DRAFT FEASIBILITY REPORT FEASIBILITY ANALYSIS OF WATER SUPPLY FOR SMALL PUBLIC WATER SYSTEMS

MIRANDO CITY PWS ID# 2400025, CCN# 12629

Prepared for:

THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY



Prepared by:

THE UNIVERSITY OF TEXAS BUREAU OF ECONOMIC GEOLOGY

AND

PARSONS

Preparation of this report was financed by the Texas Commission on Environmental Quality through the Drinking Water State Revolving Fund Small Systems Assistance Program

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AUGUST 2010

EXECUTIVE SUMMARY

INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Transportation Group Inc. (Parsons), was contracted by the Texas Commission on Environmental Quality (TCEQ) to conduct a project to assist with identifying and analyzing alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project was to promote compliance using sound engineering and financial methods and data for PWSs with recently recorded sample results exceeding maximum contaminant levels (MCL). The primary objectives of this project were to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS for future implementation.

This feasibility report provides an evaluation of water supply alternatives for the Mirando City Water Supply Corporation (PWS ID# 2400025, Certificate of Convenience and Necessity #12629), is located at 315 North Linder (also known as Farm-to-Market Road 649), Mirando City, Texas in eastern Webb County. The PWS is approximately 34 miles east of Laredo and 12 miles west northwest of Bruni. The Mirando City PWS is a community water system serving a population of 500 with 250 active connections. The water source for the Mirando City PWS comes from two groundwater wells, Well #1 (G2400025A) completed to a depth of 540 feet, and Well #2 (G2400025B), completed to a depth of 462 feet. Well #1 is rated at 60 gallons per minute (gpm) and Well #2 is rated at 58 gpm.

In November 2008 one arsenic sample was detected at 0.0238 milligrams per liter (mg/L). During January 2009 to June 2009, arsenic was detected at levels between 0.0115 mg/L to 0.0159 mg/L. These values exceed the MCL of 0.010 mg/L (USEPA 2010a; TCEQ 2008). Therefore, it is likely the Mirando City PWS faces potential compliance issues under the standard.

Basic system information for the Mirando City PWS is shown in Table ES.1.

Table ES.1 Mirando City PWS Basic System Information

| Population served | 500 |
|----------------------------|-------------------------------------|
| Connections | 250 |
| Average daily flow rate | 0.080 million gallons per day (mgd) |
| Peak demand flow rate | 242 gallons per minute |
| Water system peak capacity | 0.350 mgd |
| Typical arsenic range | 0.0115 – 0.0238 mg/L |

STUDY METHODS

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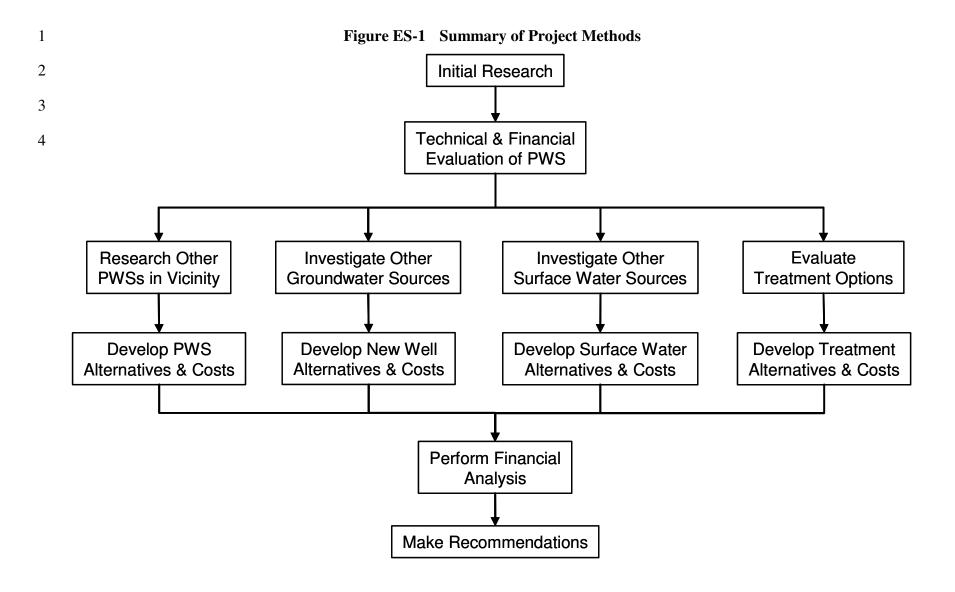
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The methods used for this project were based on a pilot project performed in 2004 and 2005 by TCEQ, BEG, and Parsons. Methods for identifying and analyzing compliance options were developed in the pilot project (a decision tree approach).

