DRAFT FEASIBILITY REPORT
FEASIBILITY ANALYSIS OF WATER SUPPLY FOR SMALL PUBLIC WATER SYSTEMS

INDIAN SPRINGS LAKE ESTATE LL

PWS ID# 1870040, CCN# 10147

Prepared for:

THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

Prepared by:

THE UNIVERSITY OF TEXAS BUREAU OF ECONOMIC GEOLOGY

AND

PARSONS

Preparation of this report was financed by the Texas Commission on Environmental Quality through the Drinking Water State Revolving Fund Small Systems Assistance Program

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AUGUST 2006

EXECUTIVE SUMMARY

INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), was contracted by the Texas Commission on Environmental Quality (TCEQ) to conduct a study to assist with identifying and analyzing alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project was to promote compliance using sound engineering and financial methods and data for PWSs that had recently recorded sample results exceeding maximum contaminant levels (MCL). The primary objectives of this project were to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS for future implementation.

The method for this project follows that of a pilot study performed in 2004 and 2005 by TCEQ, BEG, and Parsons. The pilot study evaluated compliance alternatives for three PWSs that had elevated concentrations of nitrate. The pilot project developed a method (a decision tree approach) for identifying and analyzing compliance options.

This feasibility report provides an evaluation of water supply alternatives for the Indian Springs Lake Estate LL PWS, ID# 1870040, Certificate of Convenience and Necessity (CCN) # 10147, located in Polk County, Texas (Indian Springs PWS). Indian Springs PWS is the water system for Indian Springs Lake Estate subdivision, a 500-lot rural subdivision located east of Livingston, Texas. It consists of two water plants (Ole Don and Baker plants) with three active wells, two at Ole Don set at 285 feet, and one at Baker set at 255 feet below ground surface. Ole Don also has two 45,000-gallon ground storage tanks and one 6,000-gallon hydro-pneumatic tank, a treatment shed and a distribution system. Baker plant has one 24,000-gallon ground storage tank and one 2,500-gallon hydro-pneumatic tank, a treatment shed and a distribution system. All the groundwater from both plants is disinfected with gaseous chlorine and treated with tripolyphosphate for iron before entering the distribution system. Recent sample results from the Indian Springs PWS exceeded the MCL for combined radium-226 and radium-228 of 5 picoCuries per liter (pCi/L) and the MCL for gross alpha particle activity of 15 pCi/L (USEPA 2005; TCEQ 2004a).

Basic system information for the Indian Springs PWS is shown in Table ES.1.

Table ES.1
Indian Springs PWS Basic System Information

Parameter	Result
Population served	1,080 current
Connections	360 current
Average daily flow rate	0.127 million gallons per day (mgd)
Peak demand flow rate	0.352 mgd estimated
Water system peak capacity	0.41 mgd
Typical combined radium-226 and 228 range	3.1 – 9.4 pCi/L
Typical gross alpha particle range	8.1 pCi/L to 29 pCi/L

STUDY METHODS

The methods used for this study were based on a pilot study performed in 2004 and 2005 by TCEQ, BEG, and Parsons. Methods for identifying and analyzing compliance options were developed in the pilot study (a decision tree approach).

The process for developing the feasibility study used the following general steps:

Gather data from the TCEQ and Texas Water Development Board databases, from TCEQ files, and from information maintained by the PWS;

Conduct financial, managerial, and technical (FMT) evaluations of the PWS;

Perform a geologic and hydrogeologic assessment of the study area;

Develop treatment and non-treatment compliance alternatives which, in general, consist of the following possible options:

- Connecting to neighboring PWSs via new pipeline or by pumping water from a newly installed well or an available surface water supply within the jurisdiction of the neighboring PWS;
- Installing new wells within the vicinity of the PWS into other aquifers with confirmed water quality standards meeting the MCLs;
- Installing a new intake system within the vicinity of the PWS to obtain water from a surface water supply with confirmed water quality standards meeting the MCLs;
- Treating the existing non-compliant water supply by various methods depending on the type of contaminant; and
- Delivering potable water by way of a bottled water program or a treated water dispenser as an interim measure only.
- Assess each of the potential alternatives with respect to economic and non-economic criteria;

- Prepare a feasibility report and present the results to the PWS.

This basic approach is summarized in Figure ES-1.

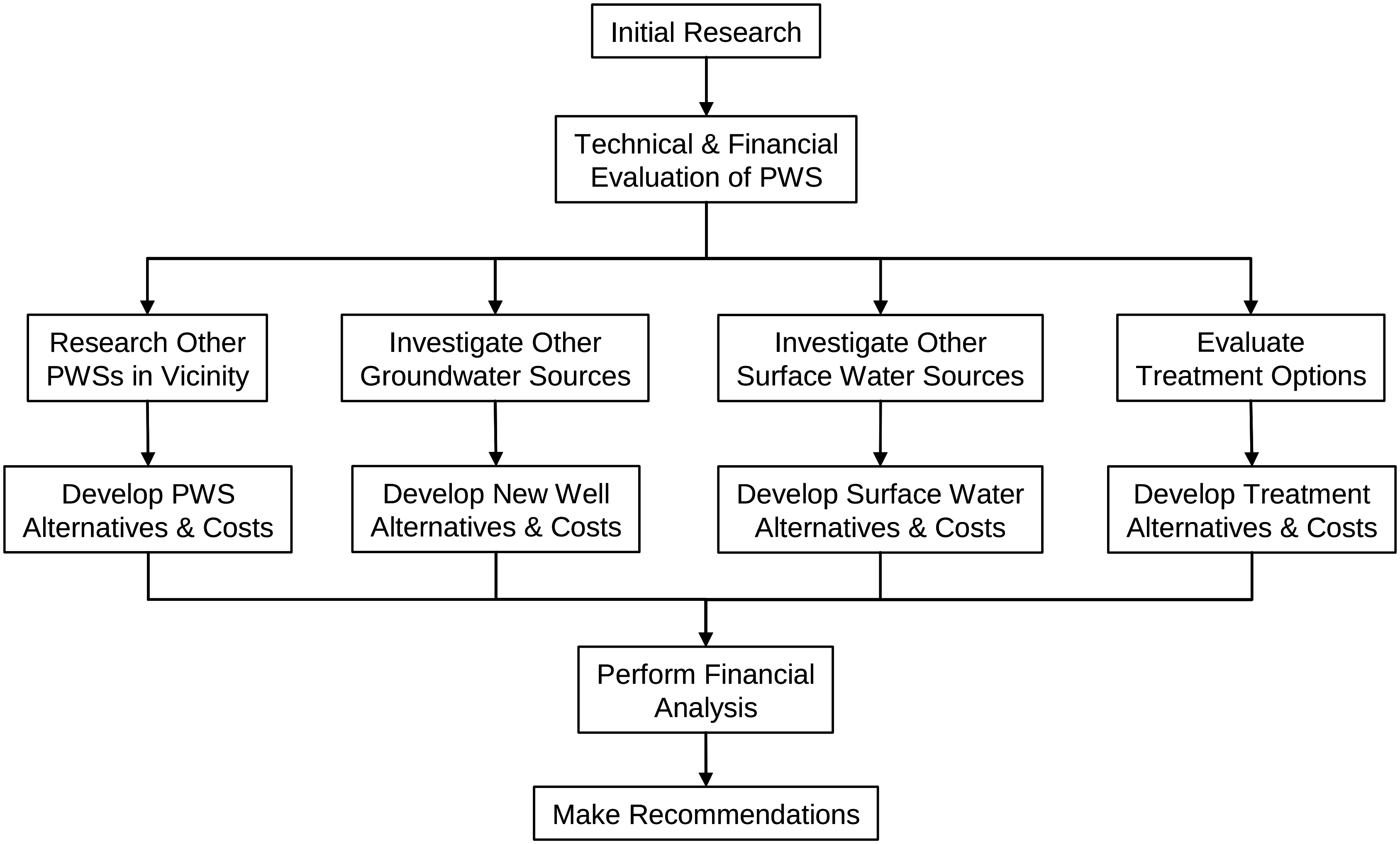
HYDROGEOLOGICAL ANALYSIS

The Indian Springs PWS obtains groundwater from the Burkeville Aquiclude subunit of the Gulf Coast aquifer. Radium and gross alpha particles are not commonly found in area wells at concentrations greater than the MCL. The Jasper subunit aquifers are known to be very productive in the area. Other nearby PWS well screens are generally set either shallower or deeper than the well screen of Indian Springs PWS. However, the variability of radium and gross alpha particle concentrations makes it difficult to determine where wells can be located to produce acceptable water. It may be possible to do down-hole testing on the Indian Springs PWS well to determine the source of the contaminants. If the contaminants derive primarily from a single part of the formation, that part could be excluded by modifying the existing well, or avoided altogether by completing a new well.

COMPLIANCE ALTERNATIVES

The Indian Springs PWS is owned by the Lake Livingston Water Supply & Sewer Service Corporation (WSSSC), which provides water to 52 other PWSs in the greater Lake Livingston area and serves 6,894 customers. The General Manager is Mr. Scott Baker and system operations are managed by Mr. Phillip Everett and Mr. Boyd McDaniel. Overall, the system does have an adequate level of FMT capacity. The system does have positive aspects, including a knowledgeable and dedicated staff, benefits from economies of scale, communication with customers, and a cross-connection control program. Capacity deficiencies are reflected in lack of compliance with radionuclides standard and water losses. Areas of concern for the system included rates and frequency of rate evaluation, lack of written long-term capital improvements plan, preventative maintenance program and an emergency plan.

1 **Figure ES-1 Summary of Project Methods**



There are several PWSs within a few miles of Indian Springs PWS, and most of the nearby systems have good quality water. In general, feasibility alternatives were developed based on obtaining water from the nearest PWSs, either by directly purchasing water, or by expanding the existing well field. Another alternative considered is modifying the existing well or installing a new well at the Indian Springs PWS. There is surface water available in the area that would be a good option for a regional solution, but the high cost of that alternative would likely make this option unattractive.

A number of centralized treatment alternatives for radium and alpha particle removal have been developed and were considered for this report, for example, ion exchange, Water Remediation Technologies, Inc. (WRT) Z-88™ adsorption, and $KMnO_4$ greensand filtration. Point-of-use (POU) and point-of-entry treatment alternatives were also considered. Temporary solutions such as providing bottled water or providing a centralized dispenser for treated or trucked-in water, were also considered as alternatives.

Developing a new well at or near the Indian Springs PWS is likely to be an attractive solution if compliant groundwater can be found. Having a new well at or near the Indian Springs PWS is likely to be one of the lower cost alternatives since the PWS already possesses the technical and managerial expertise, as well as the storage and transmission infrastructure needed to implement this option. The cost of new well alternatives quickly increases with pipeline length, making proximity of the alternate source a key concern. Additionally, there are large water suppliers that would be willing to sell water within a short distance from the Indian Springs PWS. Purchasing water or joining one of the larger PWSs may also be an attractive option for the Lake Livingston WSSSC. A new compliant well or obtaining water from a neighboring compliant PWS has the advantage of providing compliant water that may be blended with the existing source to supply compliant water to all taps in the system.

Central treatment can be cost-competitive with the alternative of new nearby wells, but would require significant institutional changes to manage and operate. Similar to obtaining an alternate compliant water source, central treatment would provide compliant water to all water taps.

POU treatment can be cost competitive, but does not supply compliant water to all taps. Additionally, significant efforts would be required for maintenance and monitoring of the POU treatment units.

Providing compliant water through a central dispenser is significantly less expensive than providing bottled water to 100 percent of the population, but a significant effort is required for clients to fill their containers at the central dispenser.

FINANCIAL ANALYSIS

Financial analysis of the Indian Springs PWS indicated that current water rates are funding operations, and maybe producing a surplus. The current average water bill of $1,439 represents 5.8 percent of the median household income (MHI. Table ES.2 provides a summary of the financial impact of implementing selected compliance alternatives, including the rate increase necessary to meet current operating expenses. The alternatives were selected to highlight results for the best alternatives from each different type or category.

Some of the compliance alternatives offer potential for shared or regional solutions. A group of PWSs could work together to implement alternatives for developing a new groundwater source or expanding an existing source, obtaining compliant water from a large regional provider, or for central treatment. Sharing the cost for implementation of these alternatives could reduce the cost on a per user basis. Additionally, merging PWSs or management of several PWSs by a single entity offers the potential for reduction in administrative costs.

Table ES.2 Selected Financial Analysis Results

Alternative	Funding Option	Average Annual Water Bill	Percent of MHI
Current	NA	$1,439	5.8
New well at Indian Springs	100% Grant	$1,045	4.2
	Loan/Bond	$1,068	4.3
Purchase water from Wilson Lake Estates	100% Grant	$1,275	5.2
	Loan/Bond	$1,599	6.5
Central treatment – WRT Z-88	100% Grant	$1,338	5.4
	Loan/Bond	$1,475	6.0
Point-of-use	100% Grant	$1,660	6.7
	Loan/Bond	$1,713	6.9

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ACRONYMS AND ABBREVIATIONS

μ/L	micrograms per liter
AFY	acre-feet per year
BEG	Bureau of Economic Geology
BV	bed volume
CA	chemical analysis
CCN	Certificate of Convenience and Necessity
CCR	Consumer Confidence Report
CFR	Code of Federal Regulations
CO	correspondence
ED	electrodialysis
EDR	electrodialysis reversal
EP	entry point
FM	farm-to-market road
FMT	financial, managerial, and technical
ft^2	square foot
GAM	Groundwater Availability Model
gpm	gallons per minute
IS	Indian Springs
ISLE	Indian Springs Lake Estates
IX	Ion exchange
$KMnO_4$	hydrous manganese oxide
MCL	Maximum contaminant level
mg/L	milligrams per Liter
mgd	million gallons per day
MHI	median household income
MnO_2	Manganese dioxide
MOR	monthly operating report
NMEFC	New Mexico Environmental Financial Center
NSF	NSF International
NURE	National Uranium Resource Evaluation
O&M	operation and maintenance
Parsons	Parsons Infrastructure and Technology Group Inc.
pCi/L	picoCuries per liter
POE	Point-of-entry
POU	Point-of-use
PWS	public water system
RO	Reverse osmosis
SDWA	Safe Drinking Water Act
TCEQ	Texas Commission on Environmental Quality
TDS	Total dissolved solids
TSS	Total suspended solids
TWDB	Texas Water Development Board
USEPA	United States Environmental Protection Agency
WAM	Water Availability Model
WRT	Water Remediation Technologies, Inc.
WSC	water supply corporation
WSSSC	Water Supply & Sewer Service Corporation

SECTION 1
INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Infrastructure and Technology Group Inc. (Parsons), have been contracted by the Texas Commission on Environmental Quality (TCEQ) to assist with identifying and analyzing compliance alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project is to promote compliance using sound engineering and financial methods and data for PWSs that have recently had sample results that exceed maximum contaminant levels (MCL). The primary objectives of this project are to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS with regard to future implementation. The feasibility studies identify a range of potential compliance alternatives, and present basic data that can be used for evaluating feasibility. The compliance alternatives addressed include a description of what would be required for implementation, conceptual cost estimates for implementation, and non-cost factors that could be used to differentiate between alternatives. The cost estimates are intended for comparing compliance alternatives, and to give a preliminary indication of potential impacts on water rates resulting from implementation.

It is anticipated the PWS will review the compliance alternatives in this report to determine if there are promising alternatives, and then select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation. This report contains a decision tree approach that guided the efforts for this study, and also contains steps to guide a PWS through the subsequent evaluation, selection, and implementation of a compliance alternative.

This feasibility report provides an evaluation of water supply compliance options for the Indian Springs PWS, ID# 1870040, Certificate of Convenience and Necessity (CCN) #10147 located in Polk County. Recent sample results from the Indian Springs Lake Estates PWS exceeded the MCL for combined radium-226 and radium-228 of 5 picoCuries per liter (pCi/L) and the MCL for gross alpha particle activity at 15 pCi/L (USEPA 2005; TCEQ 2004a). The location of the Indian Springs PWS, also referred to as the "study area" in this report, is shown on Figure 1.1. Various water supply and planning jurisdictions are shown on Figure 1.2. These water supply and planning jurisdictions are used in the evaluation of alternate water supplies that may be available in the area.

1.1 PUBLIC HEALTH AND COMPLIANCE WITH MCLS

The goal of this project is to promote compliance for PWSs that supply drinking water exceeding regulatory MCLs. This project only addresses those contaminants and does not address any other violations that may exist for a PWS. As mentioned above, the Indian Springs

PWS had recent sample results exceeding the MCL for combined radium-226 and radium-228 and gross alpha particles. In general, contaminant(s) in drinking water above the MCL(s) can have both short-term (acute) and long-term or lifetime (chronic) effects. Long-term ingestion of drinking water with radium-226 and/or radium-228 and/or gross alpha particles above the MCL may increase the risk of cancer (USEPA 2005).

1.2 METHOD

The method for this project follows that of the pilot study performed in 2004 and 2005 by TCEQ, BEG, and Parsons. The pilot study evaluated water supply alternatives for PWSs that supply drinking water with nitrate concentrations above U.S. Environmental Protection Agency (USEPA) and Texas drinking water standards. Three PWSs were evaluated in the pilot study to develop the method (*i.e.*, decision tree approach) for analyzing options for provision of compliant drinking water. This project is performed using the decision tree approach developed in the pilot study.

Other tasks of the feasibility study are as follows:

- Identifying available data sources;
- Gathering and compiling data;
- Conducting financial, managerial, and technical (FMT) evaluations of the selected PWSs;
- Performing a geologic and hydrogeologic assessment of the study area;
- Developing treatment and non-treatment compliance alternatives;
- Assessing potential alternatives with respect to economic and non-economic criteria;
- Preparing a feasibility report; and
- Suggesting refinements to the approach for future studies.

The remainder of Section 1 of this report addresses the regulatory background, and provides a summary of radium abatement options. Section 2 describes the method used to develop and assess compliance alternatives. The groundwater sources of radium-226 and radium-228 and gross alpha particles are addressed in Section 3. Findings for the Indian Springs PWS, along with compliance alternatives development and evaluation, can be found in Section 4. Section 5 references the sources used in this report.

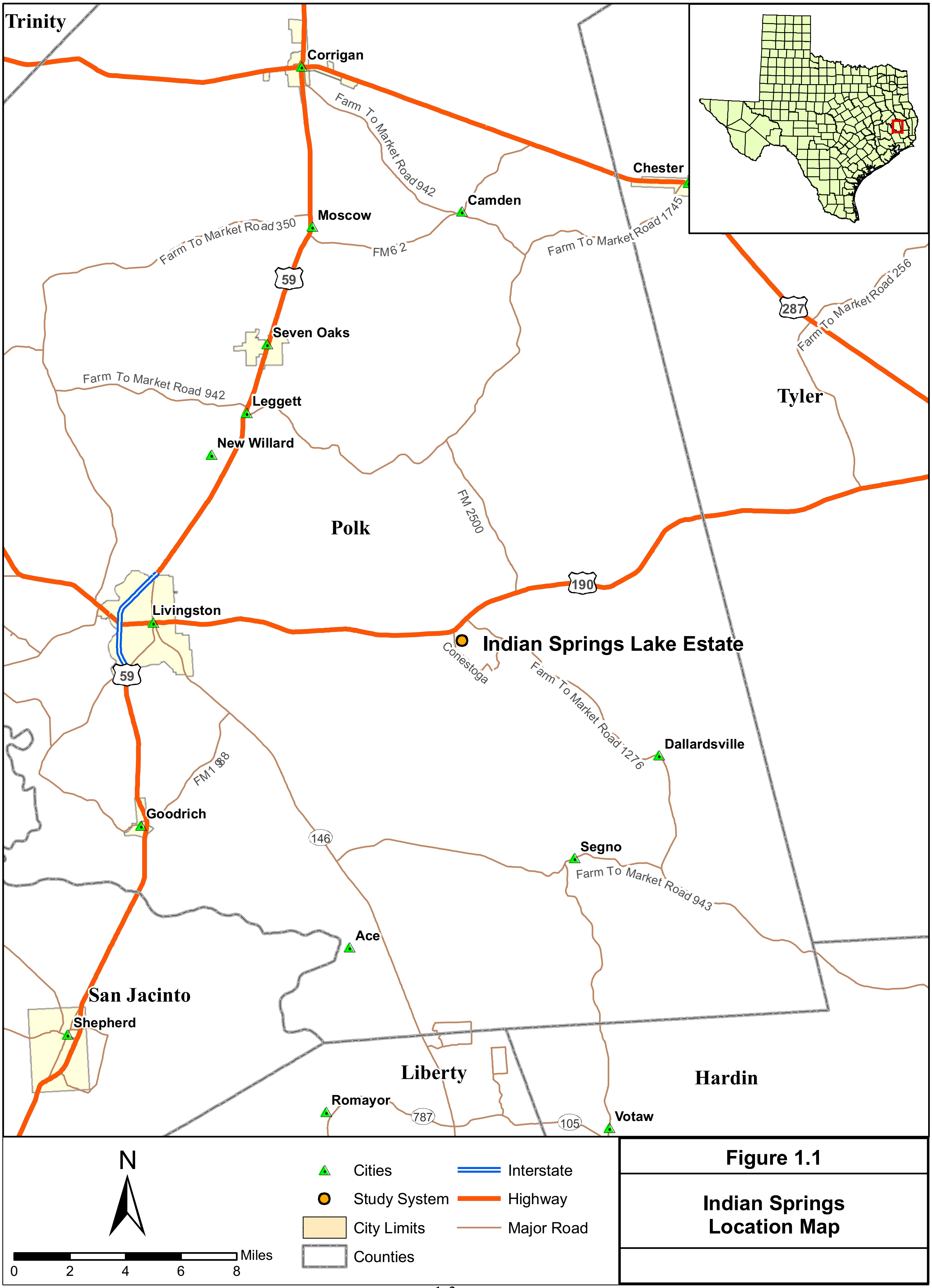

Figure 1.1

Indian Springs Location Map

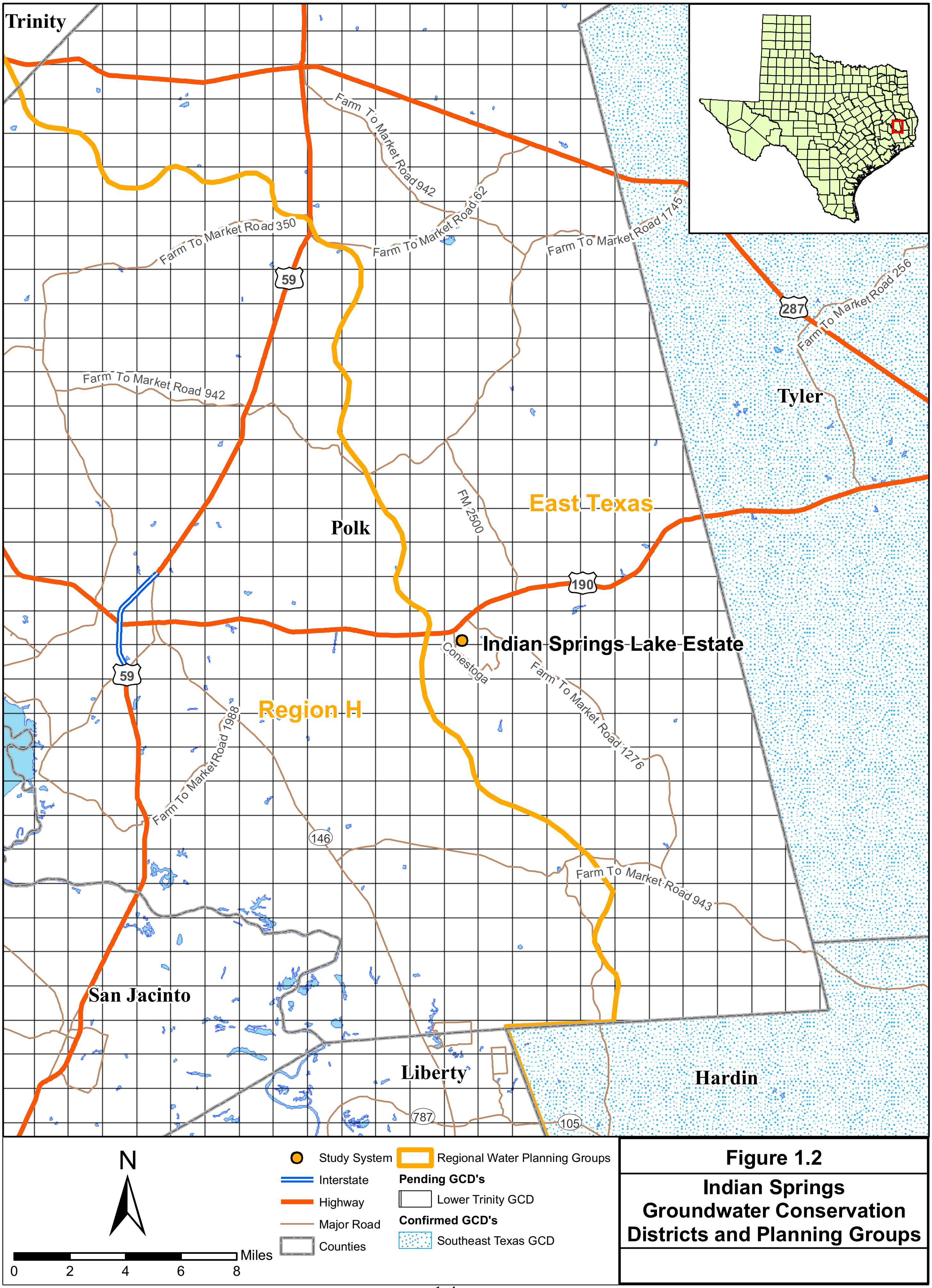

Figure 1.2

Indian Springs Groundwater Conservation Districts and Planning Groups

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1.3 REGULATORY PERSPECTIVE

The Utilities & Districts and Public Drinking Water Sections of the TCEQ Water Supply Division are responsible for implementing requirements of the Federal Safe Drinking Water Act (SDWA) which include oversight of PWSs and water utilities. These responsibilities include:

- Monitoring public drinking water quality;
- Processing enforcement referrals for MCL violators;
- Tracking and analyzing compliance options for MCL violators;
- Providing FMT assessment and assistance to PWSs;

Participating in the Drinking Water State Revolving Fund program to assist PWSs in achieving regulatory compliance; and

- Setting rates for privately-owned water utilities.

This project was conducted to assist in achieving these responsibilities.

1.4 ABATEMENT OPTIONS

When a PWS exceeds a regulatory MCL, the PWS must take action to correct the violation. The MCL exceedances at the Indian Springs PWS involve radium-226 and radium-228 and alpha particles. The following subsections explore alternatives considered as potential options for obtain/providing compliant drinking water.

1.4.1 Existing Public Water Supply Systems

A common approach to achieving compliance is for the PWS to make arrangements with a neighboring PWS for water supply. For this arrangement to work, the PWS from which water is being purchased (supplier PWS) must have water in sufficient quantity and quality, the political will must exist, and it must be economically feasible.

1.4.1.1 Quantity

For purposes of this report, quantity refers to water volume, flowrate, and pressure. Before approaching a potential supplier PWS, the non-compliant PWS should determine its water demand on the basis of average day and maximum day. Peak instantaneous demands can be met through proper sizing of storage facilities. Further, the potential for obtaining the appropriate quantity of water to blend to achieve compliance should be considered. The concept of blending involves combining water with low levels of contaminants with non-compliant water in sufficient quantity so the resulting blended water is compliant. The exact blend ratio would depend on the quality of the water a potential supplier PWS can provide, and would likely vary over time. If high quality water is purchased, produced or otherwise obtained, blending can reduce the amount of high quality water required. Implementation of blending will require a control system to ensure the blended water is compliant.

If the supplier PWS does not have sufficient quantity, the non-compliant community could pay for the facilities necessary to increase the quantity to the extent necessary to supply the needs of the non-compliant PWS. Potential improvements might include, but are not limited to:

- Additional wells;
- Developing a new surface water supply,
- Additional or larger-diameter piping;
- Increasing water treatment plant capacity
- Additional storage tank volume;
- Reduction of system losses,
- Higher-pressure pumps; or
- Upsized, or additional, disinfection equipment.

In addition to the necessary improvements, a transmission pipeline would need to be constructed to tie the two PWSs together. The pipeline must tie-in at a point in the supplier PWS where all the upstream pipes and appurtenances are of sufficient capacity to handle the new demand. In the non-compliant PWS, the pipeline must tie in at a point where no down stream bottlenecks are present. If blending is the selected method of operation, the tie-in point must be at the proper point of the existing non-compliant PWS to ensure that all the water in the system is blended to achieve regulatory compliance.

1.4.1.2 Quality

If a potential supplier PWS obtains its water from the same aquifer (or same portion of the aquifer) as the non-compliant PWS, the quality of water may not be significantly better. However, water quality can vary significantly due to well location, even within the same aquifer. If localized areas with good water quality cannot be identified, the non-compliant PWS would need to find a potential supplier PWS that obtains its water from a different aquifer or from a surface water source. Additionally, a potential supplier PWS may treat non-compliant raw water to an acceptable level.

Surface water sources may offer a potential higher-quality source. Since there are significant treatment requirements, utilization of surface water for drinking water is typically most feasible for larger local or regional authorities or other entities that may provide water to several PWSs. Where PWSs that obtain surface water are neighbors, the non-compliant PWS may need to deal with those systems as well as with the water authorities that supply the surface water.

1.4.2 Potential for New Groundwater Sources

1.4.2.1 Existing Non-Public Supply Wells

Often there are wells not associated with PWSs located in the vicinity of the non-compliant PWS. The current use of these wells may be for irrigation, industrial purposes, domestic supply, stock watering, and other purposes. The process for investigating existing wells is as follows:

- Use existing data sources (see below) to identify wells in the areas that have satisfactory quality. For Indian Springs PWS, the following standards could be used in a rough screening to identify compliant groundwater in surrounding systems:
    - Radium (total radium for radium-226 and radium-228) less than 4 pCi/L (below the MCL of 5 pCi/L); and
    - Gross alpha particle activity less than 12 pCi/L (below the MCL of 15 pCi/L).
- Review the recorded well information to eliminate those wells that appear to be unsuitable for the application. Often, the "Remarks" column in the Texas Water Development Board (TWDB) hard-copy database provides helpful information. Wells eliminated from consideration generally include domestic and stock wells, dug wells, test holes, observation wells, seeps and springs, destroyed wells, wells used by other communities, *etc*.
- Identify wells of sufficient size which have been used for industrial or irrigation purposes. Often the TWDB database will include well yields, which may indicate the likelihood that a particular well is a satisfactory source.
- At this point in the process, the local groundwater control district (if one exists) should be contacted to obtain information about pumping restrictions. Also, preliminary cost estimates should be made to establish the feasibility of pursuing further well development options.
- If particular wells appear to be acceptable, the owner(s) should be contacted to ascertain their willingness to work with the PWS. Once the owner agrees to participate in the program, questions should be asked about the wells. Many owners have more than one well, and would probably be the best source of information regarding the latest test dates, who tested the water, flowrates, and other well characteristics.
- After collecting as much information as possible from cooperative owners, the PWS would then narrow the selection of wells and sample and analyze them for quality. Wells with good quality would then be potential candidates for test pumping. In some cases, a particular well may need to be refurbished before test pumping. Information obtained from test pumping would then be used in combination with information about the general characteristics of the aquifer to determine whether a well at this location would be suitable as a supply source.

- It is recommended that new wells be installed instead of using existing wells to ensure the well characteristics are known and the well meets construction standards.
- Permit(s) would then be obtained from the groundwater control district or other regulatory authority, and an agreement with the owner (purchase or lease, access easements, *etc*.) would then be negotiated.

1.4.2.2 Develop New Wells

If no existing wells are available for development, the PWS or group of PWSs has an option of developing new wells. Records of existing wells, along with other hydrogeologic information and modern geophysical techniques, should be used to identify potential locations for new wells. In some areas, the TWDB's Groundwater Availability Model (GAM) may be applied to indicate potential sources. Once a general area has been identified, land owners and regulatory agencies should be contacted to determine an exact location for a new well or well field. Pump tests and water quality tests would be required to determine if a new well will produce an adequate quantity of good quality water. Permits from the local groundwater control district or other regulatory authority could also be required for a new well.

1.4.3 Potential for Surface Water Sources

Water rights law dominates the acquisition of water from surface water sources. For a PWS, 100 percent availability of water is required, except where a back-up source is available. For PWSs with an existing water source, although it may be non-compliant because of elevated concentrations of one or more parameters, water rights may not need to be 100 percent available.

1.4.3.1 Existing Surface Water Sources

"Existing surface water sources" of water refers to municipal water authorities and cities that obtain water from surface water sources. The process of obtaining water from such a source is generally less time consuming and less costly than the process of developing a new source; therefore, it should be a primary course of investigation. An existing source would be limited by its water rights, the safe yield of a reservoir or river, or by its water treatment or water conveyance capability. The source must be able to meet the current demand and honor contracts with communities it currently supplies. In many cases, the contract amounts reflect projected future water demand based on population or industrial growth.

A non-compliant PWS would look for a source with sufficient spare capacity. Where no such capacity exists, the non-compliant PWS could offer to fund the improvements necessary to obtain the capacity. This approach would work only where the safe yield could be increased (perhaps by enlarging a reservoir) or where treatment capacity could be increased. In some instances water rights, where they are available, could possibly be purchased.

In addition to securing the water supply from an existing source, the non-compliant PWS would need to arrange for transmission of the water to the PWS. In some cases, that could require negotiations with, contracts with, and payments to an intermediate PWS (an intermediate PWS is one where the infrastructure is used to transmit water from a "supplier" PWS to a "supplied" PWS, but does not provide any additional treatment to the supplied water). The non-compliant PWS could be faced with having to fund improvements to the intermediate PWS in addition to constructing its own necessary transmission facilities.

