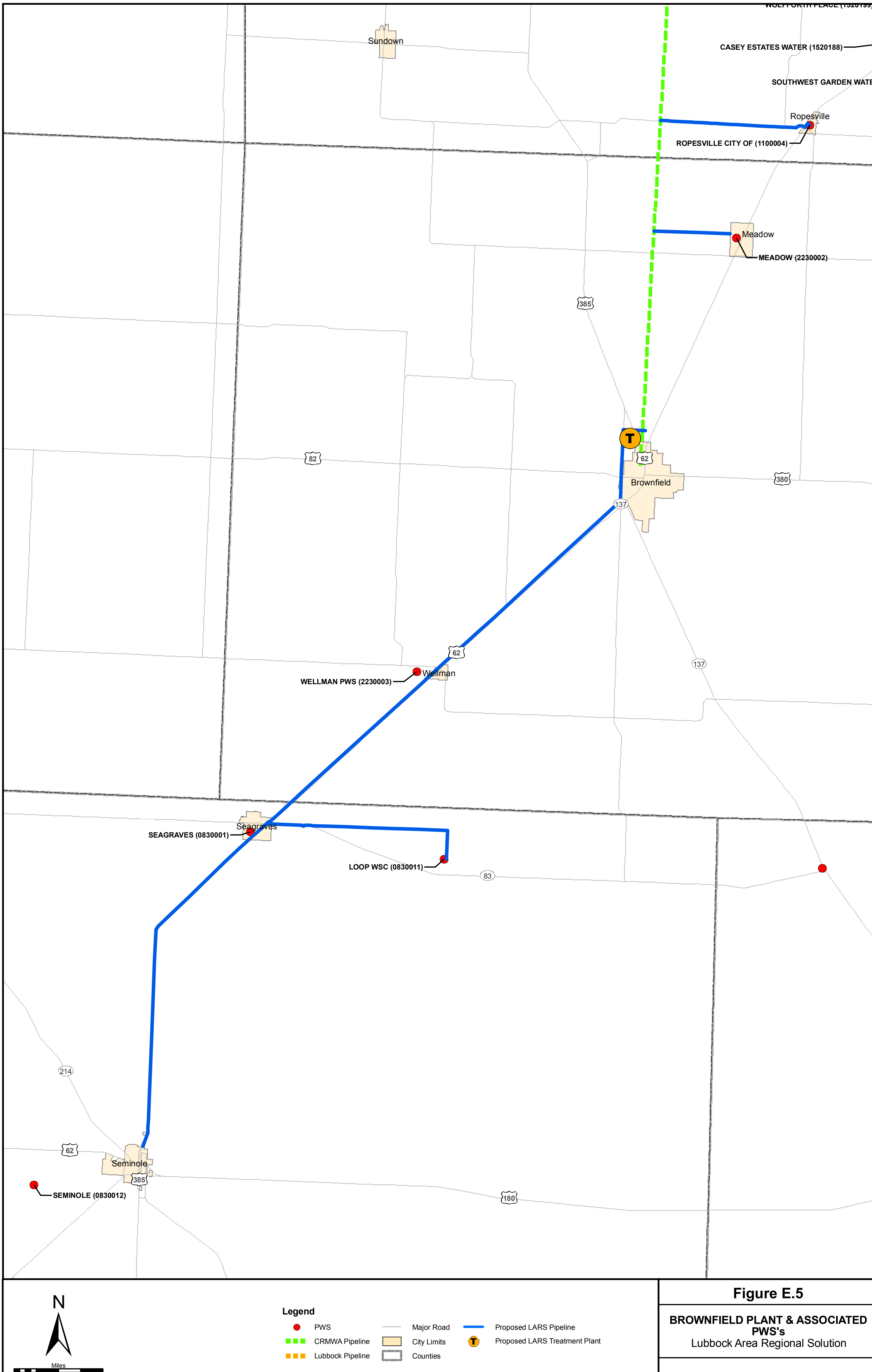

Figure E.5

BROWNFIELD PLANT & ASSOCIATED PWS's

Lubbock Area Regional Solution

ATTACHMENT E1
TEXAS COMMUNITY DEVELOPMENT BLOCK GRANTS

INTRODUCTION

Every year, the U.S. Department of Housing and Urban Development (HUD) provides federal Community Development Block Grant (CDBG) funds directly to states, which, in turn, provide the funds to small, rural cities with populations of less than 50,000, and to counties that have a non-metropolitan population under 200,000 and are not eligible for direct funding from HUD. These small communities are called "non-entitlement" areas because they must apply for CDBG dollars through the Office of Rural Community Affairs (ORCA). The grants may be used for community and economic development activities, but are primarily used for housing rehabilitation, public infrastructure projects (*e.g.*, wastewater and drinking water facilities), and economic development. Seventy percent of grant funds must be used for activities that principally benefit low- and moderate-income persons.

ORCA administers the State of Texas CDBG Program, called the Texas Community Development Block Grant Program (Texas CDBG). The Texas Department of Agriculture (TDA) administers the Texas Capital Fund through an interagency agreement between ORCA and TDA.