- The process for developing the feasibility study used the following general steps:
 - 1. Gather data from the TCEQ and Texas Water Development Board databases, from TCEQ files, and from information maintained by the PWS;
 - 2. Conduct financial, managerial, and technical (FMT) evaluations of the PWS;
 - 3. Perform a geologic and hydrogeologic assessment of the study area;
 - 4. Develop treatment and non-treatment compliance alternatives which, in general, consist of the following possible options:
 - a. Connecting to neighboring PWSs via new pipeline or by pumping water from a newly installed well or an available surface water supply within the jurisdiction of the neighboring PWS;
 - b. Installing new wells within the vicinity of the PWS into other aquifers with confirmed water quality standards meeting the MCLs;
 - c. Installing a new intake system within the vicinity of the PWS to obtain water from a surface water supply with confirmed water quality standards meeting the MCLs;
 - d. Treating the existing non-compliant water supply by various methods depending on the type of contaminant; and
 - e. Delivering potable water by way of a bottled water program or a treated water dispenser as an interim measure only.
 - 5. Assess each of the potential alternatives with respect to economic and non-economic criteria;
 - 6. Prepare a feasibility report and present the results to the PWS.
- This basic approach is summarized in Figure ES-1.

HYDROGEOLOGICAL ANALYSIS

The southeastern section of Webb County, where the Mirando City PWS is located, is a transition area between the Gulf Coast aquifer and the Yegua-Jackson aquifer. The two wells operated by the Mirando City Water Supply Corporation are completed to depths of 462 and 540 feet in the Jasper Formation of the Gulf Coast Aquifer. Arsenic is commonly found in area wells at concentrations greater than the MCL particularly at depths corresponding to the lower screened intervals on the Mirando City PWS wells. Natural geologic sources may be responsible for the arsenic found in the area groundwater. Historical data show that arsenic concentrations exceed the MCL. Casing the lower screened intervals or constructing new, shallower wells may yield higher quality groundwater. Also, arsenic concentrations can vary significantly over relatively short distances; as a result, there could be good quality groundwater nearby. However, the variability of arsenic concentrations makes it difficult to determine where wells can be located to produce acceptable water.



COMPLIANCE ALTERNATIVES

Mirando City PWS provides water to a small community, and is governed by a 5-member Board of Directors. Based on the team's assessment, this system has an inadequate level of capacity at this time. The people interviewed were enthusiastic about receiving the report and very receptive to receiving any type of technical assistance that is available. The system has the willingness and the potential to become a well-managed system, but there are several areas of present concern. The deficiencies noted could prevent the water system from being able to achieve compliance now or in the future and may also affect the water system's long-term sustainability. Areas of concern for the system included insufficient water production, and lack of budget and other financial records.

There are relatively few PWSs within 30 miles of Mirando City PWS. Many of these nearby systems also have water quality problems, but there are a few with good quality water. In general, feasibility alternatives were developed based on obtaining water from the nearest PWSs, either by directly purchasing water, or by expanding the existing well field. There is a minimum of surface water available in the area. Oilton Rural Water Supply Corporation and Webb County Water Utilities Colorado Acres are potential small water suppliers that could supply water to Mirando City PWS.

Centralized treatment alternatives for arsenic removal have been developed and were considered for this report including, reverse osmosis, iron-based adsorption and coagulation/filtration. Point-of-use (POU) and point-of-entry treatment alternatives were also considered. Temporary solutions such as providing bottled water or providing a centralized dispenser for treated or trucked-in water, were also considered as alternatives.

Developing a new well close to Mirando City PWS is likely to be the best solution if compliant groundwater can be found. Having a new well close to Mirando City is likely to be one of the lower cost alternatives since the PWS already possesses the technical and managerial expertise needed to implement this option. The cost of new well alternatives quickly increases with pipeline length, making proximity of the alternate source a key concern. A new compliant well or obtaining water from a neighboring compliant PWS has the advantage of providing compliant water to all taps in the system.