1.4.3.2 New Surface Water Sources

Communication with the TCEQ and relevant planning groups from the beginning is essential in the process of obtaining a new surface water source. Preliminary assessment of the potential for acquiring new rights may be based on surface water availability maps located on the TWDB website. Where water rights appear to be available, the following activities need to occur:

- Discussions with TCEQ to indicate the likelihood of obtaining those rights. The TCEQ may use the Water Availability Model (WAM) to assist in the determination.
- Discussions with land owners to indicate potential treatment plant locations.
- Coordination with U.S. Army Corps of Engineers and local river authorities.
- Preliminary engineering design to determine the feasibility, costs, and environmental issues of a new treatment plant.

Should these discussions indicate that a new surface water source is the best option, the community would proceed with more intensive planning (initially obtaining funding), permitting, land acquisition, and detailed designs.

1.4.4 Identification of Treatment Technologies for Radionuclides

Various treatment technologies were also investigated as compliance alternatives for treatment of radium to regulatory level (*i.e.*, MCL). The removal of radium would also remove gross alpha activity as the radium appears to be responsible for most of the gross alpha activity of the groundwater. Radium-226 and radium-228 are cations (Ra^{2+}) dissolved in water and are not easily removed by particle filtration. A 2002 USEPA document (*Radionuclides in Drinking Water: A Small Entity Compliance Guide*, EPA 815-R-02-001) lists a number of small system compliance technologies that can remove radium (combined radium-226 and radium-228) from water. These technologies include ion exchange, reverse osmosis (RO), electrodialysis/ electrodialysis reversal (ED/EDR), lime softening, greensand filtration, re-formed hydrous manganese oxide filtration ($KMnO_4$-filtration), and co-precipitation with barium sulfate. A relatively new process using the Water Remediation Technologies, Inc. (WRT) Z-88™ media that is specific for radium adsorption has been demonstrated to be an effective radium technology. Lime softening and co-precipitation with barium sulfate are technologies that are relatively complex and require chemistry skills that are not practical for small systems with limited resources and hence they are not evaluated further.

1.4.5 Description of Treatment Technologies

The application radium removal treatment technologies include ion exchange (IX), WRT Z-88 media adsorption, RO, ED/EDR, and $KMnO_4$-greensand filtration. A description of these technologies follows.

1.4.5.1 Ion Exchange

Process – In solution, salts separate into positively charged cations and negatively-charged anions. Ion exchange is a reversible chemical process in which ions from an insoluble, permanent, solid resin bed are exchanged for ions in the water. The process relies on the fact that certain ions are preferentially adsorbed on the ion exchange resin. Operations begin with a fully charged cation or anion bed having enough positively or negatively charged ions to carry out the cation or anion exchange. Usually a polymeric resin bed is composed of millions of spherical beads about the size of medium sand grains. As water passes the resin bed, the charged ions are released into the water, being substituted or replaced with the contaminants in the water (ion exchange). When the resin becomes exhausted of positively or negatively charged ions, the bed must be regenerated by passing a strong, sodium chloride, solution over the resin, displacing the contaminants ions with sodium ions for cation exchange and chloride ion for anion exchange. Many different types of resins can be used to reduce dissolved contaminant concentrations. The IX treatment train for groundwater typically includes cation or anion resins beds with a regeneration system, chlorine disinfection, and clear well storage. Treatment trains for surface water may also include raw water pumps, debris screens, and filters for pre-treatment. Additional treatment or management of the concentrate and the removed solids would be necessary prior to disposal, especially for radium removal resins which have elevated radioactivity.

For radium removal, a strong acid cation exchange resin in the sodium form can remove 99 percent of the radium. The strong acid resin has less capacity for radium on water with high hardness and it has the following adsorption preference: $Ra^{2+} > Ba^{2+} > Ca^{2+} > Mg^{2+} > Na^{+}$. Because of the selectivity, radium and barium are much more difficult to remove from the resin during regeneration than calcium and magnesium. Economical regeneration removes most of the hardness ions, but radium and barium buildup on the resin after repeated cycles to the point where equilibrium is reached and then radium and barium will begin to breakthrough shortly after hardness. Regeneration of the sodium form strong acid resin for water with 200 milligram per liter (mg/L) of hardness with application of 6.5-pound $NaCl/ft^3$ resin would produce 2.4 bed volumes (BV) of 16,400 mg/L total dissolved solids (TDS) brine per 100 BV of product water (2.4%). The radium concentration in the regeneration waste would be approximately 40 times the influent radium concentration in groundwater.

Pretreatment – There are pretreatment requirements for pH, organics, turbidity, and other raw water characteristics. Pretreatment may be required to reduce excessive amounts of total suspended solids (TSS), iron, and manganese, which could plug the resin bed, and typically includes media or carbon filtration.

Maintenance – The IX resin requires regular on-site regeneration, the frequency of which depends on the raw water characteristics (especially hardness), the contaminant concentration, and the size and number of the IX vessels. Many systems have undersized the IX vessels only to realize higher than necessary operating costs. Preparation of the sodium chloride solution is required. If used, filter replacement and backwashing would be required.

Waste Disposal – Approval from local authorities is usually required for disposal of concentrate from the regeneration cycle (highly concentrated salt solution with radioactivity); occasional solid waste (in the form of broken resin beads) from the resin backwashes during regeneration; and if used, spent filters and backwash wastewater.

Advantages

- Well-established process for radium removal.
- Fully automated and highly reliable process.
- Suitable for small and large installations.

Disadvantages

- Requires salt storage; regular regeneration.
- Concentrate disposal.
- Resins are sensitive to the presence of competing ions such as calcium and magnesium.

In considering application of IX for inorganics, it is important to understand what the effect of competing ions would be, and to what extent the brine can be recycled. Conventional IX cationic resin removes calcium and magnesium in addition to radium and, thus, the capacity for radium removal and frequency of regeneration depend on the hardness of the water to be treated. Spent regenerant is produced during IX bed regeneration, and it may have concentrations of the sorbed contaminants which would be expensive to treat and/or dispose because of hazardous waste regulations.

1.4.5.2 WRT Z-88 Media

Process – The WRT Z-88 radium treatment process is a proprietary process using a radium-specific adsorption resin or zeolite supplied by WRT. The Z-88 process is similar to IX except that no regeneration of the resin is conducted and the resin is disposed upon exhaustion. The Z-88 does not remove calcium and magnesium and, thus, can last for 2-4 years, according to WRT, before replacement is necessary. The process is operated in an upflow, fluidized mode with a surface loading rate of 10.5 gallons per minute (gpm) per square foot (ft^2). Pilot testing of this technology has been conducted for radium removal successfully in many locations, including the State of Texas. Seven full-scale systems with capacities of 750 to 1,200 gpm/ft^2 have been constructed in the Village of Oswego, Illinois since July 2005. The treatment equipment is owned by WRT and ownership of spent media is transferred to an approved disposal site. The customer pays WRT based on an agreed upon treated water unit cost (*e.g.,* $.0.50-1.00/1,000 gallons, depending on site location and volume).

Pretreatment – Pretreatment may be required to reduce excess amounts of TSS, iron, and manganese, which could plug the resin bed, and typically includes media or carbon filtration. No chemical addition is required for radium removal.

Maintenance – Maintenance is relatively low for this technology as no regeneration or chemical handling is required. Periodic water quality monitoring and inspection of mechanical equipment are required.

Waste Disposal – The Z-88 media would be disposed in an approved low level radioactive waste landfill by WRT once every 2-4 years. No liquid waste is generated for this process. However, if pretreatment filters are used then spent filters and backwash wastewater disposal would be required.

Advantages (IX)

- Simple and fully automated process.
- No liquid waste disposal.
- No chemical handling, storage, or feed systems.
- No change in water quality except radium reduction.
- Low capital cost as WRT owns the equipment.

Disadvantages (IX)

- Relatively new technology.
- Proprietary technology without direct competition.
- Long term contract with WRT required.

From a small utilities point of view, the Z-88 process is a desirable technology for radium removal as operation and maintenance (O&M) efforts are minimal and no regular liquid waste is generated. However, this technology is very new and has no long-term full-scale operating experience. But since the equipment is owned by WRT and performance is guaranteed by WRT the risk to the PWSs is minimized.

1.4.5.3 Reverse Osmosis

Process – RO is a pressure-driven membrane separation process capable of removing dissolved solutes from water by means of particle size and electrical charge. The raw water is typically called feed; the product water is called permeate, and the concentrated reject is called concentrate. Common RO membrane materials include asymmetric cellulose acetate and polyamide thin film composite. Common RO membrane configurations include spiral wound and hollow fine fiber, but most RO systems to date are of the spiral wound type. A typical RO installation includes a high pressure feed pump with chemical feed, parallel first and second stage membrane elements in pressure vessels, and valving and piping for feed, permeate, and concentrate streams. Factors influencing membrane selection are cost, recovery, rejection, raw water characteristics, and pretreatment. Factors influencing performance are raw water characteristics, pressure, temperature, and regular monitoring and maintenance. RO is capable of achieving over 95 percent removal of radium. The treatment process is relatively insensitive to pH. Water recovery is 60-80 percent, depending on the raw water characteristics. The concentrate volume for disposal can be significant.

Pretreatment – RO requires careful review of raw water characteristics and pretreatment is necessary to prevent membranes from fouling, scaling, or degrading other membranes. Removal or sequestering of suspended and colloidal solids is necessary to prevent fouling, and removal of sparingly soluble constituents such as calcium, magnesium, silica, sulfate, barium, *etc*., may be required to prevent scaling. Pretreatment can include media filters, ion exchange softening, acid and antiscalant feed, activated carbon or bisulfite feed to dechlorinate, and cartridge filters to remove any remaining suspended solids to protect membranes from upsets.

Maintenance – Monitoring rejection percentage is required to ensure contaminant removal below MCL. Regular monitoring of membrane performance is necessary to determine fouling, scaling, or other membrane degradation. Acidic or caustic solutions are regularly flushed through the system at high volume/low pressure along with a cleaning agent to remove foulants and scalants. Frequency of membrane replacement is dependent on raw water characteristics, pretreatment, and maintenance.

Waste Disposal – Pretreatment waste streams, concentrate flows, spent filters, and membrane elements all require approved disposal methods. Disposal of the significant volume of the concentrate stream is a problem for many utilities.

Advantages (RO)

- Can remove radium effectively.
- Can remove other undesirable dissolved constituents.

Disadvantages (RO)

- Relatively expensive to install and operate.
- Needs sophisticated monitoring systems.
- Requires concentrate disposal
- Needs to handle multiple chemicals.
- Waste of water because of the significant concentrate flows.

RO is an expensive alternative for removal of radium and is usually not economically competitive with other processes unless nitrate and/or TDS removal is also required. The biggest drawback for using RO to remove radium is the waste of water through concentrate disposal which is also difficult or expensive because of the volume involved.

1.4.5.4 Electrodialysis/Electrodialysis Reversal

Process – ED is an electrochemical separation process in which ions migrate through ion-selective semi-permeable membranes as a result of their attraction to two electrically charged electrodes. The driving force for ion transfer is direct electric current. ED is different from RO in that it removes only dissolved inorganics but not particulates, organics, and silica. EDR is an improved form of ED in which the polarity of the direct current is changed approximately every 15 minutes. The change of polarity helps reduce the formation of scale and fouling films and, thus, achieves higher water recovery. EDR has been the dominant form of ED systems used for the past 25-30 years. A typical EDR system includes a membrane stack with a number of cell pairs, each consisting of a cation transfer membrane, a demineralized water flow spacer, an anion transfer membrane, and a concentrate flow spacer. Electrode compartments are at opposite ends of the stack. The influent feed water (chemically treated to prevent precipitation) and concentrate reject flow in parallel across the membranes and through the demineralized water and concentrate flow spacers, respectively. The electrodes are continually flushed to reduce fouling or scaling. Careful consideration of flush feed water is required. Typically, the membranes are cation or anion exchange resins cast in sheet form; the spacers are high density polyethylene; and the electrodes are inert metal. EDR stacks are tank-contained and often staged. Membrane selection is based on review of raw water characteristics. A single-stage EDR system usually removes 40-50 percent of the dissolved salts, including radium, and multiple stages may be required to meet the MCL if radium concentration is high. The conventional EDR treatment train typically includes EDR membranes, chlorine disinfection, and clearwell storage.

Pretreatment – There are pretreatment requirements for pH, organics, turbidity, and other raw water characteristics. EDR typically requires acid and antiscalant feed to prevent scaling and a cartridge filter for prefiltration. Treatment of surface water may also require pretreatment steps such as raw water pumps, debris screens, rapid mix with addition of a coagulant, flocculation basin, sedimentation basin or clarifier, and gravity filters. Microfiltration could be used in place of flocculation, sedimentation, and filtration.

Maintenance – EDR membranes are durable, can tolerate pH from 1-10 and temperatures to 115^{o}F for cleaning. The membranes can be removed from the unit and scrubbed. Solids can be washed off by turning the power off and letting water circulate through the stack. Electrode washes flush out byproducts of electrode reaction. The byproducts are hydrogen, formed in the cathode space, and oxygen and chlorine gas, formed in the anode space. If the chlorine is not removed, toxic chlorine gas could form. Depending on the raw water characteristics, the membranes would require regular maintenance or replacement. If used, pretreatment filter replacement and backwashing would be required. The EDR stack must be disassembled, mechanically cleaned, and reassembled at regular intervals.

Waste Disposal – Highly concentrated reject flows, electrode cleaning flows, and spent membranes require approved disposal methods. Pretreatment process residuals and spent materials also require approved disposal methods.

Advantages (EDR)

- EDR can operate with minimal fouling, scaling, or chemical addition.
- Low pressure requirements; typically quieter than RO.
- Long membrane life expectancy.
- More flexible than RO in tailoring treated water quality requirements.

Disadvantages (EDR)

- Not suitable for high levels of iron, manganese, hydrogen sulfide, and hardness.
- Relatively expensive process and high energy consumption.
- Does not remove particulates, organics, or silica.

EDR can be quite expensive to run because of its energy usage. If radium removal is the only purpose, it is probably more expensive than other technologies; however, if nitrate and/or TDS removal is also required, then EDR is a competitive process.

1.4.5.5 Potassium Permanganate Greensand Filtration

Process – Manganese dioxide (MnO_2) is known to have capacity to adsorb radium from water. MnO_2 can be formed by oxidation of Mn^{2+} occurring in natural waters and/or reduction of hydrous manganese oxide ($KMnO_4$) added to the water. The MnO_2 is in the form of colloidal MnO_2 which has a large surface area for adsorption. The MnO_2 does not adsorb calcium and magnesium so hardness is not a factor, but iron and manganese and other heavy metal cations can compete strongly with radium adsorption. If these cations are present it would be necessary to install a good iron and manganese removal process before the MnO_2 filtration process or to make sure some MnO_2 is still available for radium sorption. The $KMnO_4$-greensand filtration process can accomplish this purpose because it is coated with MnO_2 which is regenerated by the continuous feeding of $KMnO_4$. Many operating treatment systems utilizing continuous feed $KMnO_4$, 30-minute contact time, and manganese greensand, remove radium to concentrations below the MCL. The treatment system equipment includes a $KMnO_4$ feed system, a pressurized reaction tank, and a manganese greensand filter. Backwashing of the greensand filter is usually required, but periodic regeneration is not required.

Pretreatment – The $KMnO_4$-greensand filtration process usually does not require pretreatment except if turbidity is very high. The greensand filter usually has an anthracite layer to filter larger particles, while the greensand adsorbs dissolved cations such as radium.

Maintenance – The greensand requires periodic backwashing to remove suspended materials and metal oxides. $KMnO_4$ is usually supplied in powder form, and preparation of $KMnO_4$ solution is required. Occasional monitoring to ensure no overfeeding of $KMnO_4$ (pink water) is important to avoid problems in the distribution system and household fixtures.

Waste Disposal – Approval from local authorities is usually required for disposal/discharge of the backwash wastewater. If local sewer is not available, a backwash storage and settling tank would be required to recycle settled water to the process and periodically dispose of the settled solids.

Advantages

- Well established process for radium removal.
- No regeneration waste generated.
- Low pressure operation and no repumping required.
- No additional process for iron and manganese removal.

Disadvantages

- Need to handle powdered $KMnO_4$, which is an oxidant.
- Need to monitor and backwash regularly.

The $KMnO_4$-greensand filtration is a well-established iron and manganese removal process and is effective for radium removal. It is suitable for small and large systems and is cost competitive with other alternative technologies.

1.4.6 Point-of-Entry and Point-of-Use Treatment Systems

Point-of-entry (POE) and point-of-use (POU) treatment systems can be used to provide compliant drinking water. For radium and gross alpha particle removal, these systems typically use small adsorption or reverse osmosis treatment units that are installed "under the sink" in the case of point-of-use, and where water enters a house or building in the case of point-of-entry. It should be noted that POU treatment units would need to be more complex than units typically found in commercial retail outlets in order to meet regulatory requirements, making purchase and installation more expensive. POE and POU treatment units would be purchased and owned by the PWS. These solutions are decentralized in nature, and require utility personnel entry into houses or at least onto private property for installation, maintenance, and testing. Due to the large number of treatment units that would be employed and would be primarily out of the control of the PWS, it is very difficult to ensure 100 percent compliance. Prior to selection of a POE or POU program for implementation, consultation with TCEQ would be required to address measurement and determination of level of compliance.

The SDWA [§1412(b)(4)(E)(ii)] regulates the design, management and operation of POU and POE treatment units used to achieve compliance with an MCL. These restrictions, relevant to radium and gross alpha particles are:

- POU and POE treatment units must be owned, controlled, and maintained by the PWS, although the utility may hire a contractor to ensure proper O&M and MCL compliance. The PWS must retain unit ownership and oversight of unit installation, maintenance and sampling; the utility ultimately is the responsible party for regulatory compliance. The PWS staff need not perform all installation, maintenance, or management functions, as these tasks may be contracted to a third party, but the final responsibility for the quality and quantity of the water supplied to the community resides with the PWS, and it must monitor all contractors closely. Responsibility for O&M of POU or POE devices installed for SDWA compliance may not be delegated to homeowners.
- POU and POE units must have mechanical warning systems to automatically notify customers of operational problems. Each POU or POE treatment device must be equipped with a warning device (*e.g.*, alarm, light) that would alert users when their unit is no longer adequately treating their water. As an alternative, units may be equipped with an automatic shut-off mechanism to meet this requirement.
- If the American National Standards Institute has issued product standards for a specific type of POU or POE treatment unit, only those units that have been independently certified according to those standards may be used as part of a compliance strategy.

The following observations with regard to using POE and POU devices for SDWA compliance were made by Raucher, *et al.* (2004):

- If POU devices are used as an SDWA compliance strategy, certain consumer behavioral changes will be necessary (*e.g.*, encouraging people to drink water only from certain treated taps) to ensure comprehensive consumer health protection.
- Although not explicitly prohibited in the SDWA, USEPA indicates that POU treatment devices should not be used to treat for radon or for most volatile organic contaminants to achieve compliance, because POU devices do not provide 100 percent protection against inhalation or contact exposure to those contaminants at untreated taps (*e.g.*, shower heads).
- Liability – PWSs considering unconventional treatment options (POU, POE, or bottled water) must address liability issues. These could be meeting drinking water standards, property entry and ensuing liabilities, and damage arising from improper installation or improper function of the POU and POE devices.

1.4.7 Water Delivery or Central Drinking Water Dispensers

Current USEPA regulations (40 Code of Federal Regulations [CFR] 141.101) prohibit the use of bottled water to achieve compliance with an MCL, except on a temporary basis. State regulations do not directly address the use of bottled water. Use of bottled water at a non-compliant PWS would be on a temporary basis. Every 3 years, the PWSs that employ interim measures are required to present the TCEQ with estimates of costs for piping compliant water to their systems. As long as the projected costs remain prohibitively high, the bottled water interim measure is extended. Until USEPA amends the noted regulation, the TCEQ is unable to accept water delivery or central drinking water dispensers as compliance solutions.

Central provision of compliant drinking water would consist of having one or more dispensers of compliant water where customers could come to fill containers with drinking water. The centralized water source could be from small to medium-sized treatment units or could be compliant water delivered to the central point by truck.

Water delivery is an interim measure for providing compliant water. As an interim measure for a small impacted population, providing delivered drinking water may be cost effective. If the susceptible population is large, the cost of water delivery would increase significantly.

Water delivery programs require consumer participation to a varying degree. Ideally, consumers would have to do no more than they currently do for a piped-water delivery system. Least desirable are those systems that require maximum effort on the part of the customer (*e.g.*, customer has to travel to get the water, transport the water, and physically handle the bottles). Such a system may appear to be lowest-cost to the utility; however, should a consumer experience ill effects from contaminated water and take legal action, the ultimate cost could increase significantly.

The ideal system would:

- Completely identify the susceptible population. If bottled water is only provided to customers who are part of the susceptible population, the utility should have an active means of identifying the susceptible population. Problems with illiteracy, language fluency, fear of legal authority, desire for privacy, and apathy may be reasons that some members of the susceptible population do not become known to the utility, and do not take part in the water delivery program.
- Maintain customer privacy by eliminating the need for utility personnel to enter the home.
- Have buffer capacity (*e.g.*, two bottles in service, so when one is empty, the other is being used over a time period sufficient to allow the utility to change out the empty bottle).
- Provide for regularly scheduled delivery so the customer would not have to notify the utility when the supply is low.
- Use utility personnel and equipment to handle water containers, without requiring customers to lift or handle bottles with water in them.
- Be sanitary (*e.g.*, where an outside connection is made, contaminants from the environment must be eliminated).
- Be vandal-resistant.
- Avoid heating the water due to exterior temperatures and solar radiation.

- Avoid freezing the water.

SECTION 2
EVALUATION METHOD

2.1 DECISION TREE

The decision tree is a flow chart for conducting feasibility studies for a non-compliant PWS. The decision tree is shown in Figures 2.1 through 2.4. The tree guides the user through a series of phases in the design process. Figure 2.1 shows Tree 1, which outlines the process for defining the existing system parameters, followed by optimizing the existing treatment system operation. If optimizing the existing system does not correct the deficiency, the tree leads to six alternative preliminary branches for investigation. The groundwater branch leads through investigating existing wells to developing a new well field. The treatment alternatives address centralized and on-site treatment. The objective of this phase is to develop conceptual designs and cost estimates for the six types of alternatives. The work done for this report follows through Tree 1 and Tree 2, as well as a preliminary pass through Tree 4.

Tree 3, which begins at the conclusion of the work for this report, starts with a comparison of the conceptual designs, selecting the two or three alternatives that appear to be most promising, and eliminating those alternatives which are obviously infeasible. It is envisaged that a process similar to this would be used by the study PWS to refine the list of viable alternatives. The selected alternatives are then subjected to intensive investigation, and highlighted by an investigation into the socio-political aspects of implementation. Designs are further refined and compared, resulting in the selection of a preferred alternative. The steps for assessing the financial and economic aspects of the alternatives (one of the steps in Tree 3) are given in Tree 4 in Figure 2.4.

2.2 DATA SOURCES AND DATA COLLECTION

2.2.1 Data Search

2.2.1.1 Water Supply Systems

The TCEQ maintains a set of files on PWSs, utilities, and districts at its headquarters in Austin, Texas. The files are organized under two identifiers: a PWS identification number and a CCN number. The PWS identification number is used to retrieve four types of files:

- CO – Correspondence,
- CA – Chemical analysis,
- MOR – Monthly operating reports (quality/quantity), and
- FMT – Financial, managerial and technical issues.

Figure 2.1
TREE 1 – EXISTING FACILITY ANALYSIS

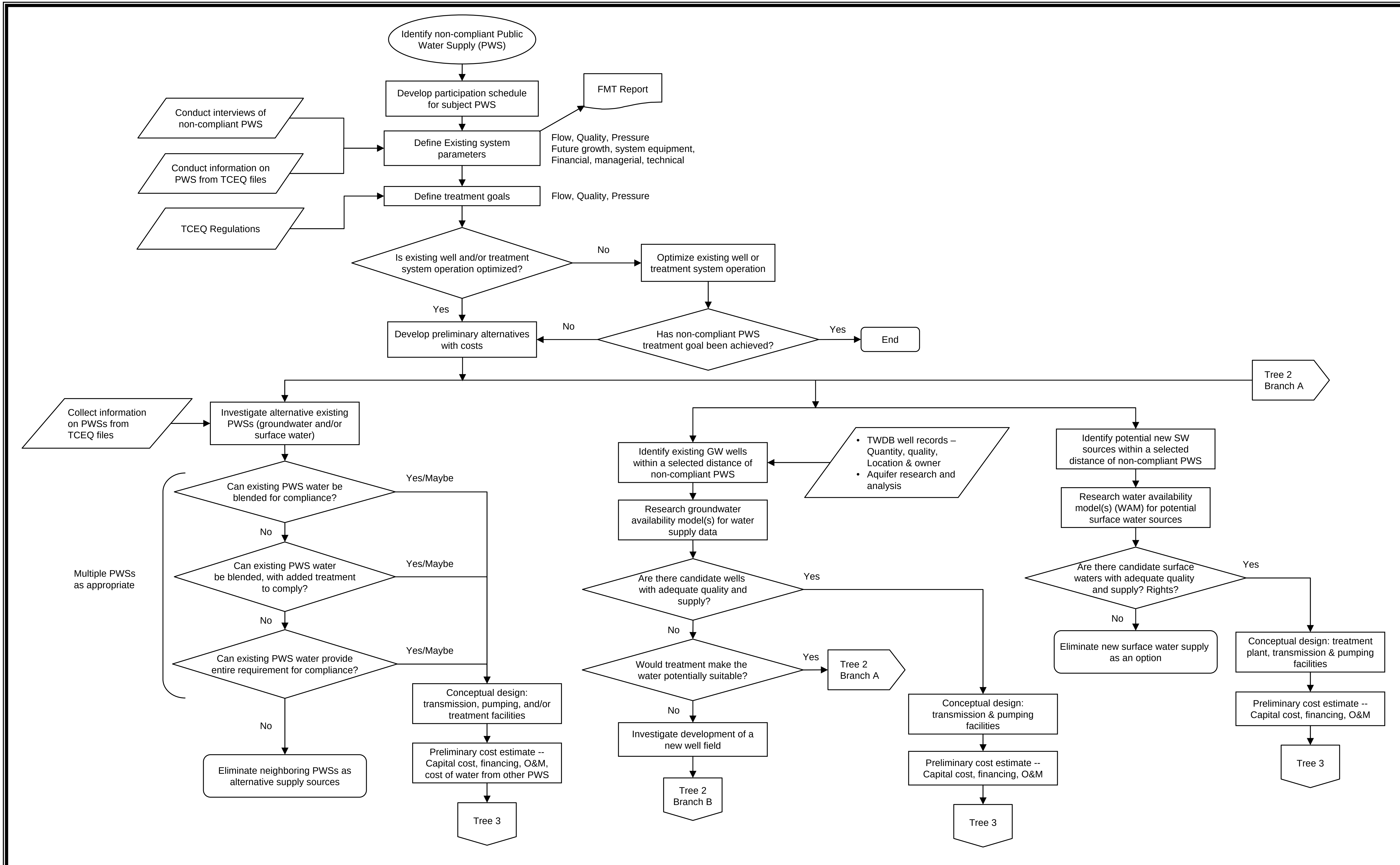


Figure 2.2
TREE 2 – DEVELOP TREATMENT ALTERNATIVES

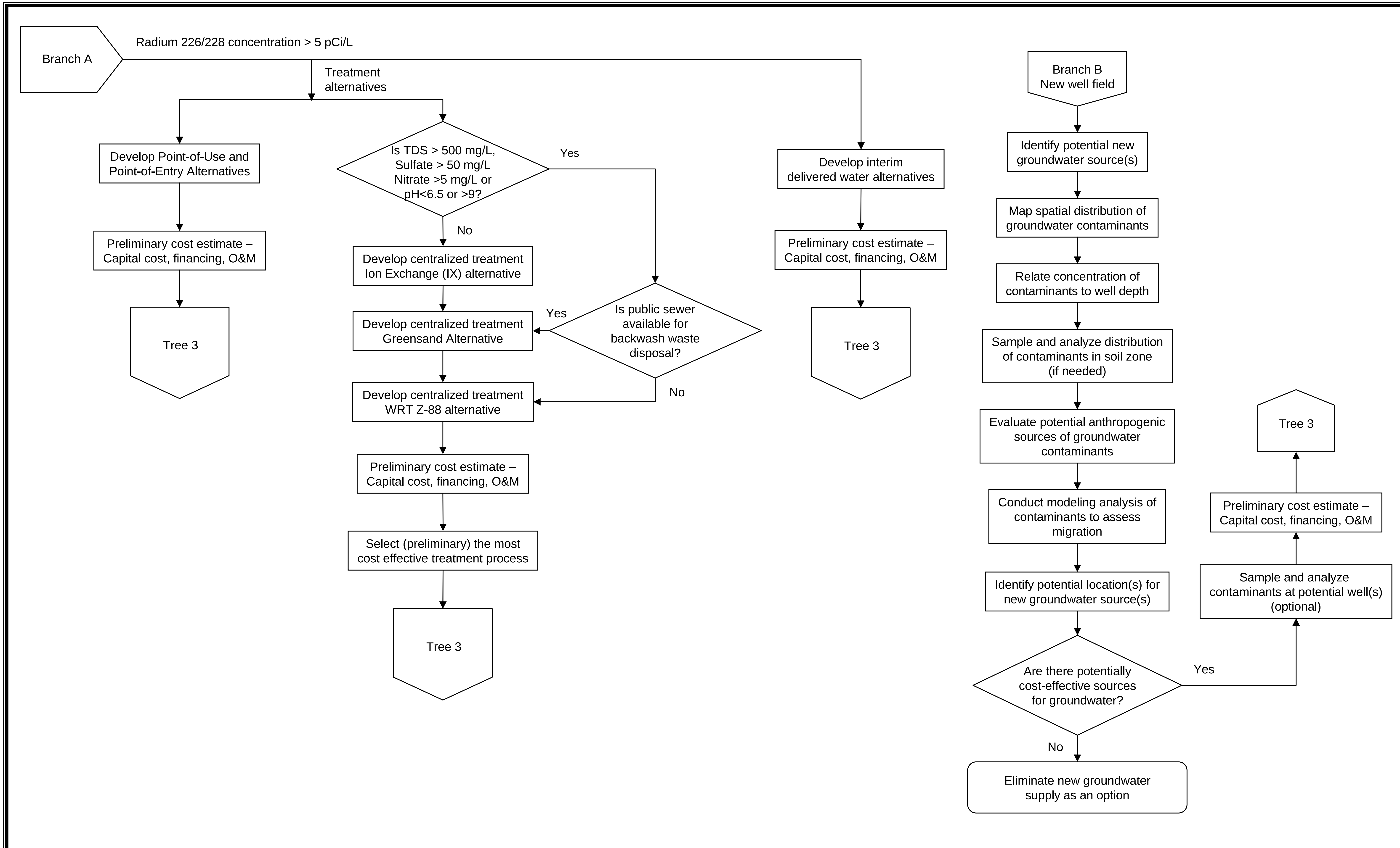


Figure 2.3

Tree 3 – PRELIMINARY ANALYSIS

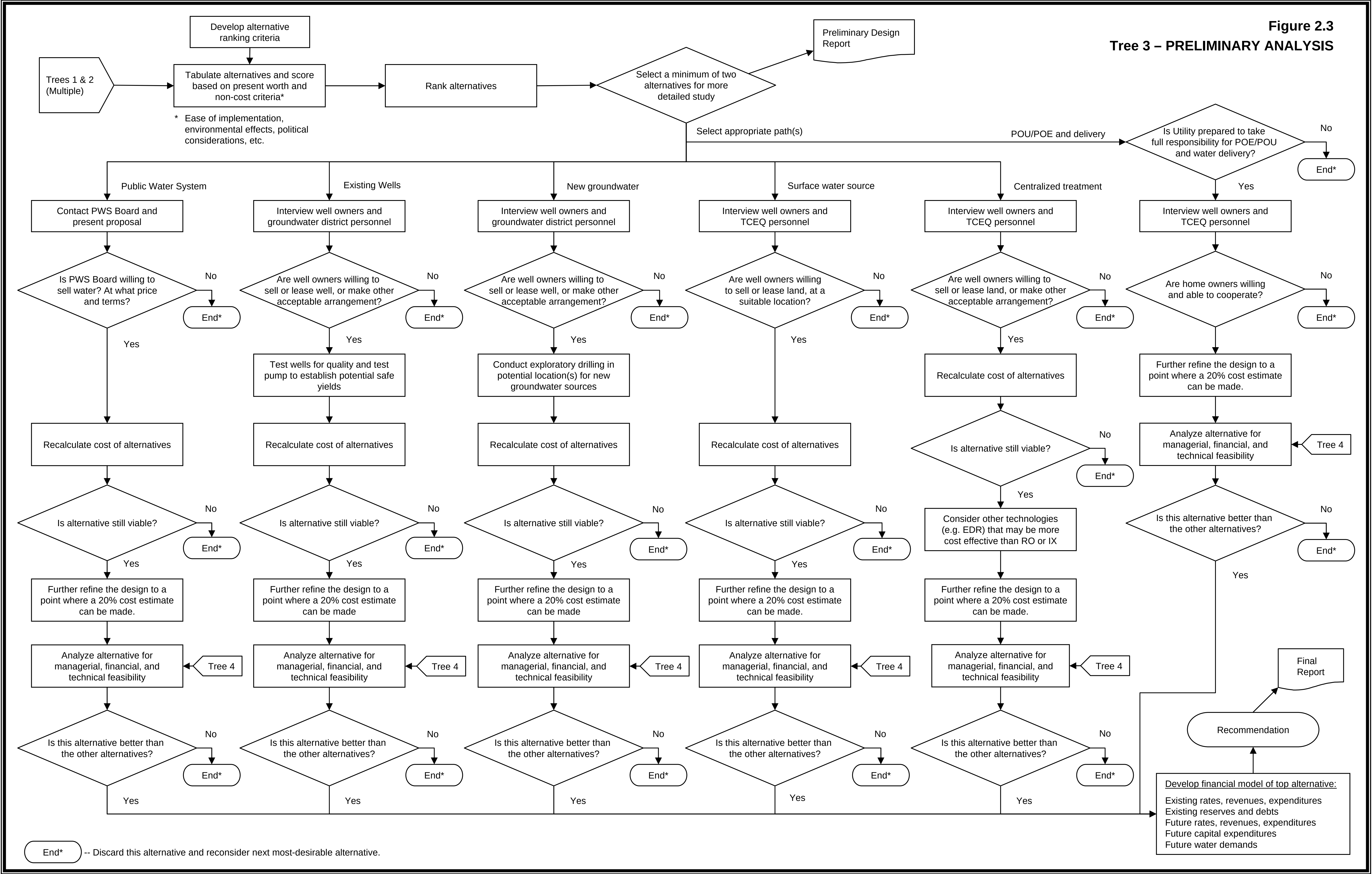
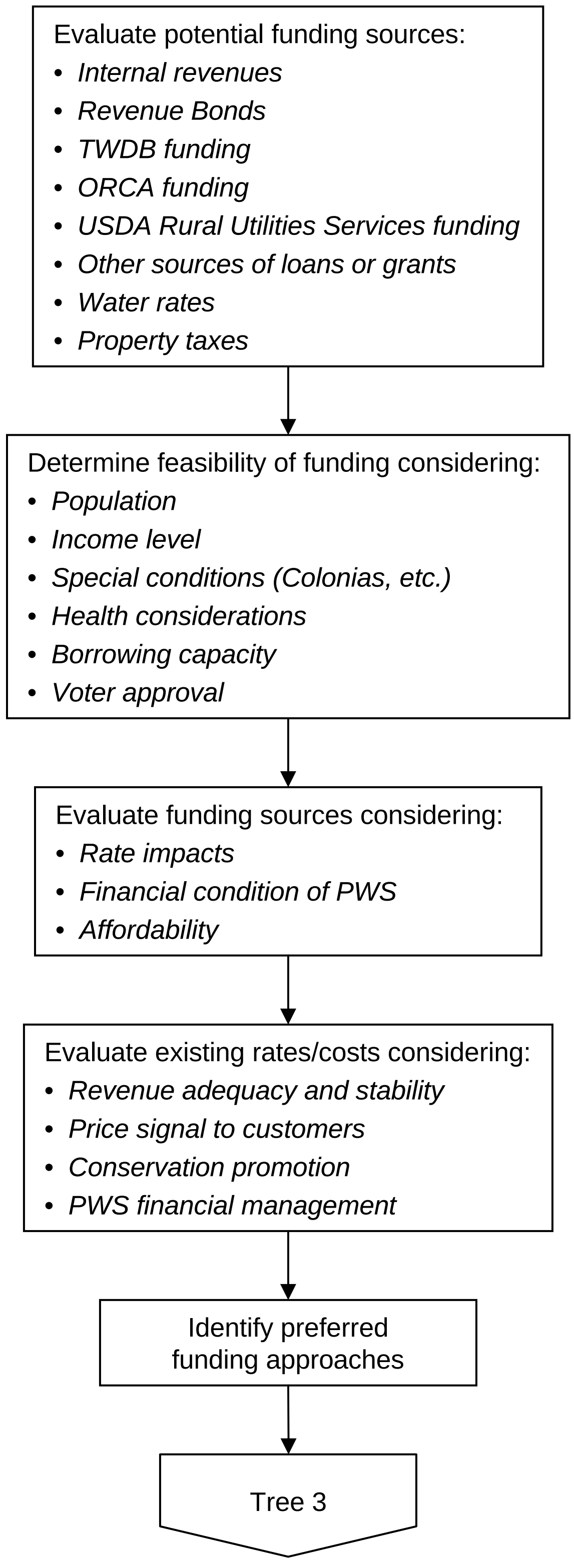


Figure 2.4
TREE 4 – FINANCIAL



The CCN files generally contain a copy of the system's Certificate of Convenience and Necessity, along with maps and other technical data.