ORCA's CDBG Program is the largest in the nation. The rural-focused program serves approximately 1,017 eligible rural communities, 245 rural counties, and provides services to over 375,000 low- to moderate-income beneficiaries each year. Of the 1,017 communities eligible for CDBG funds, 740 have a population of less than 3,000, and 424 have a population of less than 1,000. The demographics and rural characteristics of Texas have shaped a program that focuses on providing basic human needs and sanitary infrastructure to small rural communities in outlying areas.

PROGRAM ADMINISTRATION

ORCA administers the CDBG programs in accordance to funding rules and regulations set by HUD. Each year, ORCA submits an Action Plan for the next fiscal year. The Action Plan describes the methods ORCA will use for distributing funds among the various CDBG programs, including award amounts per program, application selection process, *etc*. Once HUD approves the Action Plan, it becomes codified into the Texas Administrative Code under Title 10 TAC Chapter 255. The agency then makes applications available in accordance with each program's funding cycle. Applications received for competitive funding programs are reviewed and scored using program-specific criteria and processes. These processes may include scoring by Regional Review Committees and review by the State Review Committees.

Once awards are made from ORCA's CDBG Program, contracts are executed between the agency and the city or county officials, and the grantee begins the implementation of their proposed project. To guide grantees in the implementation of their projects, the grantees follow the 2005 CDBG Implementation Manual. The Manual describes the methods a CDBG grant recipient uses to administer the CDBG contract, and includes relevant forms.

ELIGIBLE APPLICANTS

Eligible applicants are nonentitlement general purpose units of local government, including cities and counties that are not participating or designated as eligible to participate in the entitlement portion of the federal CDBG. Nonentitlement cities that are not participating in urban county programs through existing participation agreements are eligible applicants (unless the city's population is counted toward the urban county CDBG allocation).

Nonentitlement cities are located predominately in rural areas and are cities with populations less than 50,000 thousand persons; cities that are not designated as a central city of a metropolitan statistical area; and cities that are not participating in urban county programs. Nonentitlement counties are also predominately rural in nature and are counties that generally have fewer than 200,000 persons in the nonentitlement communities and unincorporated areas located in the county.

ELIGIBLE ACTIVITIES

Eligible activities under the Texas CDBG are listed in 42 United States Code Section 5305. The Texas CDBG staff reviews all proposed project activities included in applications for all fund categories except the Texas Capital Fund (TCF), to determine eligibility. The Texas Department of Agriculture determines the eligibility of activities included in TCF applications.

All proposed activities must meet one of the following three National Program Objectives:

1. Benefit principally low- and moderate-income persons; or
2. Aid in the elimination of slums or blight; or
3. Meet other community development needs of particular urgency that represent an immediate threat to the health and safety of residents of the community.

INELIGIBLE ACTIVITIES

In general, any type of activity not described or referred to in 42 United States Code Section 5305 is ineligible. Specific activities ineligible under the Texas CDBG are:

1. Construction of buildings and facilities used for the general conduct of government (*e.g.* city halls, courthouses, *etc.*);

2. Construction of new housing, except as last resort housing under 49 CFR Part 24 or affordable housing through eligible subrecipients in accordance with 24 CFR 570.204;

3. Financing of political activities;

4. Purchases of construction equipment (except in limited circumstances under the STEP Program);

5. Income payments, such as housing allowances; and

6. Most O&M expenses (including smoke testing, televising/video taping line work, or any other investigative method to determine the overall scope and location of the project work activities)

The TCF will not accept applications in support of public or private prisons, racetracks, and projects that address job creation/retention through a government supported facility. The TCF Program may be used to financially assist/facilitate the relocation of a business when certain requirements, as defined in the application guidelines, are met.

PRIMARY BENEFICIARIES

The primary beneficiaries of the Texas CDBG are low to moderate income persons as defined under HUD, Section 8 Assisted Housing Program (Section l02(c)). Low income families are defined as those earning less than 50 percent of the area MHI. Moderate income families are defined as those earning less than 80 percent of the area MHI. The area median family can be based on a metropolitan statistical area, a non-metropolitan county, or the statewide non-metropolitan MHI figure.

SECTION 108 LOAN GUARANTEE PROGRAM

Section 108 is the loan guarantee provision of the CDBG. Section 108 provides communities with a source of financing for economic development, housing rehabilitation, public facilities, and large-scale physical development projects. This makes it one of the most potent and important public investment tools that HUD offers to local governments. It allows these local governments to transform a small portion of their CDBG funds into federally guaranteed loans large enough to pursue physical and economic revitalization projects that can renew entire neighborhoods. Such public investment is often needed to inspire private economic activity, providing the initial resources, or simply the confidence that private firms and individuals may need to invest in distressed areas. Section 108 loans are not risk-free; however, local governments borrowing funds guaranteed by Section 108 must pledge their current and future CDBG allocations to cover the loan amount as security for the loan.

The loan is made by a private lender to an eligible nonentitlement city or county. HUD guarantees the loan; however, Texas CDBG must pledge the state's current and future CDBG nonentitlement area funds to cover any losses. To provide eligible nonentitlement communities an additional funding source, the State is authorizing a loan guarantee pilot program for 2008 consisting of one application up to a maximum of $500,000 for a particular project. An application guide containing the submission date and qualifications will be available for applicants interested in being selected as the pilot project under this program.