Central treatment can be cost-competitive with the alternative of new nearby wells, but would require significant institutional changes to manage and operate. Similar to obtaining an alternate compliant water source, central treatment would provide compliant water to all water taps.

POU treatment can be cost competitive, but does not supply compliant water to all taps. Additionally, significant efforts would be required for maintenance and monitoring of the POU treatment units.

Providing compliant water through a central dispenser is significantly less expensive than providing bottled water to 100 percent of the population, but a significant effort is required for clients to fill their containers at the central dispenser.

FINANCIAL ANALYSIS

Financial analysis of the Mirando City PWS could not be completed due to lack of expense data, although the PWS indicates that revenues have been sufficient to fund current operations. The current average water bill represents approximately 2.9 percent of the median household income (MHI). Table ES.2 provides a summary of the financial impact of implementing selected compliance alternatives, including the rate increase necessary to meet current operating expenses. The alternatives were selected to highlight results for the best alternatives from each different type or category.

Some of the compliance alternatives offer potential for shared or regional solutions. A group of PWSs could work together to implement alternatives for developing a new groundwater source or expanding an existing source, obtaining compliant water from a large regional provider, or for central treatment. Sharing the cost for implementation of these alternatives could reduce the cost on a per user basis. Additionally, merging PWSs or management of several PWSs by a single entity offers the potential for reduction in administrative costs.

1 Table ES.2 Selected Financial Analysis Results

| Alternative | Funding Option | Average Annual Water Bill | Percent of MHI |
|----------------------------|----------------|------------------------------|----------------|
| Current | NA | \$760 | 2.9 |
| To meet current expenses | NA | \$750 | 2.8 |
| Purchase Water from Oilton | 100% Grant | \$840 | 3.2 |
| Rural WSC | Loan/Bond | \$1,207 | 4.6 |
| N. W.H. (Mr. 1. Cr. | 100% Grant | \$784 | 3.0 |
| New Well at Mirando City | Loan/Bond | \$879 | 3.3 |
| | 100% Grant | \$945 | 3.6 |
| Central treatment | Loan/Bond | \$1,130 | 4.3 |
| D. C. | 100% Grant | \$1,483 | 5.6 |
| Point-of-use | Loan/Bond | \$1,542 | 5.8 |
| Dell's discourse | 100% Grant | \$890 | 3.4 |
| Public dispenser | Loan/Bond | \$895 | 3.4 |

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ACRONYMS AND ABBREVIATIONS

| μg/L | Micrograms per liter |
|-------------------|---|
| °F | Degrees Fahrenheit |
| AFY | Acre feet per year |
| ANSI | American National Standards Institute |
| BAT | Best available technology |
| BEG | Bureau of Economic Geology |
| bgs | Below ground surface |
| CA | Chemical analysis |
| CD | Community Development |
| CDBG | Community Development Block Grants |
| CCN | Certificate of Convenience and Necessity |
| CFR | Code of Federal Regulations |
| CO | Correspondence |
| CR | County Road |
| CRMWD | Colorado River Municipal Water District |
| DE | Diatomaceous earth |
| DWSRF | Drinking Water State Revolving Fund |
| ED | Electrodialysis |
| EDAP | Economically Distressed Areas Program |
| EDR | Electrodialysis reversal |
| FMT | Financial, managerial, and technical |
| GAM | Groundwater Availability Model |
| gpd | gallons per day |
| gpm | Gallons per minute |
| gpy | Gallons per year |
| ISD | Independent School District |
| IX | lon exchange |
| KMnO ₄ | Hydrous manganese oxide |
| MCL | Maximum contaminant level |
| mgd | Million gallons per day |
| mg/L | milligram per liter |
| MHI | Median household income |
| MnO ₂ | Manganese oxide |
| MOR | Monthly operating report |
| MTBE | methyl tertiary-butyl ether |
| NMEFC | New Mexico Environmental Financial Center |
| NPDWR | National Primary Drinking Water Regulations |