These files were reviewed for the PWS and surrounding systems.

The following websites were consulted to identify the water supply systems in the study area:

- Texas Commission on Environmental Quality
  www3.tnrcc.state.tx.us/iwud/pws/index.cfm? Under "Advanced Search", type in the name(s) of the county(ies) in the study area to get a listing of the public water supply systems.
- USEPA Safe Drinking Water Information System
  www.epa.gov/safewater/data/getdata.html

Groundwater Control Districts were identified on the TWDB web site, which has a series of maps covering various groundwater and surface water subjects. One of those maps shows groundwater control districts in the State of Texas.

2.2.1.2 Existing Wells

The TWDB maintains a groundwater database available at www.twdb.state.tx.us that has two tables with helpful information. The "Well Data Table" provides a physical description of the well, owner, location in terms of latitude and longitude, current use, and for some wells, items such as flowrate, and nature of the surrounding formation. The "Water Quality Table" provides information on the aquifer and the various chemical concentrations in the water.

2.2.1.3 Surface Water Sources

Regional planning documents were consulted for lists of surface water sources.

2.2.1.4 Groundwater Availability Model

GAMs, developed by the TWDB, are planning tools and should be consulted as part of a search for new or supplementary water sources. The GAM for the Gulf Coast aquifer (northern part) which includes the Evangeline and Jasper Aquifers, was investigated as a potential tool for identifying available and suitable groundwater resources.

2.2.1.5 Water Availability Model

The WAM is a computer-based simulation predicting the amount of water that would be in a river or stream under a specified set of conditions. WAMs are used to determine whether water would be available for a newly requested water right or amendment. If water is available, these models estimate how often the applicant could count on water under various conditions (*e.g.*, whether water would be available only 1 month out of the year, half the year, or all year, and whether that water would be available in a repeat of the drought of record).

WAMs provide information that assist TCEQ staff in determining whether to recommend the granting or denial of an application.

2.2.1.6 Financial Data

Financial data were collected through a site visit. Data sought included:

- Annual Budget
- Audited Financial Statements
    - Balance Sheet
    - Income & Expense Statement
    - Cash Flow Statement
    - Debt Schedule
- Water Rate Structure
- Water Use Data
    - Production
    - Billing
    - Customer Counts

2.2.1.7 Demographic Data

Basic demographic data were collected from the 2000 Census to establish incomes and eligibility for potential low cost funding for capital improvements. Median household income (MHI) and number of families below poverty level were the primary data points of significance. If available, MHI for the customers of the PWS should be used. In addition, unemployment data were collected from current U.S. Bureau of Labor Statistics. These data were collected for the following levels: national, state, and county.

2.2.2 PWS Interviews

2.2.2.1 PWS Capacity Assessment Process

A capacity assessment is the industry standard term for an evaluation of a PWS's FMT capacity to effectively deliver safe drinking water to its customers now and in the future at a reasonable cost, and to achieve, maintain and plan for compliance with applicable regulations. The assessment process involves interviews with staff and management who have a responsibility in the operations and management of the system.

Financial, managerial, and technical capacity are individual yet highly interrelated components of a system's capacity. A system cannot sustain capacity without maintaining adequate capability in all three components.

Financial capacity is a PWS's ability to acquire and manage sufficient financial resources to allow the system to achieve and maintain compliance with SDWA regulations. Financial capacity refers to the financial resources of the PWS, including but not limited to revenue sufficiency, credit worthiness, and fiscal controls.

Managerial capacity is the ability of a water system to conduct its affairs so the system is able to achieve and maintain compliance with SDWA requirements. Managerial capacity refers to the management structure of the water system, including but not limited to ownership accountability, staffing and organization, and effective relationships to customers and regulatory agencies.

Technical capacity is the physical and operational ability of a water system to achieve and maintain compliance with the SDWA regulations. It refers to the physical infrastructure of the PWS, including the adequacy of the source water, treatment, storage and distribution infrastructure. It also refers to the ability of system personnel to effectively operate and maintain the system and to otherwise implement essential technical knowledge.

Many aspects of PWS operations involve more than one component of capacity. Infrastructure replacement or improvement, for example, requires financial resources, management planning and oversight, and technical knowledge. A deficiency in any one area could disrupt the entire effort. A system that is able to meet both its immediate and long-term challenges demonstrates that it has sufficient FMT capacity.

Assessment of the FMT capacity of the PWS was based on an approach developed by the New Mexico Environmental Finance Center (NMEFC), which is consistent with TCEQ FMT assessment process. This method was developed from work the NMEFC did while assisting USEPA Region 6 in developing and piloting groundwater comprehensive performance evaluations. The NMEFC developed a standard list of questions that could be asked of PWS personnel. The list was then tailored slightly to have two sets of questions – one for managerial and financial personnel, and one for operations personnel (the questions are included in Appendix A). Each person with a role in the FMT capacity of the system was asked the applicable standard set of questions individually. The interviewees were not given the questions in advance and were not told the answers others provided. Also, most of the questions are open ended type questions so they were not asked in a fashion to indicate what would be the "right" or "wrong" answer. The interviews lasted between 45 minutes to 75 minutes depending on the individual's role in the system and the length of the individual's answers.

In addition to the interview process, visual observations of the physical components of the system were made. A technical information form was created to capture this information. This form is also contained in Appendix A. This information was considered supplemental to the interviews because it served as a check on information provided in the interviews. For example, if an interviewee stated he or she had an excellent preventative maintenance schedule and the visit to the facility indicated a significant amount of deterioration (more than would be expected for the age of the facility) then the preventative maintenance program could be further investigated or the assessor could decide that the preventative maintenance program was inadequate.

Following interviews and observations of the facility, answers that all personnel provided were compared and contrasted to provide a clearer picture of the true operations at the water system. The intent was to go beyond simply asking the question, "Do you have a budget?" to actually finding out if the budget was developed and being used appropriately. For example, if a water system manager was asked the question, "Do you have a budget?" he or she may say, "yes" and the capacity assessor would be left with the impression that the system is doing well in this area. However, if several different people are asked about the budget in more detail, the assessor may find that although a budget is present, operations personnel do not have input into the budget, the budget is not used by the financial personnel, the budget is not updated regularly, or the budget is not used in setting or evaluating rates. With this approach, the inadequacy of the budget would be discovered and the capacity deficiency in this area would be noted.

Following the comparison of answers, the next step was to determine which items noted as a potential deficiency truly had a negative effect on the system's operations. If a system had what appeared to be a deficiency, but this deficiency was not creating a problem in terms of the operations or management of the system, it was not considered critical and may not have needed to be addressed as a high priority. As an example, the assessment may have revealed an insufficient number of staff members to operate the facility. However, it may also have been revealed that the system was able to work around that problem by receiving assistance from a neighboring system, so no severe problems resulted from the number of staff members. Although staffing may not be ideal, the system does not need to focus on this particular issue. The system needs to focus on items that are truly affecting operations. As an example of this type of deficiency, a system may lack a reserve account which can then lead the system to delay much-needed maintenance or repair on its storage tank. In this case, the system needs to address the reserve account issue so that proper maintenance can be completed.

The intent was to develop a list of capacity deficiencies with the greatest impact on the system's overall capacity. Those were the most critical items to address through follow-up technical assistance or by the system itself.

2.2.2.2 Interview Process

PWS personnel were interviewed by the project team, and each was interviewed separately. Interview forms were completed during each interview.

2.3 ALTERNATIVE DEVELOPMENT AND ANALYSIS

The initial objective for developing alternatives to address compliance issues is to identify a comprehensive range of possible options that can be evaluated to determine which are the most promising for implementation. Once the possible alternatives are identified, they must be defined in sufficient detail so a conceptual cost estimate (capital and O&M costs) can be developed. These conceptual cost estimates are used to compare the affordability of compliance alternatives, and to give a preliminary indication of rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. The basis for the unit costs used for the compliance alternative cost estimates is summarized in Appendix B. Other non-economic factors for the alternatives, such as reliability and ease of implementation, are also addressed.

2.3.1 Existing PWS

The neighboring PWSs were identified, and the extents of their systems were investigated. PWSs farther than 10 miles from the non-compliant PWSs were not considered because the length of the pipeline required would make the alternative cost prohibitive. The quality of water provided was also investigated. For neighboring PWSs with compliant water, options for water purchase and/or expansion of existing well fields were considered. The neighboring PWSs with non-compliant water were considered as possible partners in sharing the cost for obtaining compliant water either through treatment or developing an alternate source.

The neighboring PWSs were investigated to get an idea of the water sources in use and the quantity of water that might be available for sale. They were contacted to identify key locations in their systems where a connection might be made to obtain water, and to explore on a preliminary basis their willingness to partner or sell water. Then, the major system components that would be required to provide compliant water were identified. The major system components included treatment units, wells, storage tanks, pump stations, and pipelines.

Once the major components were identified, a preliminary design was developed to identify sizing requirements and routings. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.2 New Groundwater Source

It was not possible in the scope of this study to determine conclusively whether new wells could be installed to provide compliant drinking water. To evaluate potential new groundwater source alternatives, three test cases were developed based on distance from the PWS intake point. The test cases were based on distances of 10 miles, 5 miles, and 1 mile. It was assumed that a pipeline would be required for all three test cases, and a storage tank and pump station would be required for the 10-mile and 5-mile alternatives. It was also assumed that new wells would be installed, and that their depths would be similar to the depths of the existing wells, or other existing drinking water wells in the area.

A preliminary design was developed to identify sizing requirements for the required system components. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change (*i.e*., from current expenditures) in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were; whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.3 New Surface Water Source

New surface water sources were investigated. Availability of adequate quality water was investigated for the main rivers in the study area, as well as the major reservoirs. TCEQ WAMs were inspected, and the WAM was run, where appropriate.

2.3.4 Treatment

Treatment technologies considered potentially applicable to radium and gross alpha particle removal are IX, WRT Z-88™ media, RO, EDR, and $KMnO_4$-greensand filtration. RO and EDR are membrane processes that produce a considerable amount of liquid waste: a reject stream from RO treatment and a concentrate stream from EDR treatment. As a result, the treated volume of water is less than the volume of raw water that enters the treatment system. The amount of raw water used increases to produce the same amount of treated water if RO or EDR treatment is implemented. Because the TDS is not high the use of RO or EDR would be considerably more expensive than the other potential technologies. Hence, RO and EDR are not considered further. However, RO is considered for POU and POE alternatives. IX, WRT Z-88™ media, and $KMnO_4$-greensand filtration are considered as alternative central treatment technologies. The treatment units were sized based on flow rates, and capital and annual O&M cost estimates were made based on the size of the treatment equipment required. Neighboring non-compliant PWS's were identified to look for opportunities where the costs and benefits of central treatment could be shared between systems.

Non-economic factors were also identified. Ease of implementation was considered, as well as reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.4 COST OF SERVICE AND FUNDING ANALYSIS

The primary purpose of the cost of service and funding analysis is to determine the financial impact of implementing compliance alternatives, primarily by examining the required rate increases, and also the fraction of household income that water bills represent. The current financial situation is also reviewed to determine what rate increases are necessary for the PWS to achieve or maintain financial viability.

2.4.1 Financial Feasibility

A key financial metric is the comparison of average annual household water bill for a PWS customer to the MHI for the area. MHI data from the 2000 Census are used, at the most detailed level available for the community. Typically, county level data are used for small rural water utilities due to small population sizes. Annual water bills are determined for existing, base conditions, including consideration of additional rate increases needed under current conditions. Annual water bills are also calculated after adding incremental capital and operating costs for each of the alternatives to determine feasibility under several potential funding sources.

Additionally, the use of standard ratios provides insight into the financial condition of any business. Three ratios are particularly significant for water utilities:

- Current Ratio = current assets divided by current liabilities provides insight into the ability to meet short-term payments. For a healthy utility, the value should be greater than 1.0.
- Debt to Net Worth Ratio = total debt divided by net worth shows to what degree assets of the company have been funded through borrowing. A lower ratio indicates a healthier condition.
- Operating Ratio = total operating revenues divided by total operating expenses show the degree to which revenues cover ongoing expenses. The value is greater than 1.0 if the utility is covering its expenses.

2.4.2 Median Household Income

The 2000 U.S. Census is used as the basis for MHI. In addition to consideration of affordability, the annual MHI may also be an important factor for sources of funds for capital programs needed to resolve water quality issues. Many grant and loan programs are available to lower income rural areas, based on comparisons of local income to statewide incomes. In the 2000 Census, MHI for the State of Texas was $39,927, compared to the U.S. level of $41,994.

2.4.3 Annual Average Water Bill

The annual average household water bill was calculated for existing conditions and for future conditions incorporating the alternative solutions. Average residential consumption is estimated and applied to the existing rate structure to estimate the annual water bill. The estimates are generated from a long-term financial planning model that details annual revenue, expenditure, and cash reserve requirements over a 30-year period.

2.4.4 Financial Plan Development

The financial planning model uses available data to establish base conditions under which the system operates. The model includes, as available:

- Accounts and consumption data
- Water tariff structure
- Beginning available cash balance
- Sources of receipts:
  - Customer billings
  - Membership fees
  - Capital Funding receipts from:
    - Grants
    - Proceeds from borrowing
- Operating expenditures:
  - Water purchases
  - Utilities
  - Administrative costs
  - Salaries
- Capital expenditures
- Debt service:
  - Existing principal and interest payments
  - Future principal and interest necessary to fund viable operations
- Net cash flow
- Restricted or desired cash balances:
  - Working capital reserve (based on 1-4 months of operating expenses)
  - Replacement reserves to provide funding for planned and unplanned repairs and replacements

From the model, changes in water rates are determined for existing conditions and for implementing the compliance alternatives.

2.4.5 Financial Plan Results

Results from the financial planning model are summarized in two areas: percentage of household income and total water rate increase necessary to implement the alternatives and maintain financial viability.

2.4.5.1 Funding Options

Results are summarized in a table that shows the following according to alternative and funding source:

- Percentage of the annual MHI the average annual residential water bill represents.
- The first year in which a water rate increase would be required
- The total increase in water rates required, compared to current rates

Water rates resulting from the incremental capital costs of the alternative solutions are examined under a number of funding options. The first alternative examined is always funding from existing reserves plus future rate increases. Several funding options were analyzed to frame a range of possible outcomes.

- Grant funds for 100 percent of required capital. In this case, the PWS is only responsible for the associated O&M costs.
- Grant funds for 75 percent of required capital, with the balance treated as if revenue bond funded.
- Grant funds for 50 percent of required capital, with the balance treated as if revenue bond funded.
- State revolving fund loan at the most favorable available rates and terms applicable to the communities.
    - If local MHI >75 percent of state MHI, standard terms, currently at 3.8 percent interest for non-rated entities. Additionally:
    - If local MHI = 70-75 percent of state MHI, 1 percent interest rate on loan.
    - If local MHI = 60-70 percent of state MHI, 0 percent interest rate on loan.
    - If local MHI = 50-60 percent of state MHI, 0 percent interest and 15 percent forgiveness of principal.
    - If local MHI less than 50 percent of state MHI, 0 percent interest and 35 percent forgiveness of principal.
- Terms of revenue bonds assumed to be 25-year term at 6.0 percent interest rate.

2.4.5.2 General Assumptions Embodied in Financial Plan Results

The basis used to project future financial performance for the financial plan model includes:

- No account growth (either positive or negative).
- No change in estimate of uncollectible revenues over time.
- Average consumption per account unchanged over time.
- No change in unaccounted for water as percentage of total (more efficient water use would lower total water requirements and costs).
- No inflation included in the analyses (although the model has provisions to add escalation of O&M costs, doing so would mix water rate impacts from inflation with the impacts from the alternatives being examined).
- Minimum working capital fund established for each district, based on specified months of O&M expenditures.
- O&M for alternatives begins 1 year after capital implementation.
- Balance of capital expenditures not funded from primary grant program is funded through debt (bond equivalent).
- Cash balance drives rate increases, unless provision chosen to override where current net cash flow is positive.

2.4.5.3 Interpretation of Financial Plan Results

Results from the financial plan model for each alternative are presented in Table 4.4 in Section 4 of this report. The model used six funding alternatives: paying cash up front (all revenue); 100 percent grant; 75 percent grant; 50 percent grant, State Revolving Fund (SRF); and obtaining a Loan/Bond. Table 4.4 shows the projected average annual water bill, the maximum percent of household income, and the percentage rate increase over current rates.

2.4.5.4 Potential Funding Sources

A number of potential funding sources exist for rural utilities. Both state and federal agencies offer grant and loan programs to assist rural communities in meeting their infrastructure needs.

Within Texas, the following state agencies offer financial assistance if needed:

- Texas Water Development Board,
- Office of Rural Community Affairs, and
- Texas Department of Health (Texas Small Towns Environment Program).

Small rural communities can also get assistance from the federal government. The primary agencies providing aid are:

- United States Department of Agriculture, Rural Utilities Service, and
- United States Housing and Urban Development.

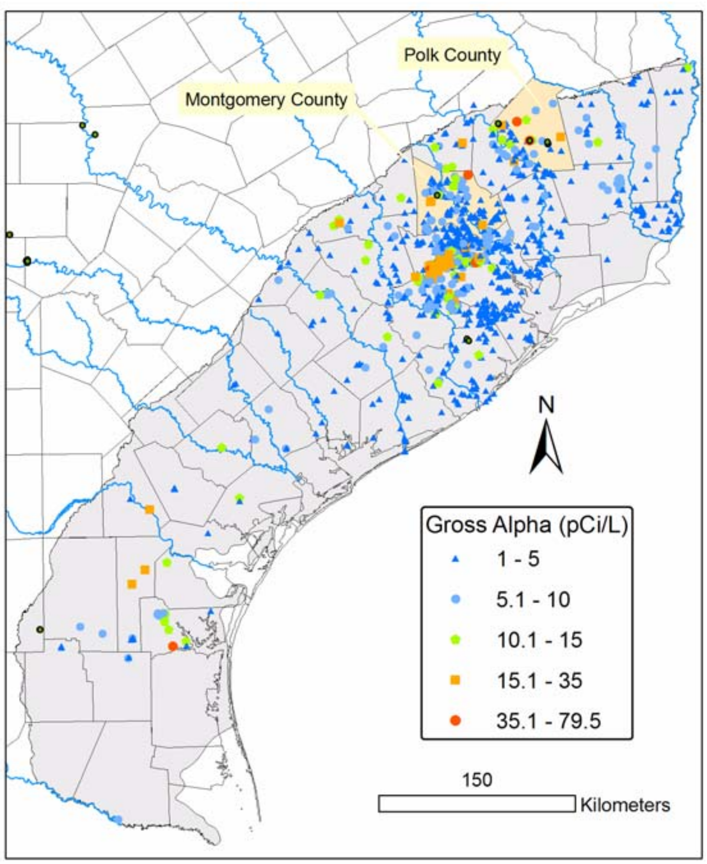
SECTION 3
UNDERSTANDING SOURCES OF CONTAMINANTS

3.1 GROSS ALPHA AND RADIUM IN THE GULF COAST AQUIFER

The Gulf Coast aquifer parallels the Texas Gulf Coast and extends from the Texas-Louisiana border to the Rio Grande. Subunits of the Gulf Coast aquifer are from oldest to youngest, the Jasper, Evangeline, and Chicot aquifers. The aquifer is a leaky artesian system composed of middle to late Tertiary and younger interbedded and hydrologically connected layers of clay, silt, sand, and gravel (Baker 1979, Ashworth and Hopkins 1992). Most PWS wells of concern in Polk and Montgomery Counties are completed in the Jasper aquifer.

The most recent gross alpha data from the TCEQ database (contaminants ID 4109 - gross alpha particle activity) were plotted to assess the spatial distribution of alpha radiation in the aquifer (Figure 3.1). Only one well with gross alpha was found for this aquifer in the TWDB database (storet code 80045), therefore this data wasn't included in the analysis.

Figure 3.1 Gross Alpha in Groundwater of the Gulf Coast Aquifer (TCEQ Database, 1095 Data Points from 2001 to 2005)



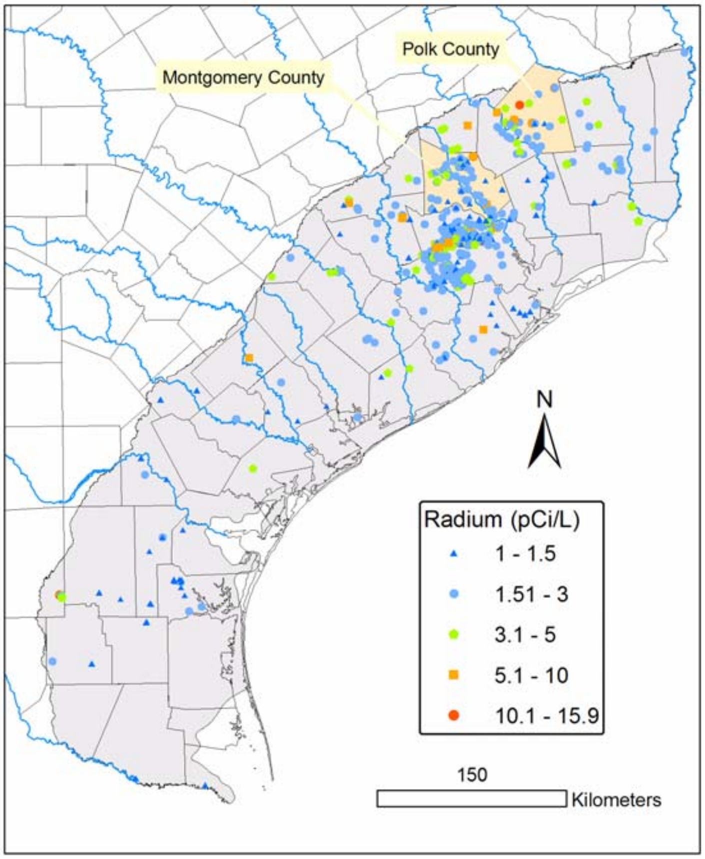
Uranium concentrations were evaluated only in wells where gross alpha exceeds 15 pCi/L. The MCL for uranium is 30 micrograms per liter (µg/L) which is equivalent to 20 pCi/L (using a conservative factor of 0.67 pCi/µg for converting mass concentration to radiation concentration). Therefore a gross alpha level of 35 pCi/L in a well reflects a level from which the well fails to comply with either the MCL for gross alpha minus alpha radiation due to uranium which is 15 pCi/L, or with the uranium MCL (neglecting the activity due to radon which is rarely measured in PWS wells). Gross alpha >5 pCi/L requires analysis of radium-226. Radium-228 testing must be done regardless of gross alpha results (TCEQ 2004b). The symbology for gross alpha levels in Figure 3.1 takes these threshold levels into account.

Relatively high gross alpha levels are common in Polk and Harris Counties and to a lesser extent in Montgomery and Walker Counties. High levels of gross alpha are found also in the southern part of the aquifer (Jim Wells and Kleberg Counties).

The most recent radium measurements from the TWDB and TCEQ databases were analyzed to assess the overall occurrence of this contaminant in the aquifer (Figure 3.2). In this study the terms *radium* or *radium combined* are generally used to refer to radium-226 + radium-228. Otherwise, radium-226 or radium-228 is specified. The values shown in Figure 3.2 generally represent the upper limit of the radium measurements, because radium-228 was below its detection limit of 1 pCi/L for more than 75 percent of the data, and the detection limit was used when summing with radium-226 for the radium combined values. Radium-228 can have negative values in the TWDB database when radiation of the sample is lower than background radiation at the laboratory, in these cases zero was used for the sum. Although TCEQ allows PWSs to subtract the reported error from the radium concentrations to assess compliance, the following analysis of general trends used the most recent radium concentration and did not subtract the reported error. This approach is considered more conservative.

The most recent values for wells from which both isotopes of radium were analyzed are shown in Figure 3.2 (number of samples shown is 526; 432 from TCEQ database and 94 from TWDB database). The codes for the contaminants are: TWDB - Storet code 09503 and 81366; and TCEQ databases - Contaminant ID 4020 and 4030, for radium-226 and radium-228, respectively. Radium-226 and radium-228 were combined and the combined value for each well is shown. Only measurements from a single entry point that can be related to a specific well were used from the TCEQ database.

Figure 3.2 Radium in Groundwater of the Gulf Coast Aquifer (TCEQ Database, Data from 1998 to 2005, and TWDB Database, Data from 1988 to 1990)

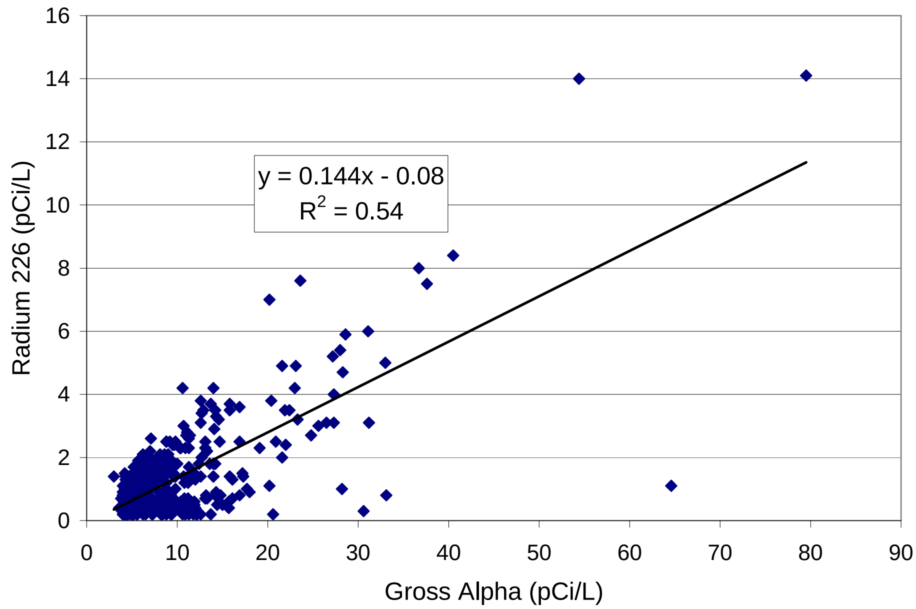


Radium levels exceeding the 5 pCi/L MCL seem more likely to be found in the central to northern parts of the aquifer; however, this distribution may be an artifact of the higher density of measurements toward the northern part of the aquifer (Figure 3.2). Relatively high levels of radium are found in the area of Polk County and the neighboring counties to the west (San Jacinto, Walker, and Montgomery Counties) in wells open to the Jasper aquifer.

3.1.1 Gross Alpha and Radium Trends

Gross alpha and radium trends were calculated with data from the TCEQ PWS database (Figures 3.3, 3.4, and 3.5). Only the most recent analyses with both parameters analyzed from a single entry point that can be related to a specific well are included in the analysis.

Figure 3.3 Radium-226 vs. Gross Alpha in Groundwater of the Gulf Coast Aquifer (TCEQ Database from 2001 to 2005, 434 Samples)



The average contribution of radium-226 to the Gross alpha count is 14.4 percent (based on the slope in Figure 3.3). All samples of radium-226 >4 pCi/L are above the regression line, which means that in wells with high levels of radium the contribution of radium to gross alpha counts is higher (~15-20%). In five out of six wells in which gross alpha is >35 pCi/L radium-226 >7 pCi/L. Therefore, non compliance with radium MCL is strongly related with non compliance with gross alpha MCL in the Gulf Coast aquifer.

Gross alpha and radium are highest in the Jasper aquifer, while the Evangeline and Chicot aquifers have radium exceeding MCL in only in 3.8 and 1.7 percent of its wells, respectively (Table 3.1). Gross alpha levels are relatively high both in the Jasper and the Evangeline aquifers and low in the Chicot aquifer. Higher levels of gross alpha (>35 pCi/L) are more frequently found in the Jasper aquifer (specifically in Polk County), whereas gross alpha levels in the Evangeline aquifer are more commonly in the medium (>5 pCi/L) and high (>15 pCi/L) levels (Table 3.1).

Table 3.1 Distribution of Gross Alpha and Radium within the Gulf Coast Aquifers (Most Recent Data for Wells in the TCEQ Database)

	Radium				Gross Alpha				
Aquifer	Number of wells with radium samples	Average radium (pCi/L)	Median radium (pCi/L)	% of wells with radium >5 pCi/L	Wells with gross alpha samples	Median gross alpha (PCi/L)	% of wells with gross alpha >5 pCi/L	% of wells with gross alpha > 15 pCi/L	% of wells with gross alpha >35 pCi/L
Chicot	121	1.7	1.4	1.7	406	<2	22.4	1.2	0.2
Evangeline	261	1.9	1.6	3.8	573	3.5	36.8	7.0	0.5
Jasper	49	3.2	2.6	10.2	142	2.5	30.3	4.9	1.4

Samples of radium-228 with concentrations equal to the detection limit of 1 pCi/L were assigned a value of 0.5 in the calculation of combined radium.

No correlation between radium and well depth was found for the combined three aquifers (Figure 3.4) nor when separately plotted (not shown). Correlation between gross alpha and well depth (plot not shown) is slightly higher ($R^2 = 0.019$) but still low. Correlations of radium with general water quality parameters such as chloride and TDS are very small as well (Figure 3.5).