APPENDIX F
GENERAL CONTAMINANT GEOCHEMISTRY

ARSENIC

The geochemistry of arsenic is complex because of the possible coexistence of two or even three redox states (-III, III, V) and because of the strong interaction of most arsenic compounds with soil particles, particularly iron oxides. Because groundwater is generally oxidizing in the High Plains, Edwards Trinity (Plateau), and Cenozoic Pecos Alluvium aquifers, it is expected to be in the arsenate form (V). Correlations between arsenic and vanadium and fluoride suggest a geologic rather than an anthropogenic source of arsenic. The large number of potential geologic sources include: volcanic ashes in the Ogallala and underlying units, shales in the Cretaceous, and saline lakes in the Southern High Plains that were evaluated in a separate study and described in Scanlon, *et al.* (2005). Arsenic mobility is generally not controlled by solubility of arsenic-bearing minerals because these minerals are highly soluble. Under oxidizing conditions, arsenic mobility increases with increasing pH (Smedley and Kinniburg 2000). Phosphate can also increase arsenic mobility because phosphate preferentially sorbs onto clays and iron oxides relative to arsenic.

NITRATE

Nitrate is negatively charged and behaves conservatively; *i.e.*, it does not sorb onto soil, volatilize, precipitate readily, *etc*. Natural sources of nitrate include fixed nitrogen by shrubs such as mesquite in rangeland settings. Nitrate concentrations in soil profiles in most rangeland settings in the Southern High Plains are generally low (Scanlon, *et al*. 2003; McMahon, *et al*. 2005). Conversion of rangeland to agriculture can result in nitrification of soil organic matter. Anthropogenic sources of nitrate include chemical and organic (manure) fertilizers, nitrogen fixation through growth of leguminous crops, and barnyard and septic tank effluent. Nitrogen isotopes have been used to distinguish these various sources; however, such a study has not been conducted in the Southern High Plains. Nitrogen profiles measured in soil in Dawson County, Texas, indicated that nitrate concentrations in soil pore water were generally low to moderate (Scanlon, *et al*. 2003). The highest concentrations were found in irrigated areas because irrigation water contains higher nitrate concentrations than rain water and irrigation rates are low enough to result in evapoconcentration of nitrate in the soil.

Fluoride

Fluorine exists naturally in solution under one valence, F-, the fluoride ion. Fluoride tends to make complexes and ion pairs with trace elements. It can also sorb significantly to oxides, especially aluminum oxides, and clays (Hem 1985). Its concentration controlled by calcium, as fluorite (CaF_2) is the most common fluorine mineral. Apatite (a calcium phosphate) can also contain a significant amount of fluorine.

SELENIUM

Selenium has a chemistry similar to that of sulfur, existing naturally in four redox states VI, IV, 0, and –II, with selenate, selenite, and selenide ions occurring in Eh-pH conditions largely parallel to those of arsenic. In oxic conditions, the selenate ion, SeO_4^{-2}, is the dominant species across all natural pHs. In slightly reducing conditions, the selenite ion exists from the fully deprotonated form, SeO_3^{-2}, at alkaline pHs to the neutral H_2SeO_3 at acid pHs and the $HSeO_3^{-1}$ form at neutral pHs. However, here are several differences with arsenic. The selenate ion is a weak sorber and its behavior resembles more that of sulfate than that of arsenate ion (White and Dubrovsky 1994). Organo-selenium compounds and possibly native selenium are also more widespread. All selenate and selenite minerals are highly soluble. Native selenium, or more likely ferroselite (pyrite with some Se substituted for S), can precipitate at relatively high Eh neutral pH. However, kinetics issues may keep selenium in solution even at reducing Ehs (Henry, *et al*. 1982).

URANIUM

The geochemistry of uranium is complicated but can be summarized by the following. Uranium(VI) in oxidizing conditions exists as the soluble positively charged uranyl UO_2^{+2}. Solubility is higher at acid pHs, decreases at neutral pHs, and increases at alkaline pHs. The uranyl ion can easily form aqueous complexes, including with hydroxyl, fluoride, carbonate, and phosphate ligands. Hence, in the presence of carbonates, uranium solubility is considerably enhanced in the form of uranyl-carbonate (UO_2CO_3) and other higher order carbonate complexes: uranyl-di-carbonate ($UO_2(CO_3)_2^{-2}$ and uranyl-tri-carbonates $UO_2(CO_3)_3^{-4}$. Adsorption of uranium is inversely related to its solubility and is highest at neutral pHs (De Soto 1978). Uranium sorbs strongly to metal oxides and clays. Uranium(IV) is the other commonly found redox state. In that state, however, uranium is not very soluble and precipitates as uranite, UO_2, coffinite, $USiO_4{}^nH_2O$ (if SiO_2>60 mg/L (Henry, *et al.* 1982), or related minerals. In most aquifers, no mineral controls uranium solubility in oxidizing conditions. However, uranite and coffinite are the controlling minerals if Eh drops below 0-100 mV.

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