| Operation and Maintenance |
|---|
| Parsons Transportation Group, Inc. |
| picoCuries per liter |
| Point-of-entry |
| Point-of-use |
| Pressure-reducing valve |
| Polyvinyl chloride |
| Public water system |
| Reverse osmosis |
| Ranch Road |
| Rural Utilities Service |
| Safe Drinking Water Act |
| State Highway |
| State Revolving Fund |
| Small System Compliance Technology |
| Texas Administrative Code |
| Texas Commission on Environmental Quality |
| Texas Department of Rural Affairs |
| Total dissolved solids |
| Total suspended solids |
| Texas Water Development Board |
| Upper Guadalupe River Authority |
| United States Environmental Protection Agency |
| Water Availability Model |
| Water Treatment Technologies, Inc. |
| |

SECTION 1
INTRODUCTION

The University of Texas Bureau of Economic Geology (BEG) and its subcontractor, Parsons Transportation Group Inc. (Parsons), were contracted by the Texas Commission on Environmental Quality (TCEQ) to assist with identifying and analyzing compliance alternatives for use by Public Water Systems (PWS) to meet and maintain Texas drinking water standards.

The overall goal of this project is to promote compliance using sound engineering and financial methods and data for PWSs that have recently had sample results that exceed maximum contaminant levels (MCL). The primary objectives of this project are to provide feasibility studies for PWSs and the TCEQ Water Supply Division that evaluate water supply compliance options, and to suggest a list of compliance alternatives that may be further investigated by the subject PWS with regard to future implementation. The feasibility studies identify a range of potential compliance alternatives, and present basic data that can be used for evaluating feasibility. The compliance alternatives addressed include a description of what would be required for implementation, conceptual cost estimates for implementation, and noncost factors that could be used to differentiate between alternatives. The cost estimates are intended for comparing compliance alternatives, and to give a preliminary indication of potential impacts on water rates resulting from implementation.

It is anticipated the PWS will review the compliance alternatives in this report to determine if there are promising alternatives, and then select the most attractive alternative(s) for more detailed evaluation and possible subsequent implementation. This report contains a decision tree approach that guided the efforts for this project, and also contains steps to guide a PWS through the subsequent evaluation, selection, and implementation of a compliance alternative.

This feasibility report provides an evaluation of water supply compliance options for the Mirando City Water Supply Corporation PWS, PWS ID# 2400025, Certificate of Convenience and Necessity (CCN) #12629, located in Webb County, hereinafter referred to in this document as the "Mirando City PWS." Recent sample results from the Mirando City Water Supply Corporation exceeded the MCL for arsenic of 0.010 milligrams per liter (mg/L) (USEPA 2010a, TCEQ 2008). The location of the Mirando City PWS is shown on Figure 1.1. Various water supply and planning jurisdictions are shown on Figure 1.2. These water supply and planning jurisdictions are used in the evaluation of alternate water supplies that may be available in the area.