Figure 3.4 Radium Concentrations vs. Well Depth (434 Wells in the Chicot, Evangeline, and Jasper Aquifers)

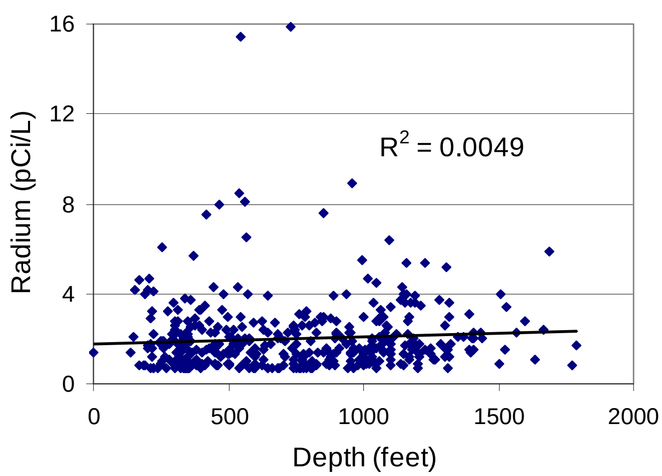
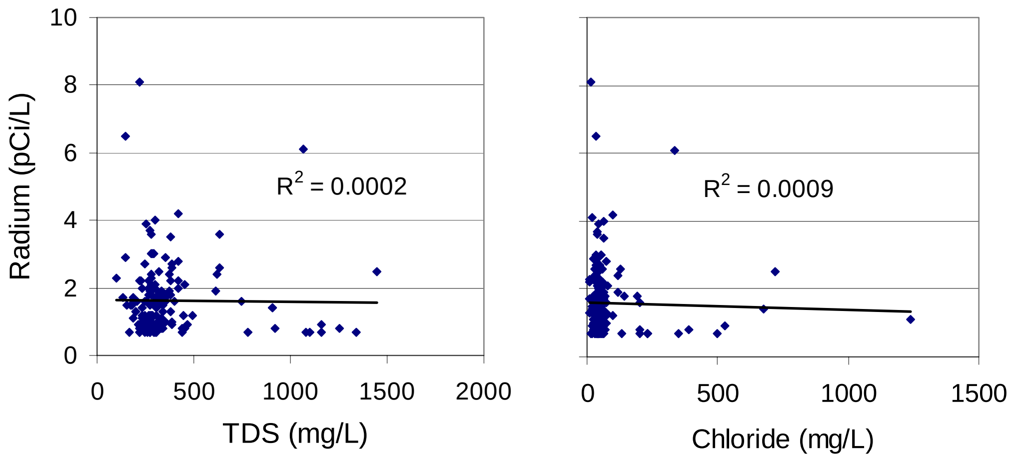


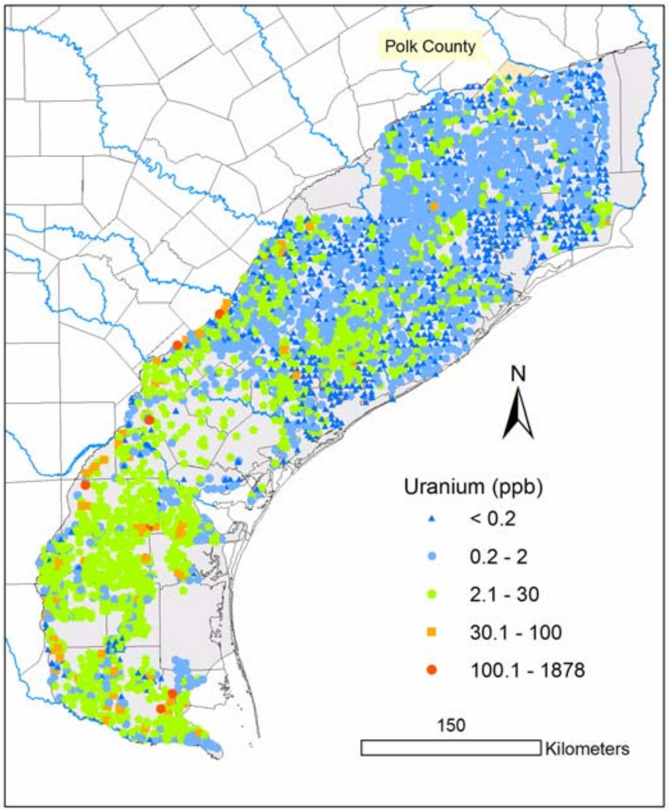
Figure 3.5 Relationship between Radium and Chloride Concentrations (186 Wells) and Radium and TDS Concentrations (163 Wells) in the Chicot, Evangeline, and Jasper Aquifers



3.1.2 Uranium in the Gulf Coast Aquifer

The National Uranium Resource Evaluation (NURE) database contains many uranium analyses from the Gulf Coast aquifer; therefore, it was used to assess the spatial distribution of uranium at the basin scale. The southern part of the aquifer has higher uranium levels than the northern part (Figure 3.6). A narrow strip of high uranium concentrations is found near the northwestern boundary of the aquifer where wells are open to the Jasper aquifer. High levels of uranium in the south and along the Jasper aquifer correspond to high levels of arsenic in these regions also. Another area with relatively high uranium levels is between the Colorado and San Antonio rivers (Wharton, Jackson, and Victoria Counties). Most wells in this area obtain water from the Chicot aquifer. Water from wells in Polk County do not exceed the uranium MCL of 30 µg/L. The relatively high gross alpha found in this county (Figure 3.1) is probably not related to uranium, but this is discussed in more detail in Subsection 3.3 where individual wells are evaluated.

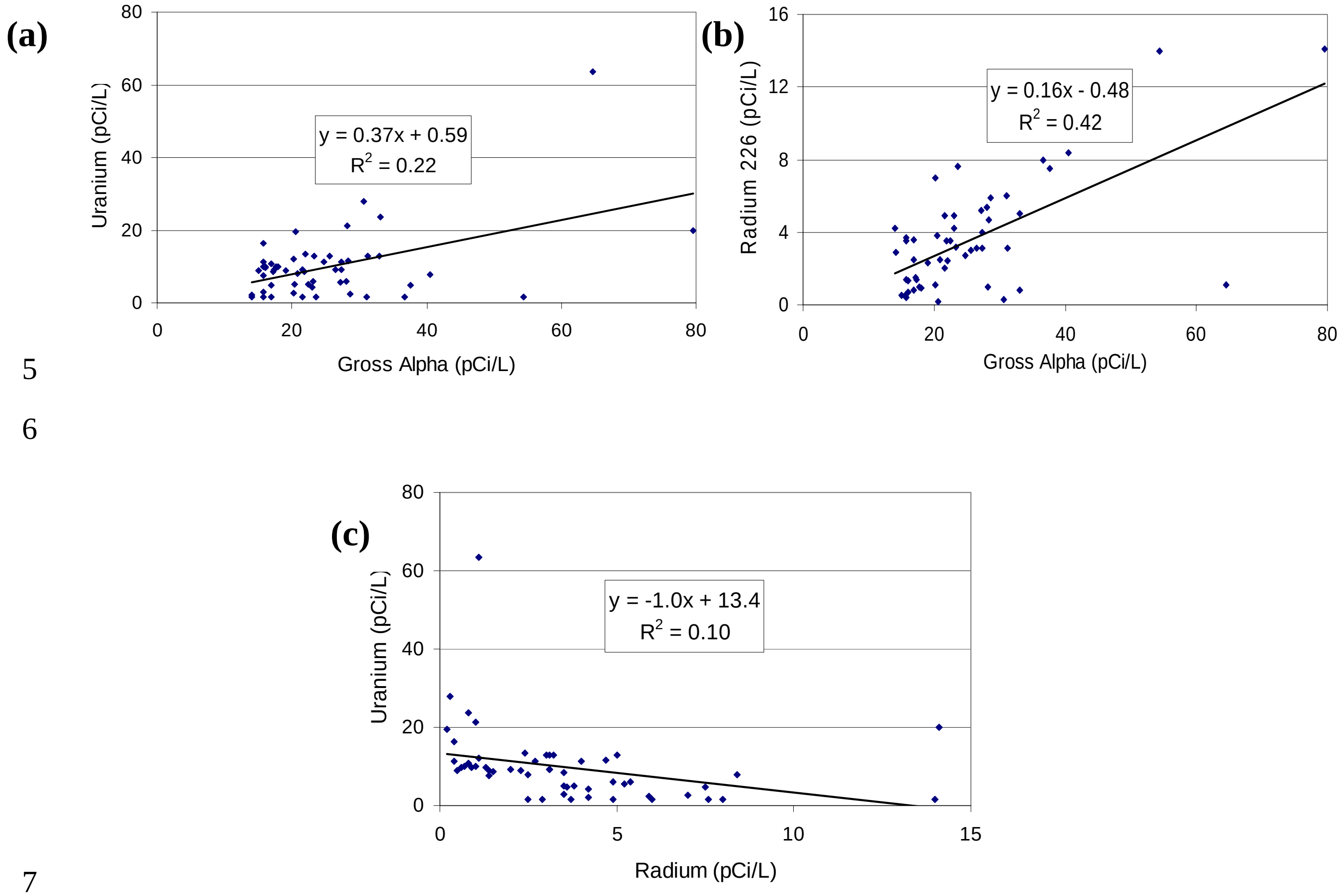
Figure 3.6 Uranium Concentrations in Groundwater of the Gulf Coast Aquifer



Note: (NURE database, analyses from 1976 to 1980). In *the NURE database there is one sample per well (number of samples shown is 2802).*

The TCEQ database contains only 62 single well source measurements of uranium in the Gulf Coast aquifer (Uranium 234, Uranium 235, and Uranium 238 are measured separately). Uranium in pCi/L is referred to as total uranium (*i.e.*, the sum of the three isotopes). A total of 55 out of these 62 most recent samples that have measurements of radium-226 and gross alpha in the same sample were used in Figure 3.7 to describe the relationship between uranium, radium, and gross alpha.

Figure 3.7 Relationships Between Uranium, Radium-226, and Gross Alpha in Groundwater of the Gulf Coast Aquifer (Data from the TCEQ Database from 2001 to 2005, Total of 55 Samples)



The correlation between uranium and gross alpha concentrations (Figure 3.7a) is not as strong as the correlation of gross alpha and radium (Figure 3.7b). Uranium contributes about 37 percent of the alpha radiation on average (based on slope in Figure 3.7a), but variability is high. The slope in Figure 3.7b is slightly larger than in Figure 3.3 where all pairs of radium and gross alpha were included. In Figure 3.7 only wells in which gross alpha >15 pCi/L are included because this is the level from which an analysis for uranium is required. The low negative correlation between radium and uranium (Figure 3.7c) implies that high gross alpha in the aquifer are due to either high uranium or high radium but most probably not high concentrations of both. Most of the samples in Figure 3.7c where uranium exceeds 20 pCi/L have low levels of radium.

3.2 HYDROGEOLOGY OF POLK, SAN JACINTO AND MONTGOMERY COUNTIES

Subsurface deposits in Polk, Montgomery and San Jacinto Counties consist mainly of sediments of Pliocene and Pleistocene age making up the last progradation wedges in the Gulf Coast. Gulf Coast sediments consist of several progradation wedges of Tertiary age composed of alternating sandstone and clay corresponding to variations in sea level and in inland sediment input as well as in other factors. Those wedges are approximately parallel to the current shoreline and the deposition process is still active today (*e.g.*, Mississippi River and delta). In the Gulf Coast lowlands, those deposits are generally divided into six or more operational units: the Fleming formation of Miocene age whose base includes the Oakville Sandstone, the Goliad/Willis formations of Pliocene age, and the Lissie and Beaumont Formations of Pleistocene age. The Lissie formation is sometimes divided into a lower unit (Lissie s.s. or Bentley) and an upper unit, the Montgomery formation. The general dip of the formations toward the Gulf is 0.01 ft/ft or less on average. Some salt domes exist at depth in the south of Polk and Montgomery Counties but they do not seem to alter the general structure of the Upper Tertiary formations.

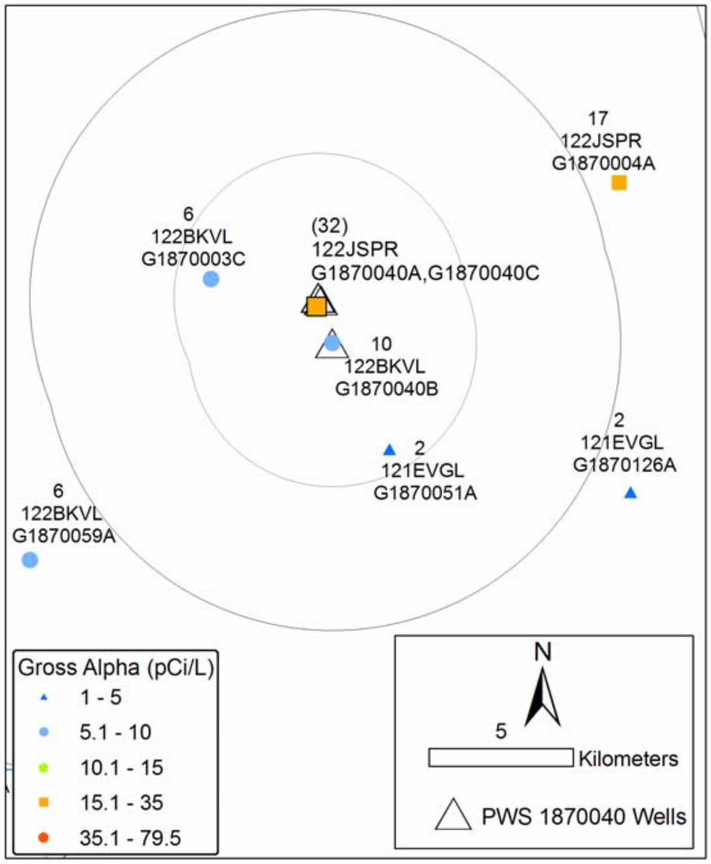
The Gulf Coast aquifer is recognized as a major aquifer in the State of Texas (Ashworth and Hopkins 1995; Mace, *et al.* 2006). In the Tertiary Gulf Coast system, the general flow system consists in water infiltrating in the outcrop areas of the more permeable formations, some of it discharging into rivers and springs along short flow paths, and some of it flowing downdip into the deeper sections of the aquifers. The fate of that slowly moving water is to slowly percolate up by cross-formational flow and discharge into the ocean. This process is necessary to maintain mass balance in the regional flow system although, because of heavy pumping in some areas, the natural upward flow has been locally reversed. The northern confines of Polk County include the upper formations of the Jackson Group of Eocene age and the Catahoula formation of mostly Oligocene age. The Catahoula formation is generally recognized as the low-permeability base of the Gulf Coast aquifer, although it can locally produce water. The other hydrostratigraphic units of the Coastal Plain are the Jasper aquifer, the Burkeville confining system, and the Evangeline and Chicot aquifers (Baker 1979). The Jasper aquifer is composed of the base of the Fleming formation, that is, the Oakville Sandstone, as well as the Catahoula sandstone hydraulically connected to them. The upper part of the Fleming formation makes up the Burkeville confining system. The Evangeline aquifer includes mostly the Goliad Sand but also the upper sections of the Fleming formation when permeable. The remainder and younger formations of the section (Willis Sand, Lissie and Beaumont Formations) make up the Chicot aquifer (Kasmarek and Robinson 2004). Polk and Montgomery Counties present a similar stratigraphy, only slightly shifted toward more recent sediments in Montgomery County; there, the oldest sediments at the surface are from the Fleming formation and they crop out in the extreme northwest area of the county. The succession is then the same in both counties with the addition of a large section of Beaumont Clay of Pleistocene age south of Lake Conroe along the West Fork San Jacinto River. Some Beaumont Clay also exists in southwest Polk County along Lake Livingstone and the Trinity River.

The base of the Jasper aquifer is at a depth of 0 (outcrop area) to 3,000 feet below ground surface. The Oakville formation, forming the bulk of the Jasper aquifer, consists of fluvial fine- to coarse-grained partially consolidated sand with silt and clay intercalations. Its thickness ranges from 700 to 1,200 feet (increasing downdip) in the Polk and Montgomery County area with a high net sand thickness (Kasmarek and Robinson 2004). The net sand thickness varies from <400 feet to >600 feet with a sand fraction >40 percent (Galloway, *et al*. 1986). The net thickness of sand within the aquifer varies according to the geological conditions in which the sediments were deposited. The Goliad formation, approximately equivalent to the Evangeline aquifer, unconformably overlies the top of the Fleming formation, which is composed of mostly clay with some calcareous sand. The Upper Fleming formation depositional systems indicate an environment near the shoreline with fluvial sediments transitioning into fluvial, deltaic, and lagoonal sediments outside of the study area toward the Gulf. This formation acts as a leaky confining layer between the Jasper and the Evangeline aquifers ("Burkeville confining system") and has an approximate thickness of 300 feet. Goliad sand is medium to coarse-grained and unconsolidated with intercalations of calcareous clay and marl whose base is located at approximately 1,000 feet below ground surface. The fluvial and deltaic sand of the Goliad formation suggest another small retreat of the shoreline toward the Gulf. Their thickness is in the range from 0 in the outcrop area to a consistent 800-foot downdip to more than 1,000 feet in Southern Montgomery County. Goliad Sand grades into the generally coarse-grained Willis Sand whose depositional system arrangement is similar to that of the Goliad Sand. The Willis Sand makes up the Chicot aquifer with the overlying fine to coarse-grained Lissie Sand. The top of the Lissie formation, with a higher clay content, and the Beaumont Clay generally pressurize the more permeable sand of the Willis and Bentley formations confining the Chicot aquifer. The Chicot aquifer is not well-expressed in Polk County but its thickness can reach 200 feet in southern Montgomery County. Water quality and well yield are generally good in the Gulf Coast aquifer in northeast Texas including in Polk, San Jacinto, and Montgomery Counties.

3.3 DETAILED ASSESSMENT OF INDIAN SPRINGS PWS

Indian Springs PWS system includes four wells: G1870040A and G1870040C, which are only 20 meters apart, G1870040D, located about 130 meters southeast of wells A and C and well G1870040B, which is about 1.7 kilometers to the south (Figure 3.8). Water samples for this system are taken from two entry points (EP). EP1 obtains water from wells A and C; therefore, analysis of these samples cannot be related to a single well (and therefore does not appear in our large scale analysis which incorporates only data that can be related to a single well). Nevertheless in the detailed assessment these data are essential. In the case of wells A and C that are 20 meters apart, drilled to the same depth (285 feet) and assigned the same hydrostratigrafic unit, their data are represented as a point in the kilometer scale map (Figure 3.8). EP2 obtains water only from well B, therefore, water quality samples are related to that well. Neither EP is related to well D, nor any raw samples exist for this well; therefore, this well was not incorporated in the following analysis.

Figure 3.8 Gross Alpha in 5 and 10-km Buffers of the Indian Springs PWS Wells



Note: The first number near each well is its gross alpha level in pCi/L, the second row is the aquifer unit from which the wells obtain water and the third row is the well identifier in the TCEQ database.

Figure 3.8 reveals three levels of gross alpha corresponding with aquifer units: high gross alpha in unit 122JSPR (Jasper aquifer), medium gross alpha in unit 122BKVL (Burkeville confining unit – a low hydraulic conductivity unit positioned below the Evangeline aquifer and above the Jasper aquifer), and very low gross alpha in 121EVGL (Evangeline aquifer). This trend can be seen in Table 3.2 where the wells are sorted according to the aquifer unit.

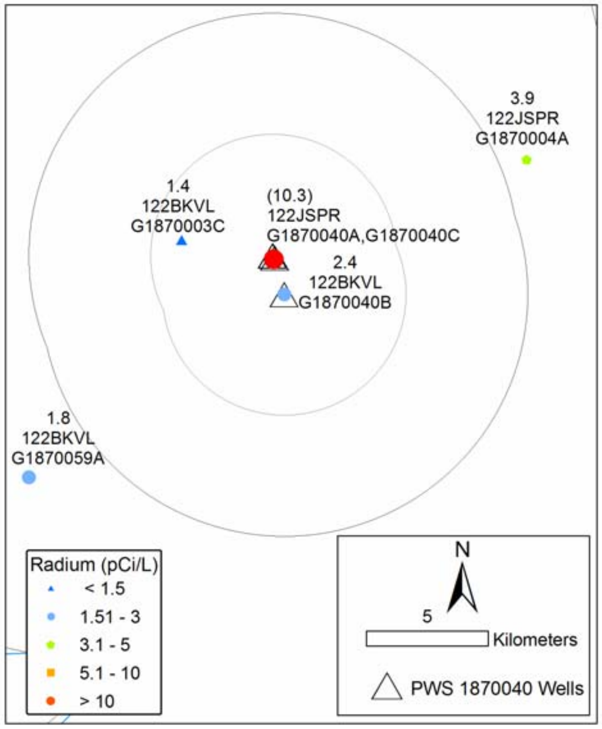
Wells A and C are located either on the Burkeville outcrop (if so the Evangeline does not exist in their profiles) or in the northwestern end of the Evangeline outcrop where this unit is still very shallow (based on the borders of aquifer units from the Groundwater Availability Modeling report for the northern Gulf Coast aquifer (Kasmarek and Robinson 2004). If the logs of these wells indicate a saturated interval in the Evangeline aquifer that is transmissive enough for production, screening this interval could help dilute the contaminated water from the Jasper aquifer. Figure 3.8 and Table 3.2 show that some wells in this area produce water from the Burkeville confining unit, and if a relatively transmissive interval exists in the Burkeville part of the profile it could also dilute the contaminated water, if screened (not as good as the Evangeline aquifer because radionuclide levels in the Burkeville unit are higher).

The nearest Evangeline well is the shallow well (130 feet) G1870051A located 4.3 km south of the Indian Springs PWS southern well B. Farther to the south and east, the Evangeline aquifer becomes thicker (well G1870126A, 11 km from well B is 365-feet deep and still in the Evangeline aquifer, Figure 3.8).

Table 3.2 Gross Alpha Levels and Aquifer Units at Indian Springs PWS and Nearby Wells

Well ID	Well Depth (ft)	Aquifer unit	Screen Top (ft)	Screen Bottom (ft)	Sampling Date	Gross Alpha (pCi/L)
G1870051A	130	121EVGL			3/29/2001	<2
G1870126A	365	121EVGL			10/20/2003	<2
G1870136A	300	122BKVL			3/29/2001	<2
G1870059A	320	122BKVL			2/18/2003	6
G1870003C	385	122BKVL			3/18/2003	6
G1870141A	378	122BKVL	310, 362	320, 378	9/16/2003	9
G1870040B	**255**	**122BKVL**	**235**	**255**	**10/21/2003**	**10**
G1870138A	310	122BKVL			7/13/2004	10
G1870146A	358	122JSPR			6/25/2001	<2
G1870007F	290	122JSPR			10/16/2003	12
G1870004A	644	122JSPR	589	634	10/20/2003	17
G1870044A	454	122JSPR	429	454	11/2/2004	37
G1870040A,C	**285**	**122JSPR**			**11/2/2004**	**32**
G1870040D	**665**	**122JSPR**	**635**	**665**		**no data**

Figure 3.9 Combined Radium (pCi/L) in 5 and 10-km Buffers of the Indian Springs PWS Wells (Data from the TCEQ Database)



The radium hydrostratigraphic distribution is the same as the distribution for gross alpha: higher levels in the Jasper aquifer than in the Burkeville confining unit (Figure 3.9). Evangeline wells with very low gross alpha were not sampled for radium. Samples from wells A and C in 2003 show lower levels of gross alpha than samples in 2004, but they are still above the MCL both for radium and gross alpha (Table 3.3).

Table 3.3 History of Gross Alpha, Combined Radium and Combined Uranium in Wells from Indian Springs PWS

Well	Sampling Date	Gross Alpha (pCi/L)	Radium (pCi/L)	Uranium (pCi/L)
G1870040A,C	10/21/2003	25.4	5.7	<2
G1870040A,C	11/2/2004	32.4	10.3	<1.5
G1870040B	10/21/2003	9.8	2.4	

3.4 SUMMARY OF ALTERNATIVE GROUNDWATER SOURCES FOR INDIAN SPRINGS PWS

Sampling well D for radionuclides could provide information on what happens in deeper parts of the Jasper aquifer in this area and is recommended before deciding on any solution for alternative PWSs. From the available data it seems that radionuclide levels in this area are higher in the Jasper aquifer; therefore, diluting water from the Jasper aquifer with water from the Evangeline aquifer or even the Burkeville confining unit is recommended. The Evangeline aquifer probably does not exist in the saturated intervals of the northern wells A and C, therefore, some improvement in radionuclide levels may be achieved by screening transmissive intervals of the Burkeville confining unit in these wells to dilute the water from the Jasper aquifer. The best water quality in this area in terms of radionuclides can be obtained from shallow wells open to the Evangeline aquifer. The nearest well penetrating this unit is 4.3 km south of well B (G1870051A). The nearest area to the existing PWS wells where new wells can be drilled into the Evangeline aquifer is east-southeast of well B. More detailed hydrogeologic data are required to find the best location for drilling such a well.

SECTION 4
ANALYSIS OF THE INDIAN SPRINGS PWS

4.1 DESCRIPTION OF EXISTING SYSTEM

4.1.1. Existing System

The location of Indian Springs PWS is shown in Figure 4.1. The Indian Springs PWS is owned and operated by Lake Livingston WSSSC. Indian Springs PWS is the water system that supplies Indian Springs Lake Estate, a large residential subdivision in Polk County. It is located off Highway 190 approximately 8 miles east of Livingston, Texas.

The water source for this PWS is from two water plants (Ole Don and Baker plants) with three groundwater wells total, the two wells located at the Ole Don plant are completed to 285 feet below ground surface in the Jasper aquifer (Code 122JSPR), with pumping capacities of 49 gpm and 180 gpm. The Baker plant well is completed to 255 feet below ground surface in the Burkeville Aquiclude (Code 122BKVL) and has a pumping capacity of 38 gpm. Together the three wells produce a total of 267 gpm. The Ole Don plant has two 45,000 gallon ground storage tanks with a total capacity of 90,000 gallons and one 6,000 gallon pressure tank and the Baker plant has one 24,000 gallon ground storage tank and one 2,500 gallon pressure tank. All of the ground water disinfects with hypochlorite and treats free iron with tripolyphosphate at the wellhead before water is pumped into the tanks and distribution system.

Total combined radium-226 and radium-228 has been detected between 3.1 pCi/L to 9.4 pCi/L since 2004, which exceeds the MCL of 5 pCi/L. Gross alpha particle activity has been detected between 8.1 pCi/L to 29 pCi/L, which exceeds the MCL of 15 pCi/L. The Indian Springs PWS has not encountered any other raw groundwater quality issues.

The treatment employed for disinfection is not appropriate or effective for removal of combined radium or alpha particles, so optimization is not expected to be effective for increasing removal of this contaminant. There is no potential opportunity for system optimization to reduce combined radium concentration in the systems one well. The only cost effective option is likely to find a new water source, either groundwater at a different depth, or acceptable water from an adjacent system.

It may also be possible to identify combined radium-producing strata through comparison of well logs or through sampling of water produced by various strata intercepted by the well screen.

Basic system information is as follows:

- Population served: 1,080
- Connections: 360

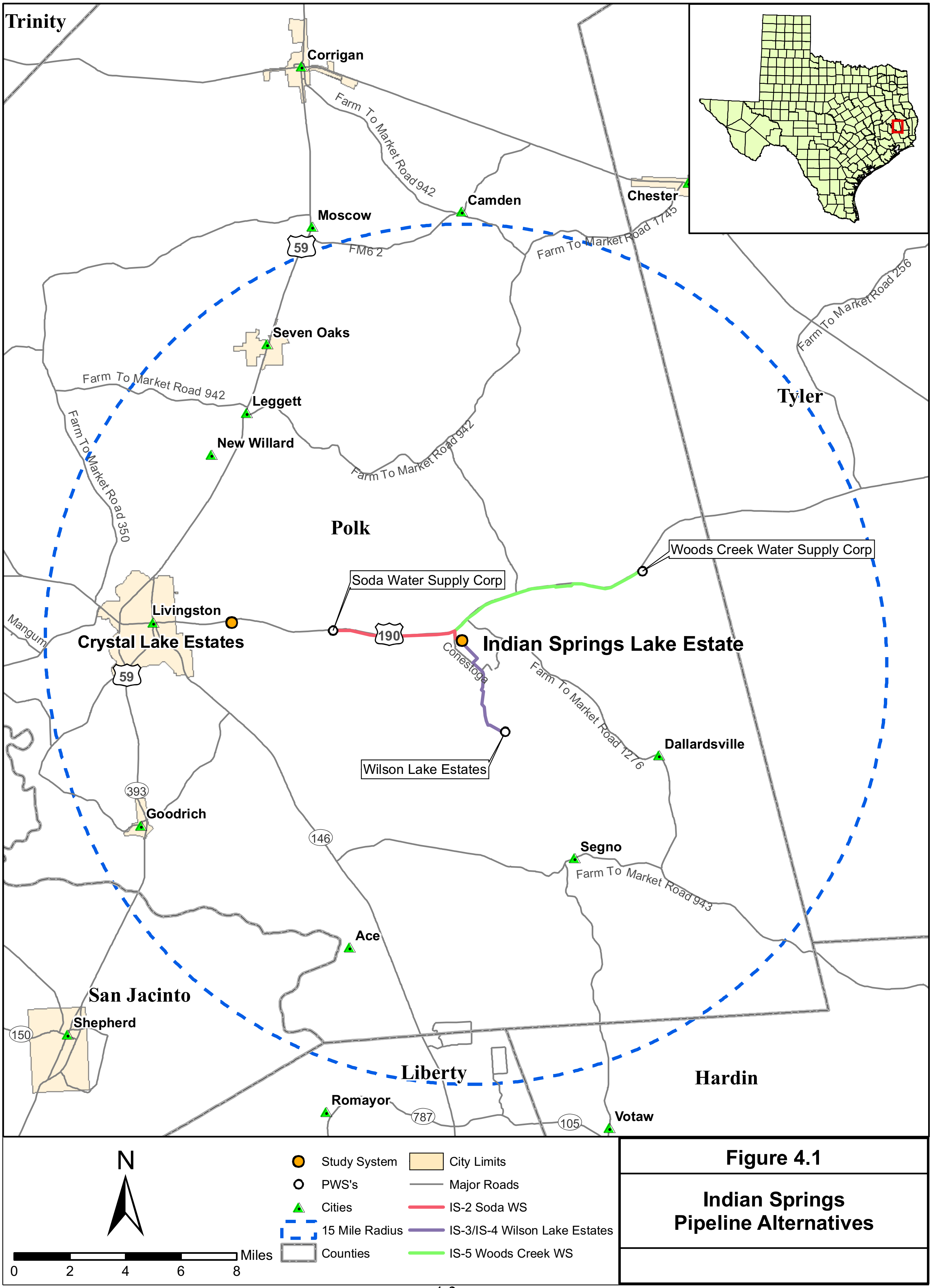


Figure 4.1

Indian Springs Pipeline Alternatives

- Average daily flow: 0.127 mgd
- Total production capacity: 0.410 mgd

Raw water characteristics:

- Typical total combined radium range: 3.1 pCi/L to 9.4 pCi/L
- Typical total alpha particle range: 8.1 pCi/L to 29 pCi/L
- Typical total dissolved solids: 209 - 324 mg/L
- Typical pH range: 7.1 – 7.9
- Typical calcium range: 16 - 74 mg/L
- Typical magnesium range: 3 – 4.8 mg/L
- Typical sodium range: 25 – 57.7 mg/L
- Typical chloride range: 16 - 46 mg/L
- Typical bicarbonate (HCO_3) range: 217 - 294 mg/L
- Typical fluoride range: 0.2 – 0.3 mg/L
- Typical iron range: 0.01 – 0.49 mg/L
- Typical manganese range: 0.31 – 0.62 mg/L

Lake Livingston WSSSC has investigated possible solutions to its combined radium and alpha particle issues, including a new treatment system, a new surface water plant at Lake Livingston, blending from another source and drilling a new groundwater well. The capital cost of a treatment system, as well as a new surface water plant was considered but considered not cost effective. Purchasing water from another PWS is a possibility and should be seriously considered. Another alternative examined was the drilling of a new groundwater well that would be completed to an undetermined depth. Drilling a new well was expected to avoid the radium problem. The estimated capital cost of completing the new well in the region was estimated at $40,000 several years ago, but is likely to be between $50,000 to over $100,000 depending on the actual depth required.

4.1.2 Capacity Assessment for Lake Livingston WSSSC – Indian Springs PWS

The project team conducted a capacity assessment of the Lake Livingston WSSSC – Indian Springs PWS. The results of this evaluation are separated into four categories: general assessment of capacity, positive aspects of capacity, capacity deficiencies, and capacity concerns. The general assessment of capacity describes the overall impression of FMT capability of the PWS. The positive aspects of capacity describe those factors that the system is doing well. These factors should provide opportunities for the system to build on to improve capacity deficiencies. The capacity deficiencies noted are those aspects that are creating a particular problem for the system related to long-term sustainability. Primarily, these problems are related to the system's ability to meet current or future compliance, ensure proper revenue to pay the expenses of running the system, and to ensure the proper operation of the system. The last category is titled capacity concerns. These are items that in general are not causing significant problems for the system at this time. However, the system may want to address them before these issues have the opportunity to cause problems.

The project team interviewed the following individuals:

- Scott Baker – General Manager
- John Ganzer – Financial Manager
- Phillip Everett – Supervisor, System Operations
- Boyd McDaniel – Supervisor, System Reports

4.1.2.1 General Structure

Lake Livingston WSSSC is a public utility corporation that provides water services to 52 PWSs in the greater Livingston area and serves a total of 6,894 customers. It is governed by a seven-member board of directors and is financed through water fees and equity buy-in fees. The Lake Livingston WSSSC purchased the Indian Springs PWS along with several others in April 1997 when the previous owner declared bankruptcy. The WSSSC borrowed $1.9 million from CoBank to upgrade the PWSs, and then received a U.S. Department of Agriculture loan for $7 million for additional improvements. Their total operations staff consists of a general manager, field supervisor, eight certified operators, and a construction/general labor crew.

The Indian Springs PWS has 360 connections and serves a population of 1,080. The system has three active well and one inactive well, three ground storage tanks, two pressure tanks, and disinfects using chlorine gas. The PWS also inject tripolyphosphates for sequestering iron. Two certified operators are responsible for O&M activities at the Indian Springs PWS. Well #4 has exceeded MCLs for both gross alpha and combined radium (radionuclides) since 2003.

4.1.2.2 General Assessment of Capacity

Based on the team's assessment, this system has a very good level of capacity. There are several positive FMT aspects of the PWS, but there are also some areas that need improvement.

4.1.2.3 Positive Aspects of Capacity

In assessing a system's overall capacity, it is important to look at all aspects – positive and negative. It is important for systems to understand those characteristics that are working well, so that those activities can be continued or strengthened. In addition, these positive aspects can assist the system in addressing the capacity deficiencies or concerns. The factors that were particularly important for the Indian Springs PWS are listed below.

- **Knowledgeable and Dedicated Staff** - While the general manager has only worked for the Lake Livingston WSSSC for about 1 year, he is certified and has over

30 years' experience in the water industry. The field supervisor is certified and has been working for the WSSSC for 24 years. The other supervisor is also certified and has 26 years with the company. All positions have written job descriptions. The operations staff meets every morning to receive work orders for the day. The water operators rotate being on-call, so the system is covered 24 hours per day. The Board of Directors is composed of individuals who live in the various communities served by the Lake Livingston WSSSC who are familiar with their own water system.

- **Benefits from Economies of Scale –** Indian Springs Lake Estates PWS is one of 52 systems operated by the Lake Livingston WSSSC. This structure allows a very small PWS to benefit from the pool of operators and a central construction/general maintenance crew. They are able to maintain a large inventory of spare parts in their warehouse. All the PWSs owned and operated by the Lake Livingston WSSSC have a single rate structure. As new compliance rules and regulations are introduced that will require more complex and expensive treatment, or as system upgrades and improvements are needed, the ability to take advantage of the economies of scale offered by a single rate structure is critical to maintaining affordability for the small systems. To ensure that the system's finances are adequate, the board reviews the operating budget every month, and compares it with the previous year's expenditures. It has an emergency fund to cover shortfalls, and maintain a reserve account. The Lake Livingston WSSSC tracks the expenses related to electricity, meter reading, and chemicals separately for each PWS. Finally, due to its prudent financial practices, the Lake Livingston WSSSC was able to build its existing office/warehouse complex without incurring any debt.

- **Communication with Customers –** The Lake Livingston WSSSC works hard to keep their customers informed about the water system. They issue a quarterly Public Notice and an annual consumer confidence report (CCR) as required by TCEQ. And because residents have been extremely vocal about the radionuclides problem, the WSSSC has invited the TCEQ to attend public meetings to reassure their customers.

  The WSSSC responds to and documents all customer complaints in a timely manner. If a water line break will take more than a couple of hours to repair, it posts a sign at the entrance to the subdivision. It also issues a "Boil Order" until it is sure the water is free of total coliform bacteria. Finally, it is in the process of developing a website that will enable its customers to view information about their accounts and the activities of the Lake Livingston WSSSC.

- **Cross-Connection Control Program –** The WSSSC has an active program for preventing cross connections in the distribution system. This program includes customer agreements, service inspections on all new taps, and hose-bib vacuum breakers at all new homes. This program provides an increased level of public health protection.

4.1.2.4 Capacity Deficiencies

The following capacity deficiencies were noted in conducting the assessment and could impact the ability of the water system to meet compliance with current and future regulations and to ensure long-term sustainability.

- **Water Losses –** A water audit conducted in 2005 estimated 60 percent water loss at Indian Springs PWS. The main lines are made of pipe that is not NSF approved. These pipes were not properly buried, and when the roads are graded, some of the pipes are exposed, causing leaks. This results in reduced pressure and increased customer complaints. A reduction in water loss would significantly reduce the amount of water that must be pumped and/or treated. Reducing water losses could result in a cost savings depending on the compliance alternative implemented. In addition, there is no water conservation program. This is especially critical due to the significant amount of water loss that this system sustains. Conservation reduces the demand on the source, reduces chemical and electrical costs, and minimizes wear and tear on equipment such as pumps.
- **Lack of Compliance with Radionuclides Standard –** The Lake Livingston WSSSC is under a Compliance Order for the Indian Springs PWS, which outlines the steps the system needs to take to return to compliance. The WSSSC has been working to address the compliance issue by hiring a geological company that is searching for areas in the aquifer that can meet the radionuclides regulations. As part of this project, they are updating maps of the WSSSCs PWSs. However, the WSSSC advised the project team it has purchased arsenic removal treatment systems for three of its other PWSs. While it is positive the WSSSC is taking a proactive approach to complying with the arsenic standard, it is unclear why it is not concentrating its efforts on the systems that are under a Compliance Order. The WSSSC needs to be working toward radionuclide compliance to avoid further escalation in enforcement actions.

4.1.2.5 Potential Capacity Concerns

The following items were concerns regarding capacity but no specific operational, managerial, or financial problems can be attributed to these items at this time. The system should address the items listed below to further improve FMT capabilities and to improve the system's long-term sustainability.

- **Lack of Written Long-Term Capital Improvements Plan –** While there appears to be some process in place to plan for future improvements and there is a Capital Budget, there is no formal written plan. The lack of a long-term written plan could negatively impact the system's ability to develop a budget and associated rate structure that will provide for the system's long term needs.

  The general manager indicated it is in the process of applying for a planning loan/grant with the TWDB which will address growth and compliance concerns. Specific projects will improve capacity, pressure and water quality compliance. It will also include replacement of its PVC pipes that are not NSF International (NSF)-approved. The planning grant should be used to develop a written long-term Capital Improvement Plan to address this concern.

- **Rates and Frequency of Rate Evaluation –** The WSSSC's water rates are based on recommendations by the staff and reviewed by the board. The last rate increase was in June 2004. Although current rates fully cover the costs of service, they are not sufficient to allow for future growth or if the system incurs additional debt. In addition, it does not appear the rates are evaluated on a regular basis.

- **Preventative Maintenance Program –** It doesn't appear to be a preventative maintenance program, and in general, the WSSSC makes repairs on a reactive basis instead of a proactive one. There is no scheduled maintenance for line flushing or valve exercising. Routine flushing clears sediment in the lines and routine valve exercising identifies valves that need replacement, and ensures proper operation during the next line repair. However, it does have a written O&M manual, which is located in the pumphouse and referred to as necessary.

- **Emergency Plan –** The Lake Livingston WSSSC does not have a written emergency plan, nor does it have enough emergency equipment such as generators. In the event of a power outage, it would have to rely solely on the water in the storage tanks. In 2005, Hurricane Rita struck the Lake Livingston area and several of its PWSs were without water for 6-7 days. As a result of the storm, a statewide program known as "TxWARN" was developed and implemented by the State of Texas. The WSSSC is now a member of this program that will enable water facilities to help each other and share resources.

  The system should have an emergency or contingency plan that outlines what actions will be taken and by whom. The emergency plan should meet the needs of the facility, the geographical area, and the nature of the likely emergencies. Conditions such as storms, floods, major line breaks, electrical failure, drought, system contamination or equipment failure should be considered. The emergency plan should be updated annually, and larger facilities should practice implementation of the plan annually.

4.2 ALTERNATIVE WATER SOURCE DEVELOPMENT

4.2.1 Identification of Alternative Existing Public Water Supply Sources

Using data drawn from the TCEQ drinking water and TWDB groundwater well databases, the PWSs surrounding the Indian Springs PWS were reviewed with regard to their reported drinking water quality and production capacity. PWSs that appeared to have water supplies with water quality issues were ruled out from evaluation as alternative sources, while those without identified water quality issues were investigated further. If it was determined that these PWSs had excess supply capacity and might be willing to sell the excess, or might be a suitable location for a new groundwater well, the system was taken forward for further consideration.

Table 4.1 is a list of the selected PWSs within 8.7 miles of Indian Springs PWS This distance was limited to 8.7 miles because the Indian Springs PWS is within that distance requires a similar water supply solution and is owned and operated by the same parent company as Indian Springs PWS and it is reasonable to assume that if a suitable supply alternative was found for Indian Springs PWS that it could be shared with Indian Springs PWS and there would not be a need to look farther away for additional supply.

From the initial list of PWSs, three were selected for further evaluation based on factors such as water quality, distance from the Indian Springs PWS, sufficient total production capacity for selling or sharing water, and willingness of the system to sell or share water or drill a new well. The PWSs selected for further evaluation are shown in Table 4.2.

Table 4.1 Selected Public Water Systems within 8.4 Miles of the Indian Springs PWS

PWS ID	PWS Name	Distance from Indian Springs	Comments/Other Issues
1870051	Wilson Lake Estates Water System	3.3 miles	Small (0.46 mgd) system with no WQ issues
1870003	Soda WSC PWS	5.4 miles	Large (1.0 mgd) system with no WQ issues
1870004	Woods Creek WSC PWS	7.0 miles	Small (0.108 mgd) system with no WQ issues
1870141	Beech Creek Village PWS	7.2 miles	Small (0.048 mgd) system with no WQ issues
1870138	Country Wood PWS	7.5 miles	Small (0.056 mgd) system with no WQ issues
1870126	Dallardsville Segno PWS	7.7 miles	Small (0.213 mgd) system with no WQ issues

Based upon the initial screening summarized in Table 4.1 above, three alternatives were selected for further evaluation. These are summarized in Table 4.2.

Table 4.2 Public Water Systems Within the Vicinity of the Indian Springs PWS Selected for Further Evaluation

PWS ID	PWS Name	Pop	Conn	Total Production (mgd)	Ave Daily Usage (mgd)	Approx. Dist. from Indian Springs	Comments/Other Issues
1870051	Wilson Lake Estates PWS	225	75	0.24	0.17	3.3 miles	Has excess capacity. It is owned by Lake Livingston Water Supply and Sewer Service Corp. and may be a new well site.
1870003	Soda WSC PWS	1788	596	1.05	0.17	Water Main <1 mile	Has excess capacity. Currently sell retail, but may consider selling wholesale or annexing an area to sell retail.
1870004	Woods Creek WSC PWS	282	94	0.108	0.019	7 miles	Has excess capacity. Currently sell retail. .

4.2.1.1 Wilson Lake Estates PWS

The Wilson Lake Estates PWS is located west of the City of Livingston, and 4.5 miles to the southwest by road of Indian Springs PWS. The Wilson Lake Estates PWS is owned and operated by Lake Livingston WSSSC and is supplied by one groundwater well completed at 750 feet deep and has a pumping capacity of 100 gpm. Water is disinfected with gaseous chlorine before being sent to a 22,000-gallon ground storage tank and then on to a 1,500-gallon pressure tank. The distribution system has substandard piping and leaks frequently. Wilson Lake Estates PWS serves a population of 225 and has 75 metered connections.

Wilson Lake Estates PWS does have sufficient excess capacity to supplement Indian Springs PWS's existing supply, and they are owned by the same parent company which would make contracting issues minimal.

4.2.1.2 Soda Water Supply Corporation PWS

Soda Water Supply Corporation (WSC) PWS is located east of the City of Livingston on Hwy 190. The water plants are located 5.4 miles to the west of Indian Springs PWS, but less than 1 mile from Soda WSC's main water supply line that passes along Highway 190 west of Indian Springs PWS and then south along farm-to-market road (FM) 1276.

The PWS is owned and operated by the Soda WSC, and is operated by the same billing and operations personnel as Woods Creek WSC. The PWS is supplied by three groundwater treatment plants. Plants 1 and 2 are supplied two groundwater wells each. Plant 3 is supplied by one groundwater well. The two groundwater wells supplying Plant 1 are completed in the Burkeville Aquiclude of the Gulf Coast aquifer are 356 feet deep and 375 feet deep and have pumping capacities of 157 gpm and 72 gpm, respectively. Plant 2 is also supplied by two groundwater wells completed in the Jasper aquifer (Code 122JSPR). One well is 500 feet deep and the other is 610 feet deep with pumping capacities of 158 gpm and 219 gpm, respectively. Plant 3 is supplied by one groundwater well completed in the Burkeville Aquiclude of the Gulf Coast aquifer and is 320 feet deep with pumping capacity of 96 gpm. The combined production capacity of the system and its five wells is approximately 1.0 mgd. Water is disinfected with hypochlorite and treated with an orthophosphate rust inhibitor before being sent to the storage tanks and distribution system.

Soda WSC has 152,000 gallons in storage capacity and 1,900 gallons of hydropneumatic storage and 3.384 mgd in service pump capacity. Soda WSC serves a population of 1,788 and has approximately 596 metered connections.

Lake Livingston WSSSC management has had communications with the Soda WSC in the past discussing the possibility of Soda WSC providing finished groundwater to the Indian Springs PWS for the purpose of blending with its current supply.

4.2.1.3 Woods Creek Water Supply Corporation

Woods Creek WSC PWS is located approximately 6 miles east of the City of Livingston, and approximately 7 miles to the northeast of Indian Springs Lake Estates. The PWS is owned and operated by Woods Creek WSC and is supplied by one groundwater well completed in the Jasper aquifer (Code 122JSPR). The well is 644 feet deep and has a pumping capacity of 120 gpm. Water is disinfected with gaseous chlorine and treated with an orthophosphate rust inhibitor before being sent to a 15,000 gallon ground storage tank and then to the 1,500 gallon pressure tank and distribution system. Woods Creek WSC PWS serves a population of 292 and has 94 metered connections.

Woods Creek WSC PWS may have sufficient excess capacity to supplement the Indian Springs PWS existing supply. Woods Creek WSC PWS is operated by the five-member board of directors that has not considered selling water wholesale in the past, but may consider it in the future.

4.2.2 Potential for New Groundwater Sources

4.2.2.1 Installing New Compliant Wells

Developing new wells or well fields is recommended, provided good quality groundwater available in sufficient quantity can be identified. Since a number of PWSs in the area do not have problems with radium, it should be possible to install a new well that has compliant groundwater without a problem.

Installation of a new well in the vicinity of the existing system intake point is likely to be an attractive option for obtaining compliant water provided compliant groundwater can be found, since the PWS is already familiar with operation of a water well. As a result, existing nearby wells with good water quality should be investigated. Re-sampling and test pumping would be required to verify and determine the quality and quantity of water at those wells.

Installation of a new well to the Burkeville, Jasper, or Evangeline aquifers may be a good option for the Indian Springs PWS. Additionally, PWSs located within 5.4 miles of the Indian Springs Lake PWS have wells drilled to a depth of 320-750 feet and produce large quantities of compliant water.

The Indian Springs PWS wells are set between 255 and 285 feet deep. Other local PWSs have wells set to 750 feet and have no water quality problems. It may be possible to adjust the screen depth of the existing well to access other water-bearing sand, although further study would be required to make that determination.

The use of existing wells should probably be limited to use as indicators of groundwater quality and availability. If a new groundwater source is to be developed, it is recommended that a new well or wells be installed instead of using existing wells. This would ensure well characteristics are known and meet standards for drinking water wells.

Some of the alternatives suggest new wells be drilled in areas where existing wells are compliant with the future arsenic MCL of 10 µg/L. In developing the cost estimates, Parsons assumed the aquifer in these areas would produce the required amount of water with only one well. Site investigations and geological research, which are beyond the scope of this study, could indicate whether the aquifer at a particular site and depth would provide the amount of water needed or if more than one well would need to be drilled in separate areas.

4.2.2.2 Results of Groundwater Availability Modeling

The Gulf Coast aquifer system that extends along the entire Texas coastal region is the groundwater source for the PWS. Five hydrogeologic units comprise the aquifer system, from land surface downward, the Chicot aquifer, the Evangeline aquifer, the Burkeville confining unit, the Jasper aquifer, and the Catahoula confining unit. For the Indian Springs PWS, both the Jasper aquifer and Burkeville confining unit are the groundwater source reported in the TCEQ well database. These two units of the northern Gulf Coast aquifer are also the primary water sources for wells located within 15 miles of the PWS, and throughout central Polk County.

Regional groundwater withdrawal throughout the northern part of the Gulf Coast aquifer system is extensive and likely to steadily increase over the next decades. Since the 1900s, large groundwater withdrawals have resulted in declines in the aquifer's potentiometric surface from tens to hundreds of feet conditions (Mace *et al*. 2006). A GAM for northern part of the Gulf Coast aquifer was recently developed by the TWDB. Modeling was performed by the U.S. Geological Survey to simulate historical conditions (Kasmerek and Robinson 2004), and to develop long-term groundwater projections (Kasmerek, Reece and Houston 2005). Modeling of a TWDB scenario based on 50-year regional projections by regional user groups anticipate extensive groundwater use and drop in aquifer levels, with the largest declines around the Houston metropolitan area.

GAM simulation data reported by Kasmerek, Reece and Houston (2005) indicate that over a 50-year simulation withdrawals for the entire Gulf Coast aquifer are expected to peak at 920 million gallons per day (mgd) in 2020, and subsequently decrease to 850 mgd. Withdrawals from the Jasper represent only a fraction of those values; a 2000 rate of 36 mgd is expected to increase to 51 mgd by 2010, an approximate 42 percent increase, and stabilize within 6 percent of that value through 2050 (Kasmerek, Reece and Houston 2005). A minimum increase in water elevation is anticipated during the 50-year simulation period throughout Polk County. Withdrawals from the Burkeville confining unit are anticipated to reach a maximum of 2 mgd through 2050.

The GAM of the northern part of the Gulf Coast aquifer was not run for the PWS as groundwater availability would reflect regional conditions primarily driven by groundwater withdrawal from the Houston area. Water use by the small PWS would represent a minor addition to the regional water use, making potential changes in aquifer levels well beyond the spatial resolution of the regional GAM model.

4.2.3 Potential for New Surface Water Sources

The Indian Springs PWS is located in the lower Neches Basin. For that basin, the TWDB 2002 Water Plan anticipates a severe reduction in water availability, up to 66 percent (from 604,037 acre-feet per year [AFY] in 2000 to 206,294 AFY in 2050). Approximately 3 miles west of the site, the Neches Basin transitions into the Trinity Basin where water availability is expected to decrease up to 11 percent over the next 50 years.

There is a potential for development of new surface water sources for the system as indicated by the 2002 TCEQ water availability map for the Neches Basin. The basin extends over the eastern half of Polk County, where the Indian Springs PWS is located. For this area, the 2002 TCEQ evaluation indicated a year-round availability of surface water for new applications (new perpetual rights). Development of a new surface water source; however, is not considered feasible for a small PWS due to the permitting required, and the cost and complexity associated with construction and operation of intake works, treatment plant, and water conveyance. Development of a new surface water source is considered more appropriate as a regional solution to be undertaken by a group of small PWSs or by a regional water supply organization. For this study, surface water source development alternatives are limited to obtaining water from existing water providers that utilize surface water.

4.2.4 Options for Detailed Consideration

The initial review of alternative sources of water results in the following options for more-detailed consideration:

1. New Well at the Indian Springs (IS) PWS. A new groundwater well would be completed at a different depth in the vicinity of the existing well at Indian Springs PWS and would utilize the rest of the existing system (Alternative IS-1). Lake Livingston WSSSC installed a new 750-foot well at the nearby Wilson Lake Estates Water PWS 2 years ago for approximately $40,000.

2. Purchase Water from Soda WSC PWS. A connection would be made to the Soda WSC PWS water main pipeline that passes along Highway 190 or FM 1276 and a pipeline and pump system would be constructed to pump the finished water to Indian Springs PWS (Alternative IS-2).

3. Purchase Water from Wilson Lake Estates PWS. A connection would be made to the Wilson Lake Estates PWS and a pipeline and pump system would be constructed to pump the finished water to Indian Springs PWS (Alternative IS-3).

4. New Well at Wilson Lake Estates PWS. A new well would be drilled adjacent to the Wilson Lake Estates PWS and a pipeline and pump system would be constructed to pump the finished water to Indian Springs PWS (Alternative IS-4).

5. Purchase Water from Woods Creek WSC. A connection would be made to the Woods Creek WSC PWS and a pipeline and pump system would be constructed to pump the finished water to Indian Springs PWS (Alternative IS-5).

6. Installing a new well within 10, 5, or 1 mile of Indian Springs PWS that would produce compliant water in place of the water produced by the existing wells (Alternatives IS-6, IS-7, and IS-8).

4.3 TREATMENT OPTIONS

4.3.1 Centralized Treatment Systems

Centralized treatment of the well water is identified as a potential option. Ion exchange, WRT Z-88, and $KMnO_4$ treatment could all be potentially applicable. The central IX treatment alternative is IS-9, the central WRT Z-88 treatment alternative is IS-10, and the central $KMnO_4$ treatment alternative is IS-11.

4.3.2 Point-of-Use Systems

POU treatment using resin-based adsorption technology or RO is valid for total radium removal. The POU treatment alternative is IS-12.

4.3.3 Point-of-Entry Systems

POE treatment using resin based adsorption technology or RO is valid for total radium removal. The POE treatment alternative is IS-13.

4.4 BOTTLED WATER

Providing bottled water is considered an interim measure to be used until a compliance alternative is implemented. Even though the community is small and people know each other; it would be reasonable to require a quarterly communication advising customers of the need to take advantage of the bottled water program. An alternative to providing delivered bottled water is to provide a central, publicly accessible dispenser for treated drinking water. Alternatives addressing bottled water are IS-14, IS-15, and IS-16.

4.4 ALTERNATIVE DEVELOPMENT AND ANALYSIS

A number of potential alternatives for compliance with the MCL for total radium have been identified. Each of the potential alternatives is described in the following subsections. It should be noted that the cost information given is the capital cost and change in O&M costs associated with implementing the particular alternative. Appendix C contains cost estimates for the compliance alternatives. These compliance alternatives represent a range of possibilities, and a number of them are likely not feasible. However, all have been presented to provide a complete picture of the range of alternatives considered. It is anticipated that a PWS will be able to use the information contained herein to select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation.

4.4.1 Alternative IS-1: New Well in the Vicinity of Indian Springs PWS

This alternative involves completing a new deeper well at the current Indian Springs PWS site, and tying it into an existing PWS. The new well would be between 300 and 750 feet deep. Based on the water quality data in the TCEQ database, it is expected that groundwater from this location at a different depth may be compliant with drinking water MCLs.

The estimated capital cost for this alternative includes completing the new well, and constructing the connection piping and a new storage tank and feed pump set to supply water to the existing system. The estimated capital cost for this alternative is $102,396, and the alternative's estimated annual O&M cost savings is $5,942.

The reliability of adequate amounts of compliant water under this alternative should be good. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as easy to operate and repair, since O&M and repair of the current system is well understood, and Lake Livingston WSSSC personnel currently operate it. If the decision were made to perform blending, then the operational complexity would increase.

Obtaining agreements is not necessary for implementing this option, and should not impact the feasibility of this alternative.

4.4.2 Alternative IS - 2: Purchase Water from Soda WSC

This alternative would require constructing a pipeline from Soda WSC to the Indian Springs PWS. A pump station would be required to overcome pipe friction and the elevation differences between Soda WSC and Indian Springs PWS. The current storage tank and feed pump set would be utilized at the Indian Springs PWS site. The required pipeline would be constructed of 6-inch pipe and would follow Highway 190, Ole Long Pull Road, the Ole Don Road, and James Boulevard. The pipeline required would be approximately 5 miles long and would terminate at the storage tank.

The pump station would include two pumps (minimum 14 hp each), one of the pumps is a standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Indian Springs Lake Estates subdivision, since the incremental cost would be relatively small, and it would provide operational flexibility.

This alternative involves regionalization by definition, since the Indian Springs PWS would obtain drinking water from an existing larger supplier. It is possible the Lake Livingston WSSSC could turn over provision of drinking water to the Soda WSC PWS instead of purchasing water.

The estimated capital cost for this alternative includes constructing the pipeline, pump station, and storage tank and feed pump set. The estimated O&M cost for this alternative includes the purchase price for the treated water minus the cost related to current operation of the Indian Springs PWS well, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is $1.65 million, and the alternatives' estimated annual O&M cost is $75,425.

The reliability of adequate amounts of compliant water under this alternative should be good. Soda WSC provides treated surface water on a large scale, facilitating adequate O&M resources. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood. If the decision were made to perform blending, then the operational complexity would increase.

The feasibility of this alternative is dependent on an agreement being reached with the Soda WSC to purchase treated drinking water.

4.4.3 Alternative IS - 3: Purchase Water from Wilson Lake Estates PWS

This alternative would require constructing a pipeline from Wilson Lake Estates PWS to the Indian Springs PWS. A pump station would be required to overcome pipe friction and the elevation differences between the Wilson Lake Estates PWS and the Indian Springs PWS, and the current storage tank and feed pump set would be utilized at the Indian Springs PWS site. The required pipeline would be constructed of 6-inch pipe and would follow Wilson Lake Estates Road, Davisville Road, unknown Road, Conestoge Trail, Lincoln Wiggins Street, the Ole Don Road, and James Boulevard to the Indian Springs PWS. Using this route shown in Figure 4.1, the pipeline required would be 4.3 miles long. The pipeline would terminate at the existing storage tanks.

The pump station would include two pumps (minimum 13 hp each), one of the pumps is a standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Indian Springs PWS, since the incremental cost would be relatively small, and would provide operational flexibility.

This alternative has limited opportunity for regionalization in that the Lake Livingston WSSSC could possibly turn over provision of drinking water to the Wilson Lake Estates PWS instead of installing its own new well. Other non-compliant systems have not been identified near Indian Springs PWS or along the pipeline route, so there is little chance to share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing the pipeline, pump station, and storage tank. The estimated O&M cost for this alternative are related to maintenance cost for the pipeline, and power and O&M labor and materials for the pump station, storage. The estimated capital cost for this alternative is $1.46 million, and the alternatives' estimated annual O&M cost is $75,003.

The reliability of adequate amounts of compliant water under this alternative should be good. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pumps stations is well understood, and Lake Livingston WSSSC currently operates pumps and wells.

Obtaining agreements is not necessary for implementing this option, and should not impact the feasibility of this alternative.

4.4.4 Alternative IS - 4: New Well at Water from Wilson Lake Estates PWS

This alternative would require constructing and new well, pump station and a pipeline from the Wilson Lake Estates PWS to the Indian Springs PWS. A pump station would be required to overcome pipe friction and the elevation differences between the Wilson Lake Estates PWS and the Indian Springs PWS, and the current storage tank and feed pump set would be utilized at the Indian Springs PWS site. The required pipeline would be constructed of 6-inch pipe and would follow Wilson Lake Estates Road, Davisville Road, an unknown road, Conestoge Trail, Lincoln Wiggins Street, the Ole Don Road, and James Boulevard to the Indian Springs PWS. Using this route, shown in Figure 4.1, the pipeline required would be 4.3 miles long. The pipeline would terminate at the existing storage tanks.

The pump station would include two pumps (minimum 13 hp each), one of the pumps is a standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Indian Springs Lake Estates Subdivision, since the incremental cost would be relatively small, and would provide operational flexibility.

This alternative has limited opportunity for regionalization because the Lake Livingston WSSSC could possibly turn over provision of drinking water to the Wilson Lake Estates PWS instead of installing its own new well. Other non-compliant systems have not been identified near the Indian Springs PWS or along the pipeline route, so there is little chance to share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing a new well, pipeline, pump station, and storage tank. The estimated O&M cost for this alternative are related to maintenance cost for the pipeline and power and O&M labor and materials for the pump station, storage, and feed pumps. The estimated capital cost for this alternative is $1.55 million, and the alternatives' estimated annual O&M cost is $11,161.

The reliability of adequate amounts of compliant water under this alternative should be good. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pumps stations is well understood. Lake Livingston WSSSC currently operates pumps and wells.

Obtaining agreements is not necessary for implementing this option, and should not impact the feasibility of this alternative.

4.4.5 Alternative IS - 5: Purchase Water from Woods Creek WSC PWS

This alternative would require constructing a pipeline from Woods Creek WSC PWS to the Indian Springs PWS. A pump station would be required to overcome pipe friction and the elevation differences between Woods Creek and the Indian Springs PWS, and the current storage tank and feed pump set would be utilized at the Indian Springs PWS site. The required pipeline would be constructed of 6-inch pipe and would follow Midway Cut Thru, Highway 190, Ole Long Pull Road, the Ole Don Road, and James Boulevard to the Indian

Springs PWS. Using this route, shown in Figure 4.1, the pipeline required would be approximately 8.0 miles long. The pipeline would terminate at the existing storage tanks.

The pump station would include two pumps (minimum 20 hp each), one of the pumps is a standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Indian Springs Lake Estates Subdivision, since the incremental cost would be relatively small, and it would provide operational flexibility.

This alternative has limited opportunity for regionalization in that Lake Livingston WSSSC could possibly turn over provision of drinking water to the Woods Creek PWS instead of installing its own new well. Other non-compliant systems have not been identified near the Indian Springs PWS or along the pipeline route, so there is little chance to share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing the pipeline, pump station, and storage tank and feed pump set. The estimated O&M cost for this alternative are related to maintenance cost for the pipeline, and power and O&M labor and materials for the pump station, storage, and feed pumps. The estimated capital cost for this alternative is $2.52 million, and the alternatives' estimated annual O&M cost is $77,328.

The reliability of adequate amounts of compliant water under this alternative should be good. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pumps stations is well understood, and Lake Livingston WSSSC currently operates pumps and wells.

The feasibility of this alternative is dependent on an agreement being reached with the Woods Creek PWS to install a well in their well field.

4.4.6 Alternative IS - 6: New Well at 10 Miles

This alternative consists of installing one new well within 10 miles of the Indian Springs PWS that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 378-foot well, a new pump station with storage tank near the new well, and a pipeline from the new well/tank to existing storage tanks. The pump station and storage tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be 10 miles long, and would be a 6-inch line that discharges to the existing storage tanks at the Indian Springs PWS. The pump station would include two pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes installing the well, and constructing the pipeline, pump station, and storage tank and feed pumps. The estimated O&M cost for this alternative includes O&M for the pipeline and pump station. The estimated capital cost for this alternative is $3.13 million, and the estimated annual O&M cost for this alternative is $15,674.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of the Lake Livingston WSSSC, this alternative would be similar to operate as the existing system. Lake Livingston WSSSC personnel have experience with O&M of wells, pipelines and pump stations.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is possible an alternate groundwater source could not be found on land owned by the Lake Livingston WSSSC, so landowner cooperation would likely be required at the new location.

4.4.6 Alternative IS - 7: New Well at 5 Miles

This alternative consists of installing one new well within 5 miles of the Indian Springs PWS that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 378-foot well, a new pump station with storage tank near the new well, and a pipeline from the new well/tank to the existing storage tanks. The pump station and storage tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be 5 miles long, and would be a 6-inch line that discharges to the existing storage tanks at the Indian Springs PWS. The pump station would include two pumps, including one standby, and would be housed in a building.

Depending on well location and capacity, this alternative could present some options for a more regional solution. It may be possible to share water and costs with another nearby system.

The estimated capital cost for this alternative includes installing the well, and constructing the pipeline, pump station, and storage tank and feed pumps. The estimated O&M cost for this alternative includes O&M for the pipeline and pump station. The estimated capital cost for this alternative is $1.50 million, and the estimated annual O&M cost savings for this alternative is $1,807. One new well would be cheaper to operate than the two existing wells.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of the Lake Livingston WSSSC, this alternative would be similar to operate as the existing system. Lake Livingston WSSSC personnel have experience with O&M of wells, pipelines and pump stations.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is possible an alternate groundwater source could not be found on land owned by the Lake Livingston WSSSC, so landowner cooperation would likely be required at the new location.

4.4.7 Alternative IS - 8: New Well at 1 Mile

This alternative consists of installing one new well within 1 mile of the Indian Springs PWS that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify an existing well or the location where a new well could be installed.

This alternative would require constructing one new 378-foot well, well pump, and a pipeline from the new well to the existing storage tanks. For this alternative, the pipeline is assumed to be 1 miles long, 6-inch line that discharges to the existing storage tanks at the Indian Springs PWS.

The estimated capital cost for this alternative includes installing the well and constructing the pipeline, pump station, and storage tank and feed pumps. The estimated O&M cost for this alternative includes O&M for the pipeline. The estimated capital cost for this alternative is $389,956, and the estimated annual O&M cost savings for this alternative is $5,753.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells and pipelines are commonly employed. From the perspective of the Lake Livingston WSSSC, this alternative would be similar to operate as the existing system. Lake Livingston WSSSC personnel have experience with O&M of wells and pump stations.

The feasibility of this alternative is dependent on the ability to find an adequate existing well or success in installing a well that produces an adequate supply of compliant water. It is possible an alternate groundwater source could not be found on land owned by Lake Livingston WSSSC. Otherwise, landowner cooperation would likely be required at the new location.

4.4.8 Alternative IS - 9: Central IX Treatment

Two individual central treatment plants would be required for Indian Springs (ISLE). The No. 1 system would continue to pump water from the ISLE Well No. 1, and would treat the water through an IX system prior to distribution to the lower distribution system. The No. 2 system would continue to pump water from ISLE Wells No. 2 and 3, and would treat the water through an IX system prior to distribution to the upper distribution. For this option, the entire flow of the raw water will be treated to obtain compliant water. Water in excess of that currently produced would be required for backwashing and regeneration of the resin beds.