1.1 PUBLIC HEALTH AND COMPLIANCE WITH MCLs

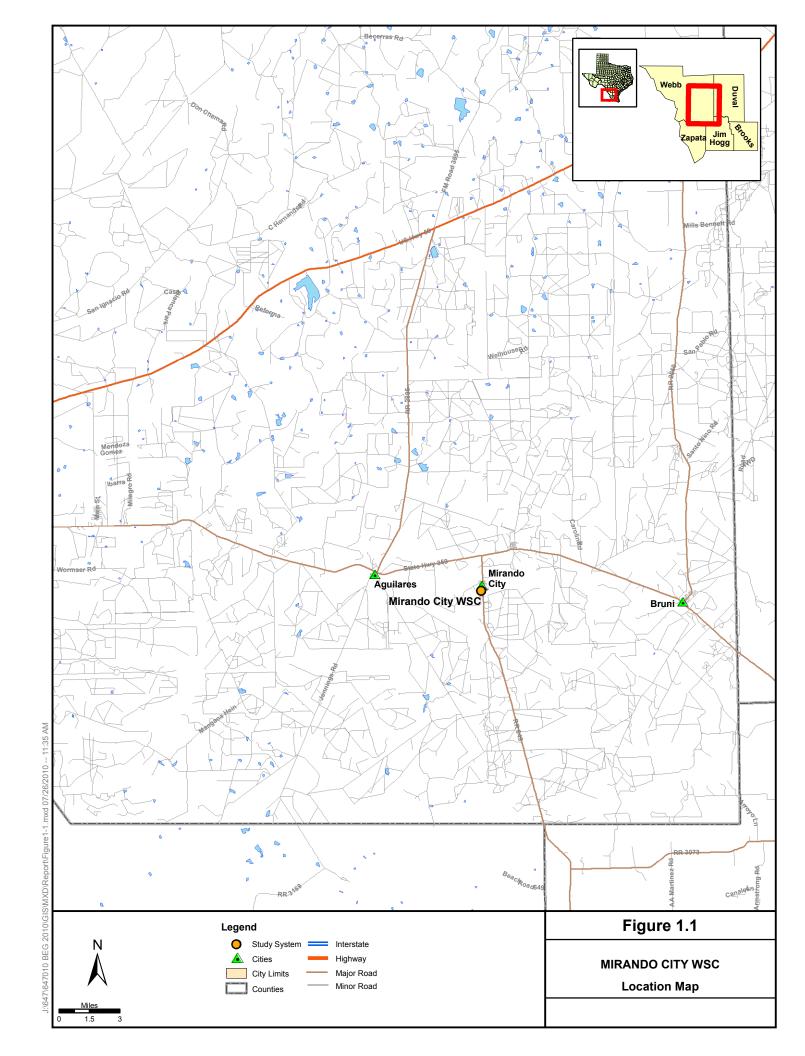
The goal of this project is to promote compliance for PWSs that supply drinking water exceeding regulatory maximum contaminant levels (MCL). This project only addresses those contaminants and does not address any other violations that may exist for a PWS. As mentioned above, the Mirando City PWS had recent sample results exceeding the MCL for arsenic. In general, contaminant(s) in drinking water above the MCL(s) can have both short-

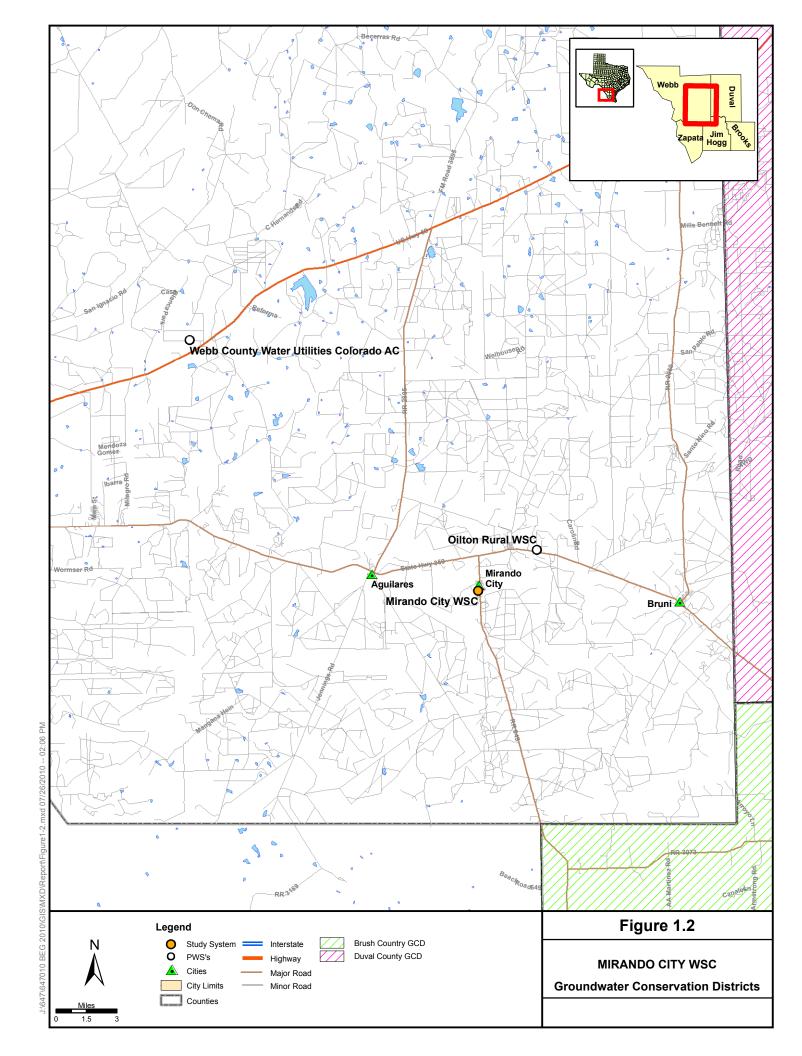
- term (acute) and long-term or lifetime (chronic) effects. According to the USEPA, potential
- 2 health effects from long-term ingestion of water with levels of arsenic above the MCL
- 3 (0.01 mg/L) include non-cancerous effects, such as thickening and discoloration of the skin,
- 4 stomach pain, nausea, vomiting, diarrhea, numbness in hands and feet, partial paralysis, and
- 5 blindness, and cancerous effects, including skin, bladder, lung, kidney, nasal passage, liver and
- 6 prostate cancer (USEPA 2010b).

1.2 METHOD

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- The method for this project follows that of a pilot project performed by TCEQ, BEG, and Parsons. The pilot project evaluated water supply alternatives for PWSs that supplied drinking water with contaminant concentrations above U.S. Environmental Protection Agency (USEPA) and Texas drinking water standards. Three PWSs were evaluated in the pilot project to develop the method (*i.e.*, decision tree approach) for analyzing options for provision of compliant drinking water. This project is performed using the decision tree approach that was developed for the pilot project, and which was also used for subsequent projects.
- Other tasks of the feasibility study are as follows:
- Identifying available data sources;
- Gathering and compiling data;
- Conducting financial, managerial, and technical (FMT) evaluations of the selected PWSs;
- Performing a geologic and hydrogeologic assessment of the area;
- Developing treatment and non-treatment compliance alternatives;
- Assessing potential alternatives with respect to economic and non-economic criteria;
- Preparing a feasibility report; and
- Suggesting refinements to the approach for future studies.





The remainder of Section 1 of this report addresses the regulatory background, and provides a summary of radium abatement options. Section 2 describes the method used to develop and assess compliance alternatives. The groundwater sources of arsenic are addressed in Section 3. Findings for the Mirando City PWS, along with compliance alternatives development and evaluation, can be found in Section 4. Section 5 references the sources used in this report.

1.3 REGULATORY PERSPECTIVE

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- The Utilities & Districts and Public Drinking Water Sections of the TCEQ Water Supply
 Division are responsible for implementing requirements of the Federal Safe Drinking Water
 Act (SDWA) which include oversight of PWSs and water utilities. These responsibilities
 include:
- Monitoring public drinking water quality;
 - Processing enforcement referrals for MCL violators;
- Tracking and analyzing compliance options for MCL violators;
- Providing FMT assessment and assistance to PWSs;
- Participating in the Drinking Water State Revolving Fund program to assist PWSs in achieving regulatory compliance; and
- Setting rates for privately owned water utilities.
- This project was conducted to assist in achieving these responsibilities.

20 1.4 ABATEMENT OPTIONS

21 When a PWS exceeds a regulatory MCL, the PWS must take action to correct the Potential MCL exceedances at the Mirando City PWS involve arsenic. 22 violation. The 23 following subsections explore alternatives considered as potential options for 24 obtaining/providing compliant drinking water.

1.4.1 Existing Public Water Supply Systems

A common approach to achieving compliance is for the PWS to make arrangements with a neighboring PWS for water supply. For this arrangement to work, the PWS from which water is being purchased (supplier PWS) must have water in sufficient quantity and quality, the political will must exist, and it must be economically feasible.

1.4.1.1 Quantity

For purposes of this report, quantity refers to water volume, flow rate, and pressure.
Before approaching a PWS as a potential supplier, the non-compliant PWS should determine its
water demand on the basis of average day and maximum day. Peak instantaneous demands can
be met through proper sizing of storage facilities. Further, the potential for obtaining the

appropriate quantity of water to blend to achieve compliance should be considered. The concept of blending involves combining water with low levels of contaminants with non-compliant water in sufficient quantity that the resulting blended water is compliant. The exact blend ratio would depend on the quality of the water a potential supplier PWS can provide, and would likely vary over time. If high quality water is purchased, produced or otherwise obtained, blending can reduce the amount of high quality water required. Implementation of blending will require a control system to ensure the blended water is compliant.