The No. 1 IX treatment plant, located at the ISLE Well No. 1 sites, features a 400 ft^2 building with a paved driveway; the pre-constructed IX equipment on a skid, a 24” x 50” commercial brine drum with regeneration equipment, two transfer pumps, a 5,000-gallon tank for storing the treated water, a 2,000-gallon tank for storing spent backwash water, and a 2,000 gallon tank for storing regenerant waste. The spent backwash water would be discharged to the sewer at a controlled rate. The regenerant waste would be trucked off-site for disposal. The treated water would be chlorinated and stored in the new treated water tanks prior to being pumped into the distribution system. The entire facilities are fenced. The No. 2 IX treatment plant, located near the ISLE Well No. 3 site, features a 400 ft^2 building with a paved driveway; the pre-constructed IX equipment on a skid, a 48” x 80” commercial brine drum with regeneration equipment, two transfer pumps, a 10,000-gallon tank for storing the treated water, a 5000-gallon tank for storing spent backwash water, and a 10,000-gallon tank for storing regenerant waste. The rest of the operations are similar to that of the No. 1 plant.

The estimated total capital cost for this alternative is $772,415, and the estimated total annual O&M cost is $82,680.

Reliability of supply of adequate amounts of compliant water under this alternative is good, since IX treatment is a common and well-understood treatment technology. IX treatment does not require high pressure, but can be affected by interfering constituents in the water. The O&M efforts required for the central IX treatment plant may be significant, and operating personnel would require training with ion exchange.

4.4.9 Alternative IS - 10: WRT Z-88 Treatment

Two individual WRT Z-88 systems would be required for this alternative – one for ISLE Well No. 1 and another for ISLE Wells No. 2 and 3. The systems would continue to pump water from the wells, and would treat the water through the Z-88 adsorption systems prior to distribution. The full flow of raw water would be treated by the Z-88 system as the media specifically adsorb radium and do not affect other constituents. There is no liquid waste generated in this process. The Z-88 media would be replaced and disposed of by WRT in an approved low-level radioactive waste landfill after 1-2 years of operation.

This alternative consists of constructing two Z-88™ treatment systems at the existing ISLE Well Nos. 1 and 3 sites. WRT owns the Z-88™ equipment and the water company pays for the installation of the system and auxiliary facilities and initial setup fees of $72,000 and $73,000 for the two systems. Each plant comprises a 400 ft^2 building with a paved driveway; the pre-constructed Z-88 adsorption system (2- 28” diameter x 115” tall vessels for Well No. 1 and 2- 64” diameter x 115” tall vessels for Well Nos. 2 and 3) owned by WRT; and piping system. The entire facility is fenced. The treated water will be chlorinated prior to distribution. It is assumed the well pumps have adequate pressure to pump the water through the Z-88 system and to the distribution system without requiring new pumps.

The estimated capital cost for this alternative is \$614,510 and the annual O&M cost is estimated to be \$97,398.

Based on many pilot testing results and some full-scale plant data this technology appears to be reliable. It is very simple to operate and the media replacement and disposal would be handled by WRT. Because WRT owns the equipment the capital cost is relatively low. The main operating cost is the treated water fee charged by WRT. One concern with this technology is the potential health effect of the level of radioactivity accumulated in the Z-88™ vessel on O&M personnel when the media have been operating for a long time.

4.4.10 Alternative IS - 11: KMnO4-Greensand Filtration

This alternative includes installing two individual KMnO4-green filtration systems – one for ISLE Well No. 1 and another for ISLE Wells No. 2 and 3. The systems would continue to pump water from the existing wells, and would treat the water through two individual greensand filter systems prior to distribution. For this option, the entire flow of the raw water will be treated and the flow will be decreased when one of the two 50 percent filters is being backwashed by raw water. It is assumed the existing well pumps have adequate pressure to pump the water through the greensand filters and to the distribution system.

The two greensand plants, one located at ISLE Well No. 1 and the other at Well No. 3 sites, each features a 400 ft^2 building with a paved driveway; the pre-constructed filters and a $KMnO_4$ solution tank on a skid; a 3,000 gallon spent backwash tank for Well No. 1 and a 10,000 gallon spent backwash tank for Well No. 3, and piping systems. The spent backwash water will be discharged to the sewer at a controlled rate. The entire facility is fenced.

The estimated total capital cost for this alternative is \$906,395 and the total annual O&M is estimated to be \$110,808.

Reliability of supply of adequate amounts of compliant water under this alternative is good, since $KMnO_4$-greensand is an established treatment technology for radium removal. The O&M efforts required is moderate and the operating personnel needs to ensure that $KMnO_4$ is not overfed. The spent backwash water contains MnO_2 particles with sorbed radium and the level of radioactivity in the backwash is relatively low.

4.4.11 Alternative IS - 12: Point-of-Use Treatment

This alternative consists of the continued operation of the Indian Springs PWS well, plus treatment of water to be used for drinking or food preparation at the point POU to remove radium and alpha particle activity. The purchase, installation, and maintenance of POU treatment systems to be installed “under the sink” would be necessary for this alternative. Blending is not an option in this case.

This alternative would require installing the POU treatment units in residences and other buildings that provide drinking or cooking water. Lake Livingston WSSSC staff would be responsible for purchase and maintenance of the treatment units, including media or membrane and filter replacement, periodic sampling, and necessary repairs. In houses, the most convenient point for installation of the treatment units is typically under the kitchen sink, with a separate tap installed for dispensing treated water. Installation of the treatment units in kitchens will require the entry of Lake Livingston WSSSC or contract personnel into the houses of customers. As a result, cooperation of customers would be important for success implementing this alternative. The treatment units could be installed so they could be accessed without house entry, but that would complicate the installation and increase costs.

For the cost estimate, it is assumed the POU radium and alpha particle activity treatment would involve RO. RO treatment processes typically produce a reject water stream that requires disposal. The reject stream results in an increase in the overall volume of water used. POU systems have the advantage of using only a minimum volume of treated water for human consumption. This minimizes the size of the treatment units, the water required for treatment, and the quantity of waste for disposal. For this alternative, it is assumed the increase in water consumption is insignificant in terms of supply cost, and that the reject waste stream could be discharged to the house septic or sewer system.

This alternative does not present options for a shared solution.

The estimated capital cost for this alternative includes the cost to purchase and install the POU treatment systems. The estimated O&M cost for this alternative includes the purchase and replacement of filters and media or membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is $237,600, and the estimated annual O&M cost for this alternative is $210,996. For the cost estimate, it is assumed that one POU treatment unit would be required for each of the 360 connections in the Indian Springs PWS.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of the customers for system installation, use, and maintenance, and only provides compliant water to single tap within a house. Additionally, the O&M efforts required for the POU systems will be significant, and the current personnel are inexperienced in this type of work. From the perspective of the Lake Livingston WSSSC this alternative would be characterized as more difficult to operate owing to the in-home requirements and the large number of individual units. It should be noted that POU treatment units would need to be more complex than units typically found in commercial retail outlets in order to meet regulatory requirements, making purchase and installation more expensive.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.12 Alternative IS - 13: Point-of-Entry Treatment

This alternative consists of the continued operation of the Indian Springs PWS well, plus treatment of water as it enters residences to remove radium and alpha particle activity. The purchase, installation, and maintenance of the treatment systems at the point of entry to a household would be necessary for this alternative. Blending is not an option in this case.

This alternative would require the installation of the POE treatment units at houses and other buildings that provide drinking or cooking water. Indian Springs would be responsible for purchasing and maintaining the treatment units, including media or membrane and filter replacement, periodic sampling, and necessary repairs. It may also be desirable to modify piping so water for non-consumptive uses can be withdrawn upstream of the treatment unit. The POE treatment units would be installed outside the residences, so entry would not be necessary for O&M. Some cooperation from customers would be necessary for installation and maintenance of the treatment systems.

For the cost estimate, it is assumed the POE radium and alpha particle activity treatment would involve RO. RO treatment processes typically produce a reject water stream that requires disposal. The waste streams result in an increased overall volume of water used. POE systems treat a greater volume of water than POU systems. For this alternative, it is assumed the increase in water consumption is insignificant in terms of supply cost, and that the reject waste stream could be discharged to the house septic or sewer system.

This alternative does not present options for a shared solution.

The estimated capital cost for this alternative includes cost to purchase and install the POE treatment systems. The estimated O&M cost for this alternative includes the purchase and replacement of filters and media or membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is $4,16 million, and the estimated annual O&M cost for this alternative is $489,996. For the cost estimate, it is assumed that one POU treatment unit would be required for each of the 360 connections in the Indian Springs PWS.

The reliability of adequate amounts of compliant water under this alternative are fair, but better than POU systems since it relies less on the active cooperation of the customers for system installation, use, and maintenance, and compliant water is supplied to all taps within a house. Additionally, the O&M efforts required for the POE systems will be significant, and the current personnel are inexperienced in this type of work. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as more difficult to operate owing to the on-property requirements and the large number of individual units.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.13 Alternative IS - 14: Public Dispenser for Treated Drinking Water

This alternative consists of the continued operation of the Indian Springs PWS well, plus dispensing treated water for drinking and cooking at a publicly accessible location. Implementing this alternative would require purchasing and installing a treatment unit where customers would be able to come and fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of water requires treatment, but customers would be required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

Lake Livingston WSSSC personnel would be responsible for maintenance of the treatment unit, including media or membrane replacement, periodic sampling, and necessary repairs. The spent media or membranes will require disposal. This alternative relies on a great deal of cooperation and action from the customers to be effective.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the treatment system to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes purchasing and replacing filters and media or membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is $34,800, and the estimated annual O&M cost for this alternative is $45,690.

The reliability of adequate amounts of compliant water under this alternative is fair, because of the large amount of effort required from the customers and the associated inconvenience. Lake Livingston WSSSC has not provided this type of service in the past. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as relatively easy to operate, since these types of treatment units are highly automated, and there is only one unit.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.14 Alternative IS - 15: 100 Percent Bottled Water Delivery

This alternative consists of the continued operation of the Indian Springs PWS well, but compliant drinking water will be delivered to customers in containers. This alternative involves setting up and operating a bottled water delivery program to serve all of the customers in the system. It is expected that Lake Livingston WSSSC would find it most convenient and economical to contract a bottled water service. The bottle delivery program would have to be flexible enough to allow the delivery of smaller containers should customers be incapable of lifting and manipulating 5-gallon bottles. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

This alternative does not involve capital cost for construction, but would require some initial costs for system setup, and then ongoing costs to have the bottled water furnished. It is assumed for this alternative that bottled water is provided to 100 percent of the Indian Springs PWS customers.

This alternative does not present options for a regional solution.

The estimated initial capital cost is for setting up the program. The estimated O&M cost for this alternative includes program administration and purchase of the bottled water. The estimated capital cost for this alternative is $20,836, and the estimated annual O&M cost for this alternative is $651,972. For the cost estimate, it is assumed that each person requires one gallon of bottled water per day.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of customers to order and utilize the water. Management and administration of the bottled water delivery program will require attention from Lake Livingston WSSSC.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.15 Alternative IS - 16: Public Dispenser for Trucked Drinking Water

This alternative consists of continued operation of the Indian Springs PWS well, plus dispensing compliant water for drinking and cooking at a publicly accessible location. The compliant water would be purchased from the City of Livingston, and delivered by truck to a tank at a central location where customers would be able to fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of water requires treatment, but customers are required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

Lake Livingston WSSSC would purchase a truck that would be suitable for hauling potable water, and install a storage tank. It is assumed the storage tank would be filled once a week, and that the chlorine residual would be tested for each truckload. The truck would have to meet requirements for potable water, and each load would be treated with bleach. This alternative relies on a great deal of cooperation and action from the customers for it to be effective.

This alternative presents limited options for a regional solution if two or more systems share the purchase and operation of the water truck.

The estimated capital cost for this alternative includes purchasing a water truck and construction of the storage tank to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes O&M for the truck, maintenance for the tank, water quality testing, record keeping, and water purchase. The estimated capital cost for this alternative is $134,959, and the estimated annual O&M cost for this alternative is $44,924.

The reliability of adequate amounts of compliant water under this alternative is fair because of the large amount of effort required from the customers and the associated inconvenience. Current personnel have not provided this type of service in the past. From the perspective of the Lake Livingston WSSSC, this alternative would be characterized as relatively easy to operate, but the water hauling and storage would have to be done with care to ensure sanitary conditions.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.4.16 Summary of Alternatives

Table 4.3 provides a summary of the key features of each alternative for Indian Springs PWS.

1 **Table 4.3 Summary of Compliance Alternatives for Indian Springs PWS**

Alt No.	Alternative Description	Major Components	Capital Cost[1]	Annual O&M Cost	Total Annualized Cost	Reliability	System Impact	Remarks
IS -1	New well at Indian Springs PWS	- New well	$ 102,396	($ 5,942)	$ 2,985	Good	N	New well at the same location set at a different depth. Sharing cost with neighboring systems is unlikely. Blending may be possible.
IS -2	Purchase water from Soda WSC	- Pump station - 3.1-mile pipeline	$ 1,654,790	$ 75,003	$ 219,697	Good	N	Agreement must be successfully negotiated with Soda Water Supply Company. Sharing cost with neighboring systems may be possible. Blending may be possible.
IS -3	Purchase water from Wilson Lake Estates WSC	- Pump station - 2.6-mile pipeline	$ 1,463,561	$ 75,003	$ 202,697	Good	N	Agreement must be successfully negotiated with Beech Creek Village LL PWS, owned by Lake Livingston Water Supply & Sewer Service Corp. Blending may be possible.
IS -4	New well at Wilson Lake Estates WSC	- New well - Storage tank - Pump station - 3.3-mile pipeline	$ 1,551,866	$ 11,161	$ 146,460	Good	N	Agreement must be successfully negotiated with Providence Water Supply Company. Sharing cost with neighboring systems may be possible. Blending may be possible.
IS -5	Purchase water from Woods Creek PWS	- Pump station - 6.9-mile pipeline	$ 2,521,352	$ 15,674	$ 297,151	Good	N	Agreement must be successfully negotiated with City of Livingston. Blending may be possible.
IS -6	Install new compliant well within 10 miles	- New well - Storage tank - Pump station - 10-mile pipeline	$ 3,126,614	$ 15,674	$ 288,267	Good	N	There is good probability for finding good quality groundwater. Costs could possibly be shared with small systems along pipeline route.
IS -7	Install new compliant well within 5 miles	- New well - Storage tank - Pump station - 5-mile pipeline	$ 1,497,234	($ 1,807)	$ 128,729	Good	N	There is good probability for finding good quality groundwater. Costs could possibly be shared with small systems along pipeline route.
IS -8	Install new compliant well within 1 mile	- New well - 1-mile pipeline	$ 389,956	($ 5,753)	$ 28,245	Good	N	There is good probability for finding good quality groundwater.
IS -9	Continue operation of Indian Springs PWS well field with central IX treatment	- Central IX treatment plant	$ 772,415	$ 85,984	$ 153,327	Good	T	Costs could possibly be shared with nearby small systems.
IS -10	Continue operation of Indian Springs PWS well field with central WRT Z-88 treatment	- Central WRT Z-88 treatment plant	$ 614,510	$ 97,389	$ 105,965	Good	T	Costs could possibly be shared with nearby small systems.
IS -11	Continue operation of Indian Springs PWS well field with central	- Central $KMnO_4$ treatment plant	$ 906,395	$ 110,808	$ 189,832	Good	T	Costs could possibly be shared with nearby small systems.

Alt No.	Alternative Description	Major Components	Capital Cost[1]	Annual O&M Cost	Total Annualized Cost	Reliability	System Impact	Remarks
	$KMnO_4$ treatment							
IS -12	Continue operation of Indian Springs PWS well field, and POU treatment	- POU treatment units.	$ 237,600	$ 210,996	$ 231,711	Fair	T, M	Only one compliant tap in home. Cooperation of residents required for installation, maintenance, and testing.
IS -13	Continue operation of Indian Springs PWS well field, and POE treatment	- POE treatment units.	$ 4,158,000	$ 489,996	$ 852,509	Fair (*better than POU*)	T, M	All home taps compliant and less resident cooperation required.
IS -14	Continue operation of Indian Springs PWS well field, but furnish public dispenser for treated drinking water	- Water treatment and dispenser unit	$ 34,800	$ 45,690	$ 48,724	Fair/interim measure	T	Does not provide compliant water to all taps, and requires a lot of effort by customers.
IS -15	Continue operation of Indian Springs PWS well field, but furnish bottled drinking water for all customers	- Set up bottled water system	$ 20,836	$ 651,972	$ 653,788	Fair/interim measure	M	Does not provide compliant water to all taps, and requires customers to order and use. Management of program may be significant.
IS-16	Continue operation of Indian Springs PWS well field, but furnish public dispenser for trucked drinking water.	- Construct storage tank and dispenser - Purchase potable water truck	$ 134,959	$ 44,924	$ 56,691	Fair/interim measure	M	Does not provide compliant water to all taps, and requires a lot of effort by customers.

Notes: *N – No significant increase required in technical or management capability*
T – Implementation of alternative will require increase in technical capability
M – Implementation of alternative will require increase in management capability
1 – See cost breakdown in Appendix C
2 – 20-year return period and 6 percent interest

4.6 COST OF SERVICE AND FUNDING ANALYSIS

To evaluate the financial impact of implementing the compliance alternatives, a 30-year financial planning model was developed. This model can be found in Appendix D. The financial model is based on estimated cash flows, with and without implementation of the compliance alternatives. Data for such models are typically derived from established budgets, audited financial reports, published water tariffs, and consumption data. Information that was available to complete the financial analysis on the Indian Springs PWS included the 2005 Consolidated Financial Statement for the parent company Lake Livingston WSSSC with combined revenues and expenses for all of the 52 PWSs it manages. Also evaluated were the "Capacity Assessment" document prepared after conducting interviews with the Lake Livingston WSSSC personnel, and the Water Usage Rates provided by the parent company. Indian Springs PWS customers use an average of 360 gpd per connection.

This analysis will need to be performed in a more detailed fashion and applied to alternatives that are deemed attractive and worthy of more detailed evaluation. A more detailed analysis should include additional factors such as:

- Cost escalation,
- Price elasticity effects where increased rates may result in lower water consumption,
- Costs for other system upgrades and rehabilitation needed to maintain compliant operation.

4.6.1 Financial Plan Development

Since Lake Livingston WSSSC does not keep separate financial records for each of the 52 PWSs it manages, the revenues and expenses for Indian Springs PWS had to be estimated. Total revenues and expenses for the PWS were obtained from a consolidated 2005 Income and Expense statement. The annual revenue for Indian Springs PWS was estimated based on its percentage water usage of 15.5 percent as shown in Table 4.4 below. The resultant 2005 annual revenue of $517,990 was entered into the financial model and is presented in Table 4.4 for comparison with the other Lake Livingston WSSSC systems.

Table 4.4 Summary of Lake Livingston WSSSC 2005 Estimated Water Revenues

PWS Name	2005 Water Usage (gallons)	% of Total Water Usage	No. Connections	2005 Water Revenues
Indian Springs Lake Estates	43,304,000	15.5 %	360	$517,990
Paradise Acres	21,626,250	7.1 %	395	$236,812
Crystal Lake Estates	7,719,750	2.5 %	93	$84,533
Other Water Systems	232,308,000	74.9 %	6,052	2,500,021
Total	304,958,000	100 %	6,900	$3,339,356

Annual expenses for Indian Springs PWS were estimated based on its percentage water usage of 15.5 percent as shown in Table 4.4. In 2005, the consolidated financial statement provided by Lake Livingston WSSSC lists the total operating expenses as $2,418,031. The resultant total expenses for Indian Springs PWS amount to $374,795, leaving a surplus of $143,195 after expenses.

4.6.2 Current Financial Condition

4.6.2.1 Cash Flow Needs

Using the estimated water usage rates as noted above, the current average annual water bill for Indian Springs PWS customers is estimated to be $1,439 or about 5.8 percent of the Block Group Tract MHI of $24,706

The 2005 estimated annual water sales revenues for the Indian Springs PWS are greater than the operating expenses. Lake Livingston WSSSC's 2005 consolidated financial report also indicates that it has a cash reserve of $1,434,450, which based on current expenditures, is sufficient to maintain operations for 7 months for all the 52 PWSs it manages. However, in an effort to maintain its reserve fund, Lake Livingston WSSSC may elect to raise rates to offset the expenditures for any capital improvements necessary to address the water quality issues concerning arsenic.

4.6.2.2 Ratio Analysis

Current Ratio= 2.28

The Current Ratio is a measure of liquidity. A Current Ratio of 2.28 indicates that the Lake Livingston WSSSC would be able to meet all of its current obligations, with total current assets of $1,188,583 exceeding total current liabilities of $520,782.

Debt to Net Worth Ratio=1.43

A Debt to Net Worth ratio is another measure of financial liquidity and stability. Lake Livingston WSSSC has a Net Worth of $4,741,473 and a debt total of $6,803,965 resulting in a Debt to Net Worth ratio of 1.43. Ratios less than 1.25 are indicative of financial stability, with lower ratios indicating greater financial stability and better credit risks for future borrowings. Based on the present ratio, Lake Livingston WSSSC could be perceived as a slight credit risk which may make it difficult to obtain financing for water improvement projects at competitive interest rates.

Operating Ratio = 1.38

In 2005 the Lake Livingston WSSSC had operating revenues of $3,339,356 and operating expenses of $2,418,031 resulting in an Operating Ratio equal to 1.38. Thus, in fiscal year 2005 the operating revenues were more than sufficient to cover the operating expenses, and resulted in a surplus income of $921,325.

4.6.3 Financial Plan Results

Each of the compliance alternatives for the Lake Livingston WSSSC was evaluated using the financial model to determine the overall increase in water rates that would be necessary to pay for the improvements. Each alternative was examined under the various funding options described in Subsection 2.4.

For State Revolving Fund (SRF) funding options, customer MHI compared to the state average determines the availability of subsidized loans. According the 2000 U.S. Census data, the Block Group MHI for customers of Indian Springs PWS was $24,706, which is 62 percent of the statewide income average of $39,927. As a result, Lake Livingston WSSSC would qualify for a 0 percent interest loan from the SRF. In the event SRF funds would be unavailable, Lake Livingston WSSSC would need to rely on revenue bonds as a funding alternative.

Results of the financial impact analysis are provided in Table 4.5 and Figure 4.2. Table 4.5 presents rate impacts assuming that any deficiencies in reserve accounts are funded immediately in the year following the occurrence of the deficiency, which would cause the first few years' water rates to be higher than they would be if the reserve account was built-up over a longer period of time. Figure 4.2 provides a bar chart that, in terms of the yearly billing to an average customer, shows the following:

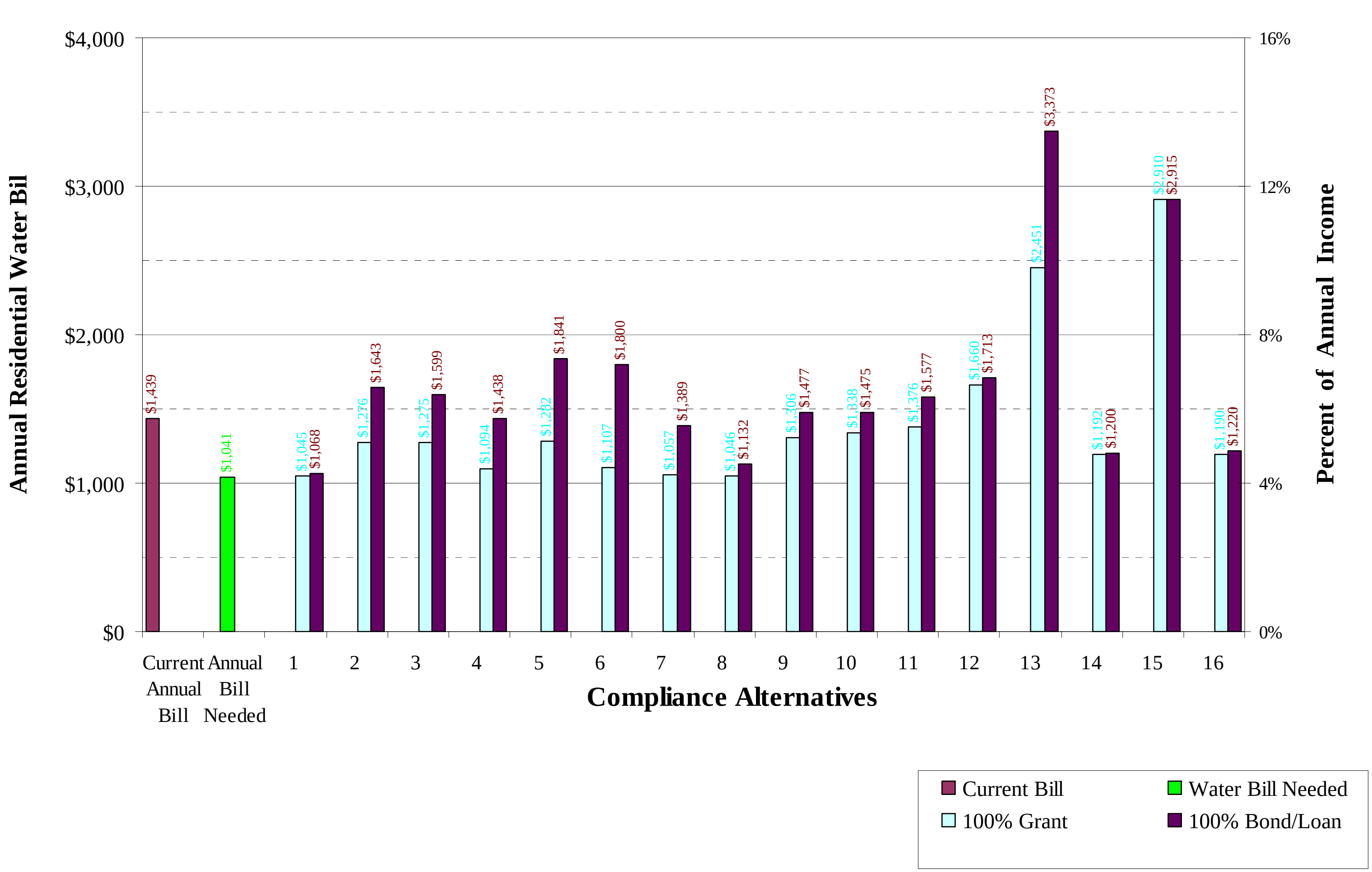
- Current annual average bill,
- Projected annual average bill including rate increase, if needed, to match existing expenditures, and
- Projected annual bill including rate increases needed to fund implementation of a compliance alternative (this does not include funding for reserve accounts).

The two bars shown for each compliance alternative represent the rate changes necessary for revenues to match total expenditures assuming 100 percent grant funding and 100 percent loan/bond funding. Most funding options will fall between 100 percent grant and 100 percent loan/bond funding, with the exception of 100 percent revenue financing. Establishing or increasing reserve accounts would require an increase in rates. If existing reserves are insufficient to fund a compliance alternative, rates would need to be raised before implementing the compliance alternative. This would allow for accumulation of sufficient reserves to avoid larger but temporary rate increases during the years the compliance alternative was being implemented.

1 **Table 4.5 Financial Impact on Households**

Alternative	Description		All Revenue	100% Grant	75% Grant	50% Grant	SRF	Bond
1	New Well at Indian Springs	Max % of HH Income	6%	6%	6%	6%	6%	6%
		Max % Rate Increase Compared to Current	0%	0%	0%	0%	0%	0%
		Average Water Bill Required by Alternative	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439
2	Purchase Water from Soda WS	Max % of HH Income	22%	7%	7%	8%	9%	10%
		Max % Rate Increase Compared to Current	271%	16%	28%	41%	48%	67%
		Average Water Bill Required by Alternative	$ 5,084	$ 1,581	$ 1,749	$ 1,918	$ 2,012	$ 2,255
3	Purchase Water from Wilson Lake Estates	Max % of HH Income	19%	7%	7%	8%	8%	9%
		Max % Rate Increase Compared to Current	233%	15%	27%	38%	44%	61%
		Average Water Bill Required by Alternative	$ 4,575	$ 1,578	$ 1,728	$ 1,877	$ 1,960	$ 2,175
4	New Well at Wilson Lake Estates	Max % of HH Income	20%	6%	6%	6%	7%	8%
		Max % Rate Increase Compared to Current	243%	0%	0%	10%	17%	34%
		Average Water Bill Required by Alternative	$ 4,709	$ 1,439	$ 1,439	$ 1,520	$ 1,608	$ 1,836
5	Purchase Water from Woods Creek WS	Max % of HH Income	32%	7%	8%	9%	10%	11%
		Max % Rate Increase Compared to Current	442%	17%	36%	55%	66%	94%
		Average Water Bill Required by Alternative	$ 7,387	$ 1,592	$ 1,849	$ 2,106	$ 2,249	$ 2,620
6	New Well at 10 Miles	Max % of HH Income	38%	6%	7%	8%	9%	11%
		Max % Rate Increase Compared to Current	553%	0%	12%	36%	50%	85%
		Average Water Bill Required by Alternative	$ 8,884	$ 1,439	$ 1,549	$ 1,868	$ 2,045	$ 2,505
7	New Well at 5 Miles	Max % of HH Income	19%	6%	6%	6%	6%	7%
		Max % Rate Increase Compared to Current	232%	0%	0%	0%	10%	26%
		Average Water Bill Required by Alternative	$ 4,564	$ 1,439	$ 1,439	$ 1,439	$ 1,518	$ 1,738
8	New Well at 1 Mile	Max % of HH Income	7%	6%	6%	6%	6%	6%
		Max % Rate Increase Compared to Current	14%	0%	0%	0%	0%	0%
		Average Water Bill Required by Alternative	$ 1,628	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439
9	Central Treatment - IX	Max % of HH Income	12%	7%	7%	8%	8%	8%
		Max % Rate Increase Compared to Current	100%	21%	26%	32%	36%	44%
		Average Water Bill Required by Alternative	$ 2,775	$ 1,643	$ 1,722	$ 1,800	$ 1,844	$ 1,958
10	Central Treatment - WRT Z-88	Max % of HH Income	10%	7%	8%	8%	8%	8%
		Max % Rate Increase Compared to Current	71%	26%	31%	35%	38%	45%
		Average Water Bill Required by Alternative	$ 2,391	$ 1,710	$ 1,772	$ 1,835	$ 1,870	$ 1,960
11	Central Treatment - KMnO4	Max % of HH Income	13%	8%	8%	8%	9%	9%
		Max % Rate Increase Compared to Current	132%	32%	39%	46%	50%	60%
		Average Water Bill Required by Alternative	$ 3,205	$ 1,788	$ 1,881	$ 1,973	$ 2,025	$ 2,158
12	Point-of-Use Treatment	Max % of HH Income	10%	10%	10%	11%	11%	11%
		Max % Rate Increase Compared to Current	78%	78%	80%	82%	83%	85%
		Average Water Bill Required by Alternative	$ 2,420	$ 2,376	$ 2,400	$ 2,425	$ 2,438	$ 2,473
13	Point-of-Entry Treatment	Max % of HH Income	56%	18%	20%	22%	23%	25%
		Max % Rate Increase Compared to Current	859%	206%	238%	270%	288%	334%
		Average Water Bill Required by Alternative	$ 12,959	$ 4,013	$ 4,437	$ 4,861	$ 5,097	$ 5,708
14	Public Dispenser for Treated Drinking Water	Max % of HH Income	6%	6%	6%	6%	6%	6%
		Max % Rate Increase Compared to Current	0%	0%	0%	0%	0%	0%
		Average Water Bill Required by Alternative	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439
15	Supply Bottled Water to 100% of Population	Max % of HH Income	22%	22%	22%	22%	22%	22%
		Max % Rate Increase Compared to Current	281%	281%	281%	281%	281%	281%
		Average Water Bill Required by Alternative	$ 4,967	$ 4,963	$ 4,966	$ 4,968	$ 4,969	$ 4,972
16	Central Trucked Drinking Water	Max % of HH Income	6%	6%	6%	6%	6%	6%
		Max % Rate Increase Compared to Current	0%	0%	0%	0%	0%	6%
		Average Water Bill Required by Alternative	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,439	$ 1,457

Figure 4-2 Alternative Cost Summary



SECTION 5
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APPENDIX A
PWS INTERVIEW FORM

CAPACITY DEVELOPMENT ASSESSMENT FORM

*Prepared By*___________________________________ *Date*___________________________

Section 1. Public Water System Information

1. PWS ID # [ ] 2. Water System Name [ ]

3. County [ ]

4. Owner [ ] Address [ ]
   Tele. [ ] E-mail [ ]
   Fax [ ] Message [ ]

5. Admin [ ] Address [ ]
   Tele. [ ] E-mail [ ]
   Fax [ ] Message [ ]

6. Operator [ ] Address [ ]
   Tele. [ ] E-mail [ ]
   Fax [ ] Message [ ]

7. Population Served [ ] 8. No. of Service Connections [ ]

9. Ownership Type [ ] 10. Metered (Yes or No) [ ]

11. Source Type [ ]

12. Total PWS Annual Water Used [ ]

13. Number of Water Quality Violations (Prior 36 months)

    Total Coliform [ ] Chemical/Radiological [ ]

    Monitoring (CCR, Public Notification, etc.) [ ] Treatment Technique, D/DBP [ ]

A. Basic Information

1. Name of Water System:

2. Name of Person Interviewed:

3. Position:

4. Number of years at job:

5. Number of years experience with drinking water systems:

6. Percent of time (day or week) on drinking water system activities, with current position (how much time is dedicated exclusively to the water system, not wastewater, solid waste or other activities):

7. Certified Water Operator (Yes or No):

   If Yes,
   7a. Certification Level (water):

   7b. How long have you been certified?