If the supplier PWS does not have sufficient quantity, the non-compliant community could pay for the facilities necessary to increase the quantity to the extent necessary to supply the needs of the non-compliant PWS. Potential improvements might include, but are not limited to:

• Additional wells:

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- Developing a new surface water supply,
- Additional or larger-diameter piping;
- Increasing water treatment plant capacity
- Additional storage tank volume;
- Reduction of system losses,
- Higher-pressure pumps; or
 - Upsized, or additional, disinfection equipment.

In addition to the necessary improvements, a transmission pipeline would need to be constructed to tie the two PWSs together. The pipeline must tie-in at a point in the supplier PWS where all the upstream pipes and appurtenances are of sufficient capacity to handle the new demand. In the non-compliant PWS, the pipeline must tie in at a point where no downstream bottlenecks are present. If blending is the selected method of operation, the tie-in point must be selected to ensure all the water in the system is blended to achieve regulatory compliance.

1.4.1.2 Quality

If a potential supplier PWS obtains its water from the same aquifer (or same portion of the aquifer) as the non-compliant PWS, the quality of water may not be significantly better. However, water quality can vary significantly due to well location, even within the same aquifer. If localized areas with good water quality cannot be identified, the non-compliant PWS would need to find a potential supplier PWS that obtains its water from a different aquifer or from a surface water source. Additionally, a potential supplier PWS may treat non-compliant raw water to an acceptable level.

Surface water sources may offer a potential higher-quality source. Since there are significant treatment requirements, utilization of surface water for drinking water is typically

- 1 most feasible for larger local or regional authorities or other entities that may provide water to
- 2 several PWSs. Where PWSs that obtain surface water are neighbors, the non-compliant PWS
- 3 may need to deal with those systems as well as with the water authorities that supply the surface
- 4 water.

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1.4.2 Potential for New Groundwater Sources

1.4.2.1 Existing Non-Public Supply Wells

Often there are wells not associated with PWSs located in the vicinity of the non-compliant PWS. The current use of these wells may be for irrigation, industrial purposes, domestic supply, stock watering, and other purposes. The process for investigating existing wells is as follows:

- Existing data sources (see below) will be used to identify wells in the areas that have satisfactory quality. For the Mirando City PWS, the following standards could be used in a rough screening to identify compliant groundwater in surrounding systems:
 - Nitrate (measured as nitrogen) concentrations less than 8 milligrams per liter (mg/L) (below the MCL of 10 mg/L);
 - \circ Fluoride concentration less than 2.0 mg/L (below the Secondary MCL of 2 mg/L);
 - o Arsenic concentration less than 0.008 mg/L (below the MCL of 0.01 mg/L);
 - o Uranium concentration less than 0.024 mg/L (below the MCL of 0.030 mg/L; and
 - o Selenium concentration less than 0.04 mg/L (below the MCL of 0.05 mg/L).
- The recorded well information will be reviewed to eliminate those wells that appear to be unsuitable for the application. Often, the "Remarks" column in the Texas Water Development Board (TWDB) hard-copy database provides helpful information. Wells eliminated from consideration generally include domestic and stock wells, dug wells, test holes, observation wells, seeps, and springs, destroyed wells, wells used by other communities, etc.
- Wells of sufficient size are identified. Some may be used for industrial or irrigation purposes. Often the TWDB database will include well yields, which may indicate the likelihood that a particular well is a satisfactory source.
- At this point in the process, the local groundwater control district (if one exists) should be contacted to obtain information about pumping restrictions. Also, preliminary cost estimates should be made to establish the feasibility of pursuing further well development options.
- If particular wells appear to be acceptable, the owner(s) should be contacted to ascertain their willingness to work with the PWS. Once the owner agrees to participate in the program, additional data should be collected to characterize the quality and quantity of the well water. Many owners have more than one well, and

- would probably be the best source of information regarding the latest test dates, who tested the water, flow rates, and other well characteristics.
 - After collecting as much information as possible from cooperative owners, the non-compliant PWS would then narrow the selection of wells and sample and analyze them for quality. Wells with good quality water would then be potential candidates for test pumping. In some cases, a particular well may need to be refurbished before test pumping. Information obtained from test pumping would then be used in combination with information about the general characteristics of the aquifer to determine whether a well at that location would be suitable as a supply source.
 - Where financial resources allow, it is recommended that new wells be installed instead of using existing wells to ensure the well characteristics are known and the well meets current construction standards.
 - Permit(s) would then be obtained from the groundwater control district or other regulatory authority, and an agreement with the owner (purchase or lease, access easements, etc.) would then be negotiated.