8. Describe your water system related duties on a typical day.

B. Organization and Structure

1. Describe the organizational structure of the Utility. Please provide an organizational chart. (Looking to find out the governance structure (who reports to whom), whether or not there is a utility board, if the water system answers to public works or city council, etc.)

2. If not already covered in Question 1, to whom do you report?

3. Do all of the positions have a written job description?

    3a. If yes, is it available to employees?

    3b. May we see a copy?

C. Personnel

1. What is the current staffing level (include all personnel who spend more than 10% of their time working on the water system)?

2. Are there any vacant positions? How long have the positions been vacant?

3. In your opinion, is the current staffing level adequate? If not adequate, what are the issues or staffing needs (how many and what positions)?

4. What is the rate of employee turnover for management and operators? What are the major issues involved in the turnover (e.g., operator pay, working conditions, hours)?

5. Is the system staffed 24 hours a day? How is this handled (on-site or on-call)? Is there an alarm system to call an operator if an emergency occurs after hours?

D. Communication

1. Does the utility have a mission statement? If yes, what is it?

2. Does the utility have water quality goals? What are they?

3. How are your work priorities set?

4. How are work tasks delegated to staff?

5. Does the utility have regular staff meetings? How often? Who attends?

6. Are there separate management meetings? If so, describe.

7. Do management personnel ever visit the treatment facility? If yes, how often?

8. Is there effective communication between utility management and state regulators (e.g., NMED)?

9. Describe communication between utility and customers.

E. Planning and Funding

1. Describe the rate structure for the utility.

2. Is there a written rate structure, such as a rate ordinance? May we see it?

    2a. What is the average rate for 6,000 gallons of water?

3. How often are the rates reviewed?

4. What process is used to set or revise the rates?

5. In general, how often are the new rates set?

6. Is there an operating budget for the water utility? Is it separate from other activities, such as wastewater, other utilities, or general city funds?

7. Who develops the budget, how is it developed and how often is a new budget created or the old budget updated?

8. How is the budget approved or adopted?

9. In the last 5 years, how many budget shortfalls have there been (i.e., didn't collect enough money to cover expenses)? What caused the shortfall (e.g., unpaid bills, an emergency repair, weather conditions)?

    9a. How are budget shortfalls handled?

10. In the last 5 years how many years have there been budget surpluses (i.e., collected revenues exceeded expenses?

    10a. How are budget surpluses handled (i.e., what is done with the money)?

11. Does the utility have a line-item in the budget for emergencies or some kind of emergency reserve account?

12. How do you plan and pay for short-term system needs?

13. How do you plan and pay for long- term system needs?

14. How are major water system capital improvements funded? Does the utility have a written capital improvements plan?

15. How is the facility planning for future growth (either new hook-ups or expansion into new areas)?

16. Does the utility have and maintain an annual financial report? Is it presented to policy makers?

17. Has an independent financial audit been conducted of the utility finances? If so, how often? When was the last one?

18. Will the system consider any type of regionalization with any other PWS, such as system interconnection, purchasing water, sharing operator, emergency water connection, sharing bookkeeper/billing or other?

F. Policies, Procedures, and Programs

1. Are there written operational procedures? Do the employees use them?

2. Who in the utility department has spending authorization? What is the process for obtaining needed equipment or supplies, including who approves expenditures?

3. Does the utility have a source water protection program? What are the major components of the program?

4. Are managers and operators familiar with current SDWA regulations?

5. How do the managers and operators hear about new or proposed regulations, such as arsenic, DBP, Groundwater Rule? Are there any new regulations that will be of particular concern to the utility?

6. What are the typical customer complaints that the utility receives?

7. Approximately how many complaints are there per month?

8. How are customer complaints handled? Are they recorded?

9. (If not specifically addressed in Question 7) If the complaint is of a water quality nature, how are these types of complaints handled?

10. Does the utility maintain an updated list of critical customers?

11. Is there a cross-connection control plan for the utility? Is it written? Who enforces the plan's requirements?

12. Does the utility have a written water conservation plan?

13. Has there been a water audit of the system? If yes, what were the results?

14. (If not specifically answered in 11 above) What is the estimated percentage for loss to leakage for the system?

15. Are you, or is the utility itself, a member of any trade organizations, such as AWWA or Rural Water Association? Are you an active member (i.e., attend regular meetings or participate in a leadership role)? Do you find this membership helpful? If yes, in what ways does it help you?

G. Operations and Maintenance

1. How is decision-making authority split between operations and management for the following items:

   a. Process Control

   b. Purchases of supplies or small equipment

   c. Compliance sampling/reporting

   d. Staff scheduling

2. Describe your utility's preventative maintenance program.

3. Do the operators have the ability to make changes or modify the preventative maintenance program?

4. How does management prioritize the repair or replacement of utility assets? Do the operators play a role in this prioritization process?

5. Does the utility keep an inventory of spare parts?

6. Where does staff have to go to buy supplies/minor equipment? How often?

   6a. How do you handle supplies that are critical, but not in close proximity (for example if chlorine is not available in the immediate area or if the components for a critical pump are not in the area)

7. Describe the system's disinfection process. Have you had any problems in the last few years with the disinfection system?

    7a. Who has the ability to adjust the disinfection process?

8. How often is the disinfectant residual checked and where is it checked?

    8a. Is there an official policy on checking residuals or is it up to the operators?

9. Does the utility have an O & M manual? Does the staff use it?

10. Are the operators trained on safety issues? How are they trained and how often?

11. Describe how on-going training is handled for operators and other staff. How do you hear about appropriate trainings? Who suggests the trainings – the managers or the operators? How often do operators, managers, or other staff go to training? Who are the typical trainers used and where are the trainings usually held?

12. In your opinion is the level of your on-going training adequate?

13. In your opinion is the level of on-going training for other staff members, particularly the operators, adequate?

14. Does the facility have mapping of the water utility components? Is it used on any routine basis by the operators or management? If so, how is it used? If not, what is the process used for locating utility components?

15. In the last sanitary survey, were any deficiencies noted? If yes, were they corrected?

16. How often are storage tanks inspected? Who does the inspection?

    16a. Have you experienced any problems with the storage tanks?

H. SDWA Compliance

1. Has the system had any violations (monitoring or MCL) in the past 3 years? If so, describe.

2. How were the violations handled?

3. Does the system properly publish public notifications when notified of a violation?

4. Is the system currently in violation of any SDWA or state regulatory requirements, including failure to pay fees, fines, or other administrative type requirements?

5. Does the utility prepare and distribute a Consumer Confidence Report (CCR)? Is it done every year? What type of response does the utility get to the CCR from customers?

I. Emergency Planning

1. Does the system have a written emergency plan to handle emergencies such as water outages, weather issues, loss of power, loss of major equipment, etc?

2. When was the last time the plan was updated?

3. Do all employees know where the plan is? Do they follow it?

4. Describe the last emergency the facility faced and how it was handled.

Attachment A

A. Technical Capacity Assessment Questions

1. Based on available information of water rights on record and water pumped has the system exceeded its water rights in the past year? YES ☐ NO ☐

   In any of the past 5 years? YES ☐ NO ☐ How many times? _____

2. Does the system have the proper level of certified operator? *(Use questions a – c to answer.)*
   YES ☐ NO ☐

   a. What is the Classification Level of the system by NMED? _____

   b. Does the system have one or more certified operator(s)? [20 NMAC 7.4.20]

   YES ☐ NO ☐

   c. If YES, provide the number of operators at each New Mexico Certification Level. [20 NMAC 7.4.12]

   _____NM Small System _____Class 2

   _____NM Small System Advanced _____Class 3

   _____Class 1 _____Class 4

3. Did the system correct any sanitary deficiency noted on the most recent sanitary survey within 6 months of receiving that information? [20 NMAC 7.20.504]

   YES ☐ NO ☐ No Deficiencies ☐

   What was the type of deficiency? *(Check all that are applicable.)*

   Source ☐ Storage ☐

   Treatment ☐ Distribution ☐

   Other _____

   From the system's perspective, were there any other deficiencies that were not noted on the sanitary survey? Please describe.

4. Will the system's current treatment process meet known future regulations?

   Radionuclides YES ☐ NO ☐ Doesn't Apply ☐

   Arsenic YES ☐ NO ☐ Doesn't Apply ☐

   Stage 1 Disinfectants and Disinfection By-Product (DBP)

   YES ☐ NO ☐ Doesn't Apply ☐

   Surface Water Treatment Rule YES ☐ NO ☐ Doesn't Apply ☐

5. Does the system have a current site plan/map? [20 NMAC 7.10.302 A.1.]

   YES ☐ NO ☐

6. Has the system had a water supply outage in the prior 24 months?

YES ☐ NO ☐

What were the causes of the outage(s)? *(Include number of outages for each cause.)*

Drought _____ Limited Supply _____

System Failure _____ Other _____

7. Has the system ever had a water audit or a leak evaluation?

YES ☐ NO ☐ Don't Know ☐

If YES, please complete the following table.

Type of Investigation	Date Done	Water Loss (%)	What approach or technology was used to complete the investigation?	Was any follow-up done? If so, describe

8. Have all drinking water projects received NMED review and approval? [20 NMAC 7.10.201]

YES ☐ NO ☐

If NO, what types of projects have not received NMED review and approval.

Source ☐ Storage ☐

Treatment ☐ Distribution ☐

Other ☐ _____

9. What are the typical customer complaints that the utility receives?

10. Approximately how many complaints are there per month? _____

11. How are customer complaints handled? Are they recorded?

12. What is the age and composition of the distribution system? ***(Collect this information from the Sanitary Survey)***

Pipe Material	Approximate Age	Percentage of the system	Comments
			Sanitary Survey Distribution System Records Attached

13. Are there any dead end lines in the system?

YES ☐ NO ☐

14. Does the system have a flushing program?

YES ☐ NO ☐

If YES, please describe.

15. Are there any pressure problems within the system?

YES ☐ NO ☐

If YES, please describe.

16. Does the system disinfect the finished water?

YES ☐ NO ☐

If yes, which disinfectant product is used? ______

Interviewer Comments on Technical Capacity:

B. Managerial Capacity Assessment Questions

17. Has the system completed a 5-year Infrastructure Capital Improvement Plan (ICIP) plan?

YES ☐ NO ☐

If YES, has the plan been submitted to Local Government Division?

YES ☐ NO ☐

18. Does the system have written operating procedures?

YES ☐ NO ☐

19. Does the system have written job descriptions for all staff?

YES ☐ NO ☐

20. Does the system have:

A preventative maintenance plan?
YES ☐ NO ☐
A source water protection plan?
YES ☐ NO ☐ N/A ☐
An emergency plan?
YES ☐ NO ☐
A cross-connection control program?
YES ☐ NO ☐
An emergency source?
YES ☐ NO ☐
System security measures?
YES ☐ NO ☐

21. Does the system report and maintain records in accordance with the drinking water regulations concerning:

Water quality violations
YES ☐ NO ☐

Public notification
YES ☐ NO ☐

Sampling exemptions
YES ☐ NO ☐

22. Please describe how the above records are maintained:

23. Describe the management structure for the water system, including board and operations staff. Please include examples of duties, if possible.

24. Please describe type and quantity of training or continuing education for staff identified above.

25. Describe last major project undertaken by the water system, including the following: project in detail, positive aspects, negative aspects, the way in which the project was funded, any necessary rate increases, the public response to the project, whether the project is complete or not, and any other pertinent information.

26. Does the system have any debt? YES ☐ NO ☐

If yes, is the system current with all debt payments?
YES ☐ NO ☐

If no, describe the applicable funding agency and the default.

27. Is the system currently contemplating or actively seeking funding for any project?
YES ☐ NO ☐

If yes, from which agency and how much?

Describe the project?

Is the system receiving assistance from any agency or organization in its efforts?

28. Will the system consider any type of regionalization with other PWS? *(Check YES if the system has already regionalized.)*

YES ☐ NO ☐

If YES, what type of regionalization has been implemented/considered/discussed? *(Check all that apply.)*

System interconnection ☐
Sharing operator ☐
Sharing bookkeeper ☐
Purchasing water ☐
Emergency water connection ☐
Other: _____

29. Does the system have any of the following? *(Check all that apply.)*

Water Conservation Policy/Ordinance ☐ Current Drought Plan ☐
Water Use Restrictions ☐ Water Supply Emergency Plan ☐

Interviewer Comments on Managerial Capacity:

C. Financial Capacity Assessment

30. Does the system have a budget?

YES ☐ NO ☐

If YES, what type of budget?

Operating Budget ☐

Capital Budget ☐

31. Have the system revenues covered expenses and debt service for the past 5 years?

YES ☐ NO ☐

If NO, how many years has the system had a shortfall? _____

32. Does the system have a written/adopted rate structure?

YES ☐ NO ☐

33. What was the date of the last rate increase? _____

34. Are rates reviewed annually?

YES ☐ NO ☐

IF YES, what was the date of the last review? _____

35. Did the rate review show that the rates covered the following expenses? *(Check all that apply.)*

Operation & Maintenance ☐

Infrastructure Repair & replacement ☐

Staffing ☐

Emergency/Reserve fund ☐

Debt payment ☐

36. Is the rate collection above 90% of the customers?

YES ☐ NO ☐

37. Is there a cut-off policy for customers who are in arrears with their bill or for illegal connections?

YES ☐ NO ☐

If yes, is this policy implemented?

38. What is the residential water rate for 6,000 gallons of usage in one month. _____

39. In the past 12 months, how many customers have had accounts frozen or dropped for non-payment? _____

[Convert to % of active connections

Less than 1% ☐ 1% - 3% ☐ 4% - 5% ☐ 6% - 10% ☐

11% - 20% ☐ 21% - 50% ☐ Greater than 50% ☐]

40. The following questions refer to the process of obtaining needed equipment and supplies.

a. Can the water system operator buy or obtain supplies or equipment when they are needed?

YES ☐ NO ☐

b. Is the process simple or burdensome to the employees?

c. Can supplies or equipment be obtained quickly during an emergency?

YES ☐ NO ☐

d. Has the water system operator ever experienced a situation in which he/she couldn't purchase the needed supplies?

YES ☐ NO ☐

e. Does the system maintain some type of spare parts inventory?

YES ☐ NO ☐

If yes, please describe.

41. Has the system ever had a financial audit?

YES ☐ NO ☐

If YES, what is the date of the most recent audit? ______

42. Has the system ever had its electricity or phone turned off due to non-payment? Please describe.

Interviewer Comments on Financial Assessment:

43. What do you think the system capabilities are now and what are the issues you feel your system will be facing in the future? In addition, are there any specific needs, such as types of training that you would like to see addressed by NMED or its contractors?

APPENDIX B
COST BASIS

This section presents the basis for unit costs used to develop the conceptual cost estimates for the compliance alternatives. Cost estimates are conceptual in nature (+50%/-30%), and are intended to make comparisons between compliance options and to provide a preliminary indication of possible rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. Capital cost includes an allowance for engineering and construction management. It is assumed that adequate electrical power is available near the site. The cost estimates specifically do not include costs for the following:

- Obtaining land or easements.
- Surveying.
- Mobilization/demobilization for construction.
- Insurance and bonds

In general, unit costs are based on recent construction bids for similar work in the area; when possible, consultations with vendors or other suppliers; published construction and O&M cost data; and USEPA cost guidance. Unit costs used for the cost estimates are summarized in Table B.1.

Unit costs for pipeline components are based on 2006 RS Means Building Construction Cost Data. The number of borings and encasements and open cuts and encasements is estimated by counting the road, highway, railroad, stream, and river crossings for a conceptual routing of the pipeline. The number of air release valves is estimated by examining the land surface profile along the conceptual pipeline route. It is assumed gate valves and flush valves would be installed, on average, every 5,000 feet along the pipeline. Pipeline cost estimates are based on use of C-900 PVC pipe. Other pipe materials could be considered for more detailed development of attractive alternatives.

Pump station unit costs are based on experience with similar installations. The cost estimate for the pump stations include two pumps, station piping and valves, station electrical and instrumentation, minor site improvement, installation of a concrete pad, fence and building, and tools. Construction cost of a storage tank is based on 2006 RS Means Building Construction Cost Data.

Labor costs are estimated based on RS Means Building Construction Data specific to each region.

Electrical power cost is estimated to be $0.136 per kWH. The annual cost for power to a pump station is calculated based on the pumping head and volume, and includes 11,800 kWH for pump building heating, cooling, and lighting, as recommended in USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992).

In addition to the cost of electricity, pump stations have other maintenance costs. These costs cover: materials for minor repairs to keep the pumps operating; purchase of a maintenance vehicle, fuel costs, and vehicle maintenance costs; utilities; office supplies, small tools and equipment; and miscellaneous materials such as safety, clothing, chemicals, and paint. The non-power O&M costs are estimated based on the USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992), which provides cost curves for O&M components. Costs from the 1992 report are adjusted to 2006 dollars based on the ENR construction cost index.

Pipeline maintenance costs include routine cleaning and flushing, as well as minor repairs to lines. The unit rate for pipeline maintenance is calculated based on the USEPA technical report, *Innovative and Alternate Technology Assessment Manual MCD 53* (1978). Costs from the 1978 report are adjusted to 2006 dollars based on the ENR construction cost index.

Storage tank maintenance costs include cleaning and renewal of interior lining and exterior coating. Unit costs for storage tank O&M are based on USEPA publication *Standardized Costs for Water Supply Distribution Systems* (1992). Costs from the 1992 report are adjusted to 2006 dollars based on the ENR construction cost index.

The purchase price for POU water treatment units is based on vendor price lists for treatment units, plus installation. O&M costs for POU treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

The purchase price for POE water treatment units is based on vendor price lists for treatment units, plus an allowance for installation, including a concrete pad and shed, piping modifications, and electrical connection. O&M costs for POE treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

Central treatment plant costs, for both adsorption and coagulation/filtration, include pricing for buildings, utilities, and site work. Costs are based on pricing given in the various R.S. Means Construction Cost Data References, as well as prices obtained from similar work on other projects. Pricing for treatment equipment was obtained from vendors.

Well installation costs are based on quotations from drillers for installation of similar depth wells in the area. Well installation costs include drilling, a well pump, electrical and instrumentation installation, well finishing, piping, and water quality testing. O&M costs for water wells include power, materials, and labor. It is assumed that new wells located more than 1 mile from the intake point of an existing system would require at least one storage tank and pump station.

Purchase price for the treatment unit dispenser is based on vendor price lists, plus an allowance for installation at a centralized public location. The O&M costs are also based on vendor price lists. It is assumed that weekly water samples would be analyzed for the contaminant of concern.

Costs for bottled water delivery alternatives are based on consultation with vendors that deliver residential bottled water. The cost estimate includes an initial allowance for set-up of the program, and a yearly allowance for program administration.

The cost estimate for a public dispenser for trucked water includes the purchase price for a water truck and construction of a storage tank. Annual costs include labor for purchasing the water, picking up and delivering the water, truck maintenance, and water sampling and testing. It is assumed the water truck would be required to make one trip per dispenser each week, and that chlorine residual would be determined for each truck load.

Table B.1
Summary of General Data
Indian Springs Lake Estate LL
PWS #1870040
General PWS Information

Service Population	1,080	**Number of Connections**	360
Total PWS Daily Water Usage	0.127 (mgd)	**Source**	TCEQ website

Unit Cost Data
East Texas

General Items	**Unit**	**Unit Cost**
Treated water purchase cost	*See alternative*	
Water purchase cost (trucked)	$/1,000 gals	$ 2.50
Contingency	20%	n/a
Engineering & Constr. Management	25%	n/a
Procurement/admin (POU/POE)	20%	n/a
Pipeline Unit Costs	**Unit**	**Unit Cost**
PVC water line, Class 200, 06"	LF	$ 32
Bore and encasement, 10"	LF	$ 60
Open cut and encasement, 10"	LF	$ 35
Gate valve and box, 06"	EA	$ 465
Air valve	EA	$ 1,000
Flush valve	EA	$ 750
Metal detectable tape	LF	$ 0.15
Bore and encasement, length	Feet	200
Open cut and encasement, length	Feet	50
Pump Station Unit Costs	**Unit**	**Unit Cost**
Pump	EA	$ 7,500
Pump Station Piping, 06"	EA	$ 4,000
Gate valve, 06"	EA	$ 590
Check valve, 06"	EA	$ 890
Electrical/Instrumentation	EA	$ 10,000
Site work	EA	$ 2,000
Building pad	EA	$ 4,000
Pump Building	EA	$ 10,000
Fence	EA	$ 5,870
Tools	EA	$ 1,000
Well Installation Unit Costs	**Unit**	**Unit Cost**
Well installation	*See alternative*	
Water quality testing	EA	$ 1,500
Well pump	EA	$ 7,500
Well electrical/instrumentation	EA	$ 5,000
Well cover and base	EA	$ 3,000
Piping	EA	$ 2,500
2 Storage Tanks - 60,000 gals	EA	$ 74,200
Electrical Power	$/kWH	$ 0.136
Building Power	kWH	11,800
Labor	$/hr	$ 26
Materials	EA	$ 1,200
Transmission main O&M	$/mile	$ 200
Tank O&M	EA	$ 1,000
POU/POE Unit Costs		
POU treatment unit purchase	EA	$ 250
POU treatment unit installation	EA	$ 150
POE treatment unit purchase	EA	$ 3,000
POE - pad and shed, per unit	EA	$ 2,000
POE - piping connection, per unit	EA	$ 1,000
POE - electrical hook-up, per unit	EA	$ 1,000
POU treatment O&M, per unit	$/year	$ 225
POE treatment O&M, per unit	$/year	$ 1,000
Contaminant analysis	$/year	$ 100
POU/POE labor support	$/hr	$ 26
Dispenser/Bottled Water Unit Costs		
Treatment unit purchase	EA	$ 3,000
Treatment unit installation	EA	$ 5,000
Treatment unit O&M	EA	$ 500
Administrative labor	hr	$ 35
Bottled water cost (inc. delivery)	gallon	$ 1.60
Water use, per capita per day	gpcd	1.0
Bottled water program materials	EA	$ 5,000
Storage Tank - 5,000 gals	EA	$ 7,025
Site improvements	EA	$ 4,000
Potable water truck	EA	$ 60,000
Water analysis, per sample	EA	$ 100
Potable water truck O&M costs	$/mile	$ 1.00

Central Treatment Unit Costs	**Unit**	**Unit Cost**
General		
Site preparation	acre	$ 4,000
Slab	CY	$ 1,000
Building	SF	$ 60
Building electrical	SF	$ 8.00
Building plumbing	SF	$ 8.00
Heating and ventilation	SF	$ 7.00
Fence	LF	$ 15
Paving	SF	$ 2.00
Chlorination point	EA	$ 2,000
Building power	kwh/yr	$ 0.136
Equipment power	kwh/yr	$ 0.136
Labor, O&M	hr	$ 26
Analyses	test	$ 200
Ion exchange		
Electrical	JOB	$ 50,000
Piping	JOB	$ 20,000
Ion exchange package plant (Well 1)	UNIT	$ 30,000
Transfer pumps (10 hp)	EA	$ 5,000
Clean water tank	gal	$ 1.00
Regenerant tank	gal	$ 1.50
Backwash tank	gal	$ 2.00
Sewer connection fee	EA	$ 15,000
Ion exchange materials (Well 1 Only)	year	$ 1,000
Ion exchange chemicals (Well 1 only)	year	$ 1,000
Backwash discharge to sewer	kgal/year	$ 5.00
Waste haulage truck rental	days	$ 700
Mileage charge	mile	$ 1.00
Waste disposal fee	kgal/yr	$ 200
WRT Z-88 package		
Electrical	JOB	$ 50,000
Piping	JOB	$ 20,000
WRT Z-88 package plant	UNIT	$ 72,500
(Initial setup *cost for WRT Z-88 package*)		
WRT treated water charge No.1	1,000 gal/yr	$ 1.95
WRT treated water charge No.2&3	1000 gal/yr	$0.95
KMnO4-greensand package		
Electrical	JOB	$ 50,000
Piping	JOB	$ 20,000
KMnO4-greensand package plant (1)	UNIT	$ 60,000
Backwash tank	gal	$ 2.00
Sewer connection fee	EA	$ 15,000
KMnO4-greensand materials (1)	year	$ 2,000
KMnO4-greensand chemicals (1)	year	$ 2,000
Backwash discharge to sewer	1,000 gal/yr	$ 5.00
Sludge truck rental	days	$ 700
Sludge truck mileage fee	miles	$ 1.00
Sludge disposal fee	1,000 gal/yr	$ 2,200.00

APPENDIX C
COMPLIANCE ALTERNATIVE CONCEPTUAL COST ESTIMATES

This appendix presents the conceptual cost estimates developed for the compliance alternatives. The conceptual cost estimates are given in Tables C.1 through C.16. The cost estimates are conceptual in nature (+50%/-30%), and are intended for making comparisons between compliance options and to provide a preliminary indication of possible water rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation.

Table C.1

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *New Well at Indian Springs*
Alternative Number *IS-1*

Distance from PWS to new well location	0.06	miles
Estimated well depth	378	feet
Number of wells required	2	
Well installation cost (location specific)	$25	per foot
Number of pump stations needed	0	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	-	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	300	LF	$ 32	$ 9,600
Bore and encasement, 10"	-	LF	$ 60	$ -
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	0	EA	$ 465	$ 28
Air valve	-	EA	$ 1,000	$ -
Flush valve	0	EA	$ 750	$ 45
Metal detectable tape	300	LF	$ 0.15	$ 45
Subtotal				**$ 9,718**
Pump Station(s) Installation				
Pump	-	EA	$ 7,500	$ -
Pump Station Piping, 06"	-	EA	$ 4,000	$ -
Gate valve, 06"	-	EA	$ 590	$ -
Check valve, 06"	-	EA	$ 890	$ -
Electrical/Instrumentation	-	EA	$ 10,000	$ -
Site work	-	EA	$ 2,000	$ -
Building pad	-	EA	$ 4,000	$ -
Pump Building	-	EA	$ 10,000	$ -
Fence	-	EA	$ 5,870	$ -
Tools	-	EA	$ 1,000	$ -
2 Storage Tanks - 60,000 gals	-	EA	$ 74,200	$ -
Subtotal				**$ -**
Well Installation				
Well installation	756	LF	$ 25	$ 18,900
Water quality testing	4	EA	$ 1,500	$ 6,000
Well pump	2	EA	$ 7,500	$ 15,000
Well electrical/instrumentation	2	EA	$ 5,000	$ 10,000
Well cover and base	2	EA	$ 3,000	$ 6,000
Piping	2	EA	$ 2,500	$ 5,000
Subtotal				**$ 60,900**
Subtotal of Component Costs				**$ 70,618**
Contingency	20%			$ 14,124
Design & Constr Management	25%			$ 17,654
TOTAL CAPITAL COSTS				**$ 102,396**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	0.1	mile	$ 200	$ 11
Subtotal				**$ 11**
Pump Station(s) O&M				
Building Power	-	kWH	$ 0.136	$ -
Pump Power	-	kWH	$ 0.136	$ -
Materials	-	EA	$ 1,200	$ -
Labor	-	Hrs	$ 26	$ -
Tank O&M	-	EA	$ 1,000	$ -
Subtotal				**$ -**
Well O&M				
Pump power	6,212	kWH	$ 0.136	$ 845
Well O&M matl	2	EA	$ 1,200	$ 2,400
Well O&M labor	360	Hrs	$ 26	$ 9,400
Subtotal				**$ 12,644**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**
TOTAL ANNUAL O&M COSTS				**$ (5,942)**

Table C.2

PWS Name	***Indian Springs Lake Estate LL***
Alternative Name	***Purchase Water from Soda WS***
Alternative Number	***IS-2***

Distance from Alternative to PWS (along pipe)	5.0	miles
Total PWS annual water usage	46.355	MG
Treated water purchase cost	$ 1.65	per 1,000 gals
Number of Pump Stations Needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	13	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	26,241	LF	$ 32	$ 839,712
Bore and encasement, 10"	2,600	LF	$ 60	$ 156,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	5	EA	$ 465	$ 2,440
Air valve	5	EA	$ 1,000	$ 5,000
Flush valve	5	EA	$ 750	$ 3,936
Metal detectable tape	26,241	LF	$ 0.15	$ 3,936
Subtotal				**$ 1,011,025**
Pump Station(s) Installation				
Pump	2	EA	$ 7,500	$ 15,000
Pump Station Piping, 06"	1	EA	$ 4,000	$ 4,000
Gate valve, 06"	4	EA	$ 590	$ 2,360
Check valve, 06"	2	EA	$ 890	$ 1,780
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,000	$ 2,000
Building pad	1	EA	$ 4,000	$ 4,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 5,870	$ 5,870
Tools	1	EA	$ 1,000	$ 1,000
2 Storage Tanks - 60,000 gals	1	EA	$ 74,200	$ 74,200
Subtotal				**$ 130,210**

Subtotal of Component Costs		**$ 1,141,235**
Contingency	20%	$ 228,247
Design & Constr Management	25%	$ 285,309
TOTAL CAPITAL COSTS		**$ 1,654,790**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	5.0	mile	$ 200	$ 994
Subtotal				**$ 994**
Water Purchase Cost				
From BWA	46,355	1,000 gal	$ 1.65	$ 76,486
Subtotal				**$ 76,486**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.136	$ 1,605
Pump Power	23,591	kWH	$ 0.136	$ 3,208
Materials	1	EA	$ 1,200	$ 1,200
Labor	365	Hrs	$ 26	$ 9,530
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 16,543**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**

TOTAL ANNUAL O&M COSTS	**$ 75,425**

Table C.3

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *Purchase Water from Wilson Lake Estates*
Alternative Number *IS-3*

Distance from Alternative to PWS (along pipe)	4.3	miles
Total PWS annual water usage	46.355	MG
Treated water purchase cost	$ 1.65	per 1,000 gals
Number of Pump Stations Needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	12	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	22,571	LF	$ 32	$ 722,272
Bore and encasement, 10"	2,400	LF	$ 60	$ 144,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	5	EA	$ 465	$ 2,099
Air valve	4	EA	$ 1,000	$ 4,000
Flush valve	5	EA	$ 750	$ 3,386
Metal detectable tape	22,571	LF	$ 0.15	$ 3,386
Subtotal				**$ 879,142**
Pump Station(s) Installation				
Pump	2	EA	$ 7,500	$ 15,000
Pump Station Piping, 06"	1	EA	$ 4,000	$ 4,000
Gate valve, 06"	4	EA	$ 590	$ 2,360
Check valve, 06"	2	EA	$ 890	$ 1,780
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,000	$ 2,000
Building pad	1	EA	$ 4,000	$ 4,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 5,870	$ 5,870
Tools	1	EA	$ 1,000	$ 1,000
2 Storage Tanks - 60,000 gals	1	EA	$ 74,200	$ 74,200
Subtotal				**$ 130,210**
Subtotal of Component Costs				**$ 1,009,352**
Contingency	20%			$ 201,870
Design & Constr Management	25%			$ 252,338
TOTAL CAPITAL COSTS				**$ 1,463,561**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	4.3	mile	$ 200	$ 855
Subtotal				**$ 855**
Water Purchase Cost				
From BWA	46,355	1,000 gal	$ 1.65	$ 76,486
Subtotal				**$ 76,486**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.136	$ 1,605
Pump Power	21,505	kWH	$ 0.136	$ 2,925
Materials	1	EA	$ 1,200	$ 1,200
Labor	365	Hrs	$ 26	$ 9,530
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 16,260**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**
TOTAL ANNUAL O&M COSTS				**$ 75,003**

Table C.4

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *New Well at Wilson Lake Estates*
Alternative Number *IS-4*

Distance from PWS to new well location	4.27	miles
Estimated well depth	378	feet
Number of wells required	2	
Well installation cost (location specific)	$25	per foot
Number of pump stations needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	12	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	22,571	LF	$ 32	$ 722,272
Bore and encasement, 10"	2,400	LF	$ 60	$ 144,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	5	EA	$ 465	$ 2,099
Air valve	4	EA	$ 1,000	$ 4,000
Flush valve	5	EA	$ 750	$ 3,386
Metal detectable tape	22,571	LF	$ 0.15	$ 3,386
Subtotal				**$ 879,142**
Pump Station(s) Installation				
Pump	2	EA	$ 7,500	$ 15,000
Pump Station Piping, 06"	1	EA	$ 4,000	$ 4,000
Gate valve, 06"	4	EA	$ 590	$ 2,360
Check valve, 06"	2	EA	$ 890	$ 1,780
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,000	$ 2,000
Building pad	1	EA	$ 4,000	$ 4,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 5,870	$ 5,870
Tools	1	EA	$ 1,000	$ 1,000
2 Storage Tanks - 60,000 gals	1	EA	$ 74,200	$ 74,200
Subtotal				**$ 130,210**
Well Installation				
Well installation	756	LF	$ 25	$ 18,900
Water quality testing	4	EA	$ 1,500	$ 6,000
Well pump	2	EA	$ 7,500	$ 15,000
Well electrical/instrumentation	2	EA	$ 5,000	$ 10,000
Well cover and base	2	EA	$ 3,000	$ 6,000
Piping	2	EA	$ 2,500	$ 5,000
Subtotal				**$ 60,900**

Subtotal of Component Costs		**$ 1,070,252**
Contingency	20%	$ 214,050
Design & Constr Management	25%	$ 267,563
TOTAL CAPITAL COSTS		**$ 1,551,866**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	4.3	mile	$ 200	$ 855
Subtotal				**$ 855**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.136	$ 1,605
Pump Power	21,505	kWH	$ 0.136	$ 2,925
Materials	1	EA	$ 1,200	$ 1,200
Labor	365	Hrs	$ 26	$ 9,530
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 16,260**
Well O&M				
Pump power	6,212	kWH	$ 0.136	$ 845
Well O&M matl	2	EA	$ 1,200	$ 2,400
Well O&M labor	360	Hrs	$ 26	$ 9,400
Subtotal				**$ 12,644**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**

TOTAL ANNUAL O&M COSTS	**$ 11,161**

Table C.5

PWS Name ***Indian Springs Lake Estate LL***
Alternative Name ***Purchase Water from Woods Creek WS***
Alternative Number ***IS-5***

Distance from Alternative to PWS (along pipe)	8.0	miles
Total PWS annual water usage	46.355	MG
Treated water purchase cost	$ 1.65	per 1,000 gals
Number of Pump Stations Needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	19	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	42,375	LF	$ 32	$ 1,356,000
Bore and encasement, 10"	3,800	LF	$ 60	$ 228,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	8	EA	$ 465	$ 3,941
Air valve	8	EA	$ 1,000	$ 8,000
Flush valve	8	EA	$ 750	$ 6,356
Metal detectable tape	42,375	LF	$ 0.15	$ 6,356
Subtotal				**$ 1,608,653**
Pump Station(s) Installation				
Pump	2	EA	$ 7,500	$ 15,000
Pump Station Piping, 06"	1	EA	$ 4,000	$ 4,000
Gate valve, 06"	4	EA	$ 590	$ 2,360
Check valve, 06"	2	EA	$ 890	$ 1,780
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,000	$ 2,000
Building pad	1	EA	$ 4,000	$ 4,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 5,870	$ 5,870
Tools	1	EA	$ 1,000	$ 1,000
2 Storage Tanks - 60,000 gals	1	EA	$ 74,200	$ 74,200
Subtotal				**$ 130,210**
Subtotal of Component Costs				**$ 1,738,863**
Contingency	20%			$ 347,773
Design & Constr Management	25%			$ 434,716
TOTAL CAPITAL COSTS				**$ 2,521,352**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	8.0	mile	$ 200	$ 1,605
Subtotal				**$ 1,605**
Water Purchase Cost				
From BWA	46,355	1,000 gal	$ 1.65	$ 76,486
Subtotal				**$ 76,486**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.136	$ 1,605
Pump Power	33,091	kWH	$ 0.136	$ 4,500
Materials	1	EA	$ 1,200	$ 1,200
Labor	365	Hrs	$ 26	$ 9,530
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 17,835**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**
TOTAL ANNUAL O&M COSTS				**$ 77,328**

Table C.6

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *New Well at 10 Miles*
Alternative Number *IS-6*

Distance from PWS to new well location	10.0	miles
Estimated well depth	378	feet
Number of wells required	2	
Well installation cost (location specific)	$25	per foot
Number of pump stations needed	1	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	26	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	52,800	LF	$ 32	$ 1,689,600
Bore and encasement, 10"	5,200	LF	$ 60	$ 312,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	11	EA	$ 465	$ 4,910
Air valve	10	EA	$ 1,000	$ 10,000
Flush valve	11	EA	$ 750	$ 7,920
Metal detectable tape	52,800	LF	$ 0.15	$ 7,920
Subtotal				**$ 2,032,350**
Pump Station(s) Installation				
Pump	2	EA	$ 7,500	$ 15,000
Pump Station Piping, 06"	1	EA	$ 4,000	$ 4,000
Gate valve, 06"	4	EA	$ 590	$ 2,360
Check valve, 06"	2	EA	$ 890	$ 1,780
Electrical/Instrumentation	1	EA	$ 10,000	$ 10,000
Site work	1	EA	$ 2,000	$ 2,000
Building pad	1	EA	$ 4,000	$ 4,000
Pump Building	1	EA	$ 10,000	$ 10,000
Fence	1	EA	$ 5,870	$ 5,870
Tools	1	EA	$ 1,000	$ 1,000
2 Storage Tanks - 60,000 gals	1	EA	$ 7,025	$ 7,025
Subtotal				**$ 63,035**
Well Installation				
Well installation	756	LF	$ 25	$ 18,900
Water quality testing	4	EA	$ 1,500	$ 6,000
Well pump	2	EA	$ 7,500	$ 15,000
Well electrical/instrumentation	2	EA	$ 5,000	$ 10,000
Well cover and base	2	EA	$ 3,000	$ 6,000
Piping	2	EA	$ 2,500	$ 5,000
Subtotal				**$ 60,900**
Subtotal of Component Costs				**$ 2,156,285**
Contingency	20%			$ 431,257
Design & Constr Management	25%			$ 539,071
TOTAL CAPITAL COSTS				**$ 3,126,614**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	10.0	mile	$ 200	$ 2,000
Subtotal				**$ 2,000**
Pump Station(s) O&M				
Building Power	11,800	kWH	$ 0.136	$ 1,605
Pump Power	46,271	kWH	$ 0.136	$ 6,293
Materials	1	EA	$ 1,200	$ 1,200
Labor	365	Hrs	$ 26	$ 9,530
Tank O&M	1	EA	$ 1,000	$ 1,000
Subtotal				**$ 19,628**
Well O&M				
Pump power	6,212	kWH	$ 0.136	$ 845
Well O&M matl	2	EA	$ 1,200	$ 2,400
Well O&M labor	360	Hrs	$ 26	$ 9,400
Subtotal				**$ 12,644**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**
TOTAL ANNUAL O&M COSTS				**$ 15,674**

Table C.7

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *New Well at 5 Miles*
Alternative Number *IS-7*

Distance from PWS to new well location	5.0	miles
Estimated well depth	378	feet
Number of wells required	2	
Well installation cost (location specific)	$25	per foot
Number of pump stations needed	0	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	13	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	26,400	LF	$ 32	$ 844,800
Bore and encasement, 10"	1,800	LF	$ 60	$ 108,000
Open cut and encasement, 10"	100	LF	$ 35	$ 3,500
Gate valve and box, 06"	5	EA	$ 465	$ 2,455
Air valve	5	EA	$ 1,000	$ 5,000
Flush valve	5	EA	$ 750	$ 3,960
Metal detectable tape	26,400	LF	$ 0.15	$ 3,960
Subtotal				**$ 971,675**
Pump Station(s) Installation				
Pump	-	EA	$ 7,500	$ -
Pump Station Piping, 06"	-	EA	$ 4,000	$ -
Gate valve, 06"	-	EA	$ 590	$ -
Check valve, 06"	-	EA	$ 890	$ -
Electrical/Instrumentation	-	EA	$ 10,000	$ -
Site work	-	EA	$ 2,000	$ -
Building pad	-	EA	$ 4,000	$ -
Pump Building	-	EA	$ 10,000	$ -
Fence	-	EA	$ 5,870	$ -
Tools	-	EA	$ 1,000	$ -
2 Storage Tanks - 60,000 gals	-	EA	$ 7,025	$ -
Subtotal				**$ -**
Well Installation				
Well installation	756	LF	$ 25	$ 18,900
Water quality testing	4	EA	$ 1,500	$ 6,000
Well pump	2	EA	$ 7,500	$ 15,000
Well electrical/instrumentation	2	EA	$ 5,000	$ 10,000
Well cover and base	2	EA	$ 3,000	$ 6,000
Piping	2	EA	$ 2,500	$ 5,000
Subtotal				**$ 60,900**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	5.0	mile	$ 200	$ 1,000
Subtotal				**$ 1,000**
Pump Station(s) O&M				
Building Power	-	kWH	$ 0.136	$ -
Pump Power	23,136	kWH	$ 0.136	$ 3,146
Materials	-	EA	$ 1,200	$ -
Labor	-	Hrs	$ 26	$ -
Tank O&M	-	EA	$ 1,000	$ -
Subtotal				**$ 3,146**
Well O&M				
Pump power	6,212	kWH	$ 0.136	$ 845
Well O&M matl	2	EA	$ 1,200	$ 2,400
Well O&M labor	360	Hrs	$ 26	$ 9,400
Subtotal				**$ 12,644**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**

Subtotal of Component Costs		**$ 1,032,575**
Contingency	20%	$ 206,515
Design & Constr Management	25%	$ 258,144
TOTAL CAPITAL COSTS		**$ 1,497,234**

TOTAL ANNUAL O&M COSTS **$ (1,807)**

Table C.8

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *New Well at 1 Mile*
Alternative Number *IS-8*

Distance from PWS to new well location	1.0	miles
Estimated well depth	378	feet
Number of wells required	2	
Well installation cost (location specific)	$25	per foot
Number of pump stations needed	0	

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline Construction				
Number of Crossings, bore	3	n/a	n/a	n/a
Number of Crossings, open cut	-	n/a	n/a	n/a
PVC water line, Class 200, 06"	5,280	LF	$ 32	$ 168,960
Bore and encasement, 10"	600	LF	$ 60	$ 36,000
Open cut and encasement, 10"	-	LF	$ 35	$ -
Gate valve and box, 06"	1	EA	$ 465	$ 491
Air valve	1.00	EA	$ 1,000	$ 1,000
Flush valve	1	EA	$ 750	$ 792
Metal detectable tape	5,280	LF	$ 0.15	$ 792
Subtotal				**$ 208,035**
Pump Station(s) Installation				
Pump	-	EA	$ 7,500	$ -
Pump Station Piping, 06"	-	EA	$ 4,000	$ -
Gate valve, 06"	-	EA	$ 590	$ -
Check valve, 06"	-	EA	$ 890	$ -
Electrical/Instrumentation	-	EA	$ 10,000	$ -
Site work	-	EA	$ 2,000	$ -
Building pad	-	EA	$ 4,000	$ -
Pump Building	-	EA	$ 10,000	$ -
Fence	-	EA	$ 5,870	$ -
Tools	-	EA	$ 1,000	$ -
2 Storage Tanks - 60,000 gals	-	EA	$ 7,025	$ -
Subtotal				**$ -**
Well Installation				
Well installation	756	LF	$ 25	$ 18,900
Water quality testing	4	EA	$ 1,500	$ 6,000
Well pump	2	EA	$ 7,500	$ 15,000
Well electrical/instrumentation	2	EA	$ 5,000	$ 10,000
Well cover and base	2	EA	$ 3,000	$ 6,000
Piping	2	EA	$ 2,500	$ 5,000
Subtotal				**$ 60,900**
Subtotal of Component Costs				**$ 268,935**
Contingency	20%			$ 53,787
Design & Constr Management	25%			$ 67,234
TOTAL CAPITAL COSTS				**$ 389,956**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Pipeline O&M				
Pipeline O&M	1.0	mile	$ 200	$ 200
Subtotal				**$ 200**
Pump Station(s) O&M				
Building Power	-	kWH	$ 0.136	$ -
Pump Power	-	kWH	$ 0.136	$ -
Materials	-	EA	$ 1,200	$ -
Labor	-	Hrs	$ 26	$ -
Tank O&M	-	EA	$ 1,000	$ -
Subtotal				**$ -**
Well O&M				
Pump power	6,212	kWH	$ 0.136	$ 845
Well O&M matl	2	EA	$ 1,200	$ 2,400
Well O&M labor	360	Hrs	$ 26	$ 9,400
Subtotal				**$ 12,644**
O&M Credit for Existing Well Closure				
Pump power	6,606	kWH	$ 0.136	$ (898)
Well O&M matl	3	EA	$ 1,200	$ (3,600)
Well O&M labor	540	Hrs	$ 26	$ (14,099)
Subtotal				**$ (18,598)**
TOTAL ANNUAL O&M COSTS				**$ (5,753)**

Table C.9

PWS Name — ***Indian Springs Lake Estate LL***
Alternative Name — ***Central Treatment - IX***
Alternative Number — ***IS-9***

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Ion Exchange Unit Purchase/Installation				
Site preparation	2	acre	$ 4,000	$ 6,000
Slab	60	CY	$ 1,000	$ 60,000
Building	800	SF	$ 60	$ 48,000
Building electrical	800	SF	$ 8	$ 6,400
Building plumbing	800	SF	$ 8	$ 6,400
Heating and ventilation	800	SF	$ 7	$ 5,600
Fence	0	LF	$ 15	$ -
Paving	6,400	SF	$ 2	$ 12,800
Electrical	2	JOB	$ 50,000	$ 100,000
Piping	2	JOB	$ 20,000	$ 40,000
Ion exchange package including:				
Regeneration system				
Brine tank				
IX resins & FRP vessels	1	UNIT	$ 30,000	$ 30,000
	1	UNIT	$ 100,000	$ 100,000
Transfer pumps (10 hp)	4	EA	$ 5,000	$ 20,000
Clean water tank	15,000	gal	$ 1.00	$ 15,000
Regenerant tank	7,000	gal	$ 1.50	$ 10,500
Backwash Tank	36,000	gal	$ 2.00	$ 72,000
Sewer Connection Fee	0	EA	$ 15,000	$ -
Subtotal of Component Costs				**$ 532,700**
Contingency	20%			$ 106,540
Design & Constr Management	25%			$ 133,175
TOTAL CAPITAL COSTS				**$ 772,415**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Ion Exchange Unit O&M				
Building Power	24,000	kwh/yr	$ 0.136	$ 3,264
Equipment power	20,000	kwh/yr	$ 0.136	$ 2,720
Labor	800	hrs/yr	$ 40	$ 32,000
Materials	2	year	$ 2,000	$ 4,000
Chemicals	2	year	$ 2,000	$ 4,000
Analyses	48	test	$ 200	$ 9,600
Backwash disposal	10	kgal/yr	$ 200.00	$ 2,000
Subtotal				**$ 57,584**
Haul Regenerant Waste and Brine				
Waste haulage truck rental	20	days	$ 700	$ 14,000
Mileage charge	2,000	miles	$ 1.00	$ 2,000
Waste disposal	62	kgal/yr	$ 200.00	$ 12,400
Subtotal				**$ 28,400**
TOTAL ANNUAL O&M COSTS				**$ 85,984**

Table C.10

PWS Name ***Indian Springs Lake Estate LL***
Alternative Name ***Central Treatment - WRT Z-88***
Alternative Number ***IS-10***

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Coagulation/Filtration Unit Purchase/Installation				
Site preparation	2	acre	$ 4,000	$ 6,000
Slab	60	CY	$ 1,000	$ 60,000
Building	800	SF	$ 60	$ 48,000
Building electrical	800	SF	$ 8	$ 6,400
Building plumbing	800	SF	$ 8	$ 6,400
Heating and ventilation	800	SF	$ 7	$ 5,600
Fence	0	LF	$ 15	$ -
Paving	3,200	SF	$ 2	$ 6,400
Electrical	2	JOB	$ 50,000	$ 100,000
Piping	2	JOB	$ 20,000	$ 40,000
WRT Z-88 package including:				
Z-88 vessels				
Adsorption media	2	UNIT	$ 72,500	$ 145,000
(Initial Setup Cost for WRT Z-88 package plant)				
Subtotal of Component Costs				**$ 423,800**
Contingency	20%			$ 84,760
Design & Constr Management	25%			$ 105,950
TOTAL CAPITAL COSTS				**$ 614,510**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Coagulation/Filtration Unit O&M				
Building Power	12,000	kwh/yr	$ 0.136	$ 1,632
Equipment power	10,000	kwh/yr	$ 0.136	$ 1,360
Labor	800	hrs/yr	$ 40	$ 32,000
Analyses	48	test	$ 200	$ 9,600
WRT treated water charge	46,355	kgal/yr		$ 52,797
Subtotal				**$ 97,389**
TOTAL ANNUAL O&M COSTS				**$ 97,389**

Table C.11

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *Central Treatment - KMnO4*
Alternative Number *IS-11*

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Coagulation/Filtration Unit Purchase/Installation				
Site preparation	1	acre	$ 4,000	$ 4,000
Slab	60	CY	$ 1,000	$ 60,000
Building	900	SF	$ 60	$ 54,000
Building electrical	900	SF	$ 8	$ 7,200
Building plumbing	900	SF	$ 8	$ 7,200
Heating and ventilation	900	SF	$ 7	$ 6,300
Fence	-	LF	$ 15	$ -
Paving	3,200	SF	$ 2	$ 6,400
Electrical	2	JOB	$ 50,000	$ 100,000
Piping	2	JOB	$ 20,000	$ 40,000
KMnO4-Greensand package including:				
Greensand filters				
Solution tank	1	UNIT	$ 260,000	$ 260,000
Backwash tank	40,000	gal	$ 2.00	$ 80,000
Sewer connection fee	-	EA	$ 15,000	$ -
Subtotal of Component Costs				**$ 625,100**
Contingency	20%			$ 125,020
Design & Constr Management	25%			$ 156,275
TOTAL CAPITAL COSTS				**$ 906,395**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Coagulation/Filtration Unit O&M				
Building Power	12,000	kwh/yr	$ 0.136	$ 1,632
Equipment power	16,000	kwh/yr	$ 0.136	$ 2,176
Labor	1,000	hrs/yr	$ 40	$ 40,000
Materials	1	year	$ 5,000	$ 5,000
Chemicals	1	year	$ 5,000	$ 5,000
Analyses	48	test	$ 200	$ 9,600
Backwash disposal	69	kgal/yr	$ 200.00	$ 13,800
Subtotal				**$ 77,208**
Sludge Disposal				
Truck rental	28	days	$ 700	$ 19,600
Mileage	2,800	miles	$ 1.00	$ 2,800
Disposal fee	56	kgal/yr	$ 200.00	$ 11,200
Subtotal				**$ 33,600**
TOTAL ANNUAL O&M COSTS				**$ 110,808**

Table C.12

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *Point-of-Use Treatment*
Alternative Number *IS-12*

Number of Connections for POU Unit Installation 360

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
POU-Treatment - Purchase/Installation				
POU treatment unit purchase	360	EA	$ 250	$ 90,000
POU treatment unit installation	360	EA	$ 150	$ 54,000
Subtotal				**$ 144,000**
Subtotal of Component Costs				**$ 144,000**
Contingency	20%			$ 28,800
Design & Constr Management	25%			$ 36,000
Procurement & Administration	20%			$ 28,800
TOTAL CAPITAL COSTS				**$ 237,600**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
O&M				
POU materials, per unit	360	EA	$ 225	$ 81,000
Contaminant analysis, 1/yr per unit	360	EA	$ 100	$ 36,000
Program labor, 10 hrs/unit	3,600	hrs	$ 26	$ 93,996
Subtotal				**$ 210,996**
TOTAL ANNUAL O&M COSTS				**$ 210,996**

Table C.13

PWS Name ***Indian Springs Lake Estate LL***
Alternative Name ***Point-of-Entry Treatment***
Alternative Number ***IS-13***

Number of Connections for POE Unit Installation 360

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
POE-Treatment - Purchase/Installation				
POE treatment unit purchase	360	EA	$ 3,000	$ 1,080,000
Pad and shed, per unit	360	EA	$ 2,000	$ 720,000
Piping connection, per unit	360	EA	$ 1,000	$ 360,000
Electrical hook-up, per unit	360	EA	$ 1,000	$ 360,000
Subtotal				**$ 2,520,000**
Subtotal of Component Costs				**$ 2,520,000**
Contingency	20%			$ 504,000
Design & Constr Management	25%			$ 630,000
Procurement & Administration	20%			$ 504,000
TOTAL CAPITAL COSTS				**$ 4,158,000**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
O&M				
POE materials, per unit	360	EA	$ 1,000	$ 360,000
Contaminant analysis, 1/yr per unit	360	EA	$ 100	$ 36,000
Program labor, 10 hrs/unit	3,600	hrs	$ 26	$ 93,996
Subtotal				**$ 489,996**
TOTAL ANNUAL O&M COSTS				**$ 489,996**

Table C.14

PWS Name ***Indian Springs Lake Estate LL***
Alternative Name ***Public Dispenser for Treated Drinking Water***
Alternative Number ***IS-14***

Number of Treatment Units Recommended 3

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Public Dispenser Unit Installation				
POE-Treatment unit(s)	3	EA	$ 3,000	$ 9,000
Unit installation costs	3	EA	$ 5,000	$ 15,000
Subtotal				**$ 24,000**
Subtotal of Component Costs				**$ 24,000**
Contingency	20%			$ 4,800
Design & Constr Management	25%			$ 6,000
TOTAL CAPITAL COSTS				**34,800**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Program Operation				
Treatment unit O&M, 1 per unit	3	EA	$ 500	$ 1,500
Contaminant analysis, 1/wk per ui	156	EA	$ 100	$ 15,600
Sampling/reporting, 1 hr/day	1,095	HRS	$ 26	$ 28,590
Subtotal				**$ 45,690**
TOTAL ANNUAL O&M COSTS				**$ 45,690**

Table C.15

PWS Name ***Indian Springs Lake Estate LL***
Alternative Name ***Supply Bottled Water to Population***
Alternative Number ***IS-15***

Service Population	1,080	
Percentage of population requiring supply	100%	
Water consumption per person	1.00	gpcd
Calculated annual potable water needs	394,200	gallons

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Program Implementation				
Initial program set-up	500	hours	$ 35	$ 17,363
Subtotal				**$ 17,363**
Subtotal of Component Costs				**$ 17,363**
Contingency	20%			$ 3,473
TOTAL CAPITAL COSTS				**$ 20,836**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Program Operation				
Water purchase costs	394,200	gals	$ 1.60	$ 630,720
Program admin, 9 hrs/wk	468	hours	$ 35	$ 16,252
Program materials	1	EA	$ 5,000	$ 5,000
Subtotal				**$ 651,972**
TOTAL ANNUAL O&M COSTS				**$ 651,972**

Table C.16

PWS Name *Indian Springs Lake Estate LL*
Alternative Name *Central Trucked Drinking Water*
Alternative Number *IS-16*

Service Population	1,080	
Percentage of population requiring supply	100%	
Water consumption per person	1.00	gpcd
Calculated annual potable water needs	394,200	gallons
Travel distance to compliant water source (roundtrip)	25	miles

Capital Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Storage Tank Installation				
Storage Tank - 5,000 gals	3	EA	$ 7,025	$ 21,075
Site improvements	3	EA	$ 4,000	$ 12,000
Potable water truck	1	EA	$ 60,000	$ 60,000
Subtotal				**$ 93,075**
Subtotal of Component Costs				**$ 93,075**
Contingency	20%			$ 18,615
Design & Constr Management	25%			$ 23,269
TOTAL CAPITAL COSTS				**$ 134,959**

Annual Operations and Maintenance Costs

Cost Item	Quantity	Unit	Unit Cost	Total Cost
Program Operation				
Water delivery labor, 4 hrs/wk	624	hrs	$ 26	$ 16,293
Truck operation, 1 round trip/wk	3,900	miles	$ 1.00	$ 3,900
Water purchase	394	1,000 gals	$ 2.50	$ 986
Water testing, 1 test/wk	156	EA	$ 100	$ 15,600
Sampling/reporting, 2 hrs/wk	312	hrs	$ 26	$ 8,146
Subtotal				**$ 44,924**
TOTAL ANNUAL O&M COSTS				**$ 44,924**

APPENDIX D
EXAMPLE FINANCIAL MODEL

Table D.1 Example Financial Model

Water System	Indian Springs
Funding Alternative	Bond
Alternative Description	Purchase Water from Soda WS

Sum of Amount		Year										
Group	Type	2004	2005	2006	2007	2008	2009	2010	2011	2012	2013	2014
Capital Expenditures	Capital Expenditures-Funded from Bonds	$ -	$ -	$ -	########	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Grants	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Revenue/Reserves	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from SRF Loans	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Capital Expenditures Sum		$ -	$ -	$ -	########	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service	Revenue Bonds				$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449
	State Revolving Funds				$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service Sum					$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449
Operating Expenditures	Other Operating Expenditures 1			$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396
	Professional and Directors Fees			$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088
	Repairs			$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554
	Salaries & Benefits			$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780
	Utilities			$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363
	O&M Associated with Alternative					$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425
	Accounting and Legal Fees			$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613
Operating Expenditures Sum				$ 374,794	$ 374,794	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219
Residential Operating Revenues	Residential Base Monthly Rate			$ 507,651	$ 507,651	$ 507,651	$ 507,651	$ 656,900	$ 826,157	$ 846,166	$ 846,166	$ 846,166
	Residential Tier 1 Monthly Rate			$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier2 Monthly Rate			$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier3 Monthly Rate			$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier4 Monthly Rate			$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Unmetered Monthly Rate			$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Residential Operating Revenues Sum				$ 507,651	$ 507,651	$ 507,651	$ 507,651	$ 656,900	$ 826,157	$ 846,166	$ 846,166	$ 846,166

Group	Type	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025
Capital Expenditures	Capital Expenditures-Funded from Bonds	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Grants	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from Revenue/Reserves	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Capital Expenditures-Funded from SRF Loans	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Capital Expenditures Sum		$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service	Revenue Bonds	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449
	State Revolving Funds	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Debt Service Sum		$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449	$ 129,449
Operating Expenditures	Other Operating Expenditures 1	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396	$ 200,396
	Professional and Directors Fees	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088	$ 4,088
	Repairs	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554	$ 18,554
	Salaries & Benefits	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780	$ 111,780
	Utilities	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363	$ 26,363
	O&M Associated with Alternative	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425	$ 75,425
	Accounting and Legal Fees	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613	$ 13,613
Operating Expenditures Sum		$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219	$ 450,219
Residential Operating Revenues	Residential Base Monthly Rate	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166
	Residential Tier 1 Monthly Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier2 Monthly Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier3 Monthly Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Tier4 Monthly Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Residential Unmetered Monthly Rate	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
Residential Operating Revenues Sum		$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166

Location_Name	Indian Springs
Alt_Desc	Purchase Water from Soda WS

		Current_Year										
Funding_Alt	Data	2006	2007	2008	2009	2010	2011	2012	2013	2014	2015	2016
Bond	Sum of Beginning_Cash_Bal	$ 222,340	$ 270,527	$ 189,266	$ 20,009	$ (149,249)	$ (169,257)	$ (20,009)	$ 149,249	$ 318,506	$ 487,763	$ 657,021
	Sum of Total_Expenditures	$ 374,794	$ 2,159,033	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668
	Sum of Total_Receipts	$ 507,651	$ 2,162,441	$ 507,651	$ 507,651	$ 656,900	$ 826,157	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166
	Sum of Net_Cash_Flow	$ 132,857	$ 3,408	$ (72,017)	$ (72,017)	$ 77,232	$ 246,489	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498
	Sum of Ending_Cash_Bal	$ 355,197	$ 273,936	$ 117,249	$ (52,008)	$ (72,017)	$ 77,232	$ 246,489	$ 415,747	$ 585,004	$ 754,261	$ 923,518
	Sum of Working_Cap	$ 62,466	$ 62,466	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037
	Sum of Repl_Resv	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204
	Sum of Total_Reqd_Resv	$ 84,670	$ 84,670	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241
	Sum of Net_Avail_Bal	$ 270,527	$ 189,266	$ 20,009	$ (149,249)	$ (169,257)	$ (20,009)	$ 149,249	$ 318,506	$ 487,763	$ 657,021	$ 826,278
	Sum of Add_Resv_Needed	$ -	$ -	$ -	$ (149,249)	$ (169,257)	$ (20,009)	$ -	$ -	$ -	$ -	$ -
	Sum of Rate_Inc_Needed	0%	0%	0%	29%	26%	2%	0%	0%	0%	0%	0%
	Sum of Percent_Rate_Increase	0%	0%	0%	0%	29%	63%	67%	67%	67%	67%	67%

Funding_Alt	Data	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027
Bond	Sum of Beginning_Cash_Bal	$ 826,278	$ 995,535	$ 1,164,793	$ 1,334,050	$ 1,503,307	$ 1,672,565	$ 1,841,822	$ 2,011,079	$ 2,180,336	$ 2,349,594	$ 2,518,851
	Sum of Total_Expenditures	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668	$ 579,668
	Sum of Total_Receipts	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166	$ 846,166
	Sum of Net_Cash_Flow	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498	$ 266,498
	Sum of Ending_Cash_Bal	$ 1,092,776	$ 1,262,033	$ 1,431,290	$ 1,600,548	$ 1,769,805	$ 1,939,062	$ 2,108,320	$ 2,277,577	$ 2,446,834	$ 2,616,092	$ 2,785,349
	Sum of Working_Cap	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037	$ 75,037
	Sum of Repl_Resv	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204	$ 22,204
	Sum of Total_Reqd_Resv	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241	$ 97,241
	Sum of Net_Avail_Bal	$ 995,535	$ 1,164,793	$ 1,334,050	$ 1,503,307	$ 1,672,565	$ 1,841,822	$ 2,011,079	$ 2,180,336	$ 2,349,594	$ 2,518,851	$ 2,688,108
	Sum of Add_Resv_Needed	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -	$ -
	Sum of Rate_Inc_Needed	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0
	Sum of Percent_Rate_Increase	67%	67%	67%	67%	67%	67%	67%	67%	67%	67%	67%

APPENDIX E
RADIONUCLIDE GEOCHEMISTRY

Radionuclide impact on water quality is measured according to two scales: intrinsic measurement of radioactivity and impact on human beings. Activity or number of disintegrations per unit time is typically measured in pico Curies (pCi), whereas impact on living organisms is measured in mrem. Radioactive decay can generate alpha or beta particles, as well as gamma rays. Two radioactive elements with the same activity may have vastly different impacts on life, depending on the energy released during decay. Each radionuclide has a conversion factor from pCi to mrem as a function of exposure pathway. Activity is related to contaminant concentration and half-life. A higher concentration and a shorter half-life lead to increased activity. Given the ratio of the half-life of each (Table E.1), it is apparent that radium is approximately 1 million times more radioactive than uranium. Concentrations of gross alpha and beta emitters take into account the whole decay series and not just uranium and radium, as well as other elements such as K 40.

Uranium and thorium (atomic number 92 and 90, respectively), both radium sources, are common trace elements and have a crustal abundance of 2.6 and 10 mg/kg, respectively. They are abundant in acidic rock. Intrusive rock such as granite will partly sequester uranium and thorium in erosion-resistant accessory minerals (*e.g.*, monazite, thorite) while uranium in volcanic rock is much more labile and can be leached by surface water and groundwater. Lattice substitution in minerals (*e.g.*, Ca^{+2} and U^{+4} have almost the same ionic radius) as well as micrograins of uranium and thorium minerals are other possibilities. In sedimentary rock, uranium and thorium aqueous concentrations are controlled mainly by the sorbing potential of the rock (metal oxide, clay, and organic matter).

The geochemistry of uranium is complicated but can be summarized by the following. Uranium (VI) in oxidizing conditions exists as the soluble positively charged uranyl UO_2^{+2}. Solubility is higher at acid pHs, decreases at neutral pHs, and increases at alkaline pHs. The uranyl ion can easily form aqueous complexes, including with hydroxyl, fluoride, carbonate, and phosphate ligands. Hence, in the presence of carbonates, uranium solubility is considerably enhanced in the form of uranyl-carbonate (UO_2CO_3) and other higher order carbonate complexes: uranyl-di-carbonate ($UO_2(CO_3)_2^{-2}$ and uranyl-tri-carbonates $UO_2(CO_3)_3^{-4}$). Adsorption of uranium is inversely related to its solubility and is highest at neutral pH's (De Soto 1978). Uranium sorbs strongly to metal oxides and clay. Uranium(IV) is the other commonly found redox state. In that state, however, uranium is not very soluble and precipitates as uraninite, UO_2, coffinite, $USiO_4.nH_2O$ (if SiO_2 >60 mg/L, Henry, *et al*. 1982, p.18), or related minerals. In most aquifers, no mineral controls uranium solubility in oxidizing conditions. However, uranite and coffinite are the controlling minerals if Eh drops below 0-100 mV.

Thorium exists naturally only in one redox state Th(IV). Th^{+4} forms complexes with most common aqueous anions. However, thorium solubility remains low except perhaps at higher pH when complexed by carbonate ions (USEPA 1999). Thorium sorbs strongly to metal oxides in a way similar to uranium.

Radium has an atomic number of 88. Radium originates from the radioactive decay of uranium and thorium. Radium-226 is an intermediate product of U238 (the most common uranium isotope >99%, Table A-1) decay, whereas radium-228 belongs to the Th232 (~100% of natural thorium) decay series. Both radium isotopes further decay to radon and, ultimately, to lead. Radon is a gas and tends to volatilize from shallower units. Radium-223 and radium-224 isotopes are also naturally present but in minute quantities. Radium-224 belongs to the thorium decay series, whereas radium-223 derives from the much rarer U235 (~0.7%). Radium is an alkaline Earth element and belongs to the same group (2A in periodic table) as magnesium, calcium, strontium, and barium. It most resembles barium chemically, as evidenced by removal technologies such as ion exchange with Na and lime softening. Sorption on iron and manganese oxides is also a common trait of alkaline Earth elements. Radium exists only under one oxidation state, the divalent cation Ra^{+2}, similar to other alkaline Earth elements (Ca^{+2}, Mg^{+2}, Sr^{+2}, and Ba^{+2}). $RaSO_4$ is extremely insoluble (more so than barium sulfate), with a log K solubility product of -10.5, compared to that of barium sulfate at ~-10. Radium solubility is mostly controlled by sulfate activity.

Table E.1 Uranium, thorium, and radium abundance and half-lives

Decay series	Uranium/thorium	Radium	Radon
U238	U238 – ~99.3% (4.47×10^9 yrs)	Ra226 - (1,599 yrs)	Rn222 - (3.8 days)
	U234 – 0.0055% (0.246×10^9 yrs)	Intermediate product of U238 decay	
U235	U235 - ~0.7% (0.72×10^9 yrs)	Ra223 – (11.4 days)	Rn219 - (4 seconds)
Th232	Th232 – ~100% (14.0×10^9 yrs)	Ra228 - (5.76 yrs) Ra224 - (3.7 days)	Rn220 - (~1 min)

NOTE: half-life from Parrington, et al. (1996)

USEPA Maximum Contaminant Levels

- Uranium: 30 ppb
- Gross alpha : 15 pCi/L
- Beta particles and photon emitters: 4 mrem/yr
- Combined Radium 226 and radium 228: 5 pCi/L

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