1.4.2.2 Develop New Wells

If no existing wells are available for development, the PWS or group of PWSs has an option of developing new wells. Records of existing wells, along with other hydrogeologic information and modern geophysical techniques, should be used to identify potential locations for new wells. In some areas, the TWDB's Groundwater Availability Model (GAM) may be applied to indicate potential sources. Once a general area is identified, landowners and regulatory agencies should be contacted to determine an exact location for a new well or well field. Pump tests and water quality tests would be required to determine if a new well will produce an adequate quantity of good quality water. Permits from the local groundwater control district or other regulatory authority could also be required for a new well.

1.4.3 Potential for Surface Water Sources

Water rights law dominates the acquisition of water from surface water sources. For a PWS, 100 percent availability of water is required, except where a back-up source is available. For PWSs with an existing water source, although it may be non-compliant because of elevated concentrations of one or more parameters, water rights may not need to be 100 percent available.

1.4.3.1 Existing Surface Water Sources

"Existing surface water sources" of water refers to municipal water authorities and cities that obtain water from surface water sources. The process of obtaining water from such a source is generally less time consuming and less costly than the process of developing a new source; therefore, it should be a primary course of investigation. An existing source would be limited by its water rights, the safe yield of a reservoir or river, or by its water treatment or water conveyance capability. The source must be able to meet the current demand and honor

1 contracts with communities it currently supplies. In many cases, the contract amounts reflect 2 projected future water demand based on population or industrial growth.

A non-compliant PWS would look for a source with sufficient spare capacity. Where no such capacity exists, the non-compliant PWS could offer to fund the improvements necessary to obtain the capacity. This approach would work only where the safe yield could be increased (perhaps by enlarging a reservoir) or where treatment capacity could be increased. In some instances water rights, where they are available, could possibly be purchased.

In addition to securing the water supply from an existing source, the non-compliant PWS would need to arrange for transmission of the water to the PWS. In some cases, that could require negotiations with, contracts with, and payments to an intermediate PWS (an intermediate PWS is one where the infrastructure is used to transmit water from a "supplier" PWS to a "supplied" PWS, but does not provide any additional treatment to the supplied water). The non-compliant PWS could be faced with having to fund improvements to the intermediate PWS in addition to constructing its own necessary transmission facilities.

1.4.3.2 New Surface Water Sources

Communication with the TCEQ and relevant planning groups from the beginning is essential in the process of obtaining a new surface water source. Preliminary assessment of the potential for acquiring new rights may be based on surface water availability maps located on the TWDB website. Where water rights appear to be available, the following activities need to occur:

- Discussions with TCEQ to indicate the likelihood of obtaining those rights. The TCEQ may use the Water Availability Model (WAM) to assist in the determination.
- Discussions with landowners to indicate potential treatment plant locations.
- Coordination with U.S. Army Corps of Engineers and local river authorities.
- Preliminary engineering design to determine the feasibility, costs, and environmental issues of a new treatment plant.

Should these discussions indicate that the best option is a new surface water source, the community would proceed with more intensive planning (initially obtaining funding), permitting, land acquisition, and detailed designs.

1.4.4 Identification of Treatment Technologies

- In January 2001, the USEPA published a final rule in the Federal Register that established an MCL for arsenic of 0.01 mg/L (USEPA 2010a). The regulation applies to all community water systems and non-transient, non-community water systems, regardless of size.
- The new arsenic MCL of 0.01 mg/L became effective January 23, 2006, at which time the running average annual arsenic level would have to be at or below 0.01 mg/L at each entry

point to the distribution system, although point-of-use (POU) treatment could be instituted in place of centralized treatment. All surface water systems had to complete initial monitoring for the new arsenic MCL or have a state-approved waiver by December 31, 2006. All groundwater systems are to have completed initial monitoring or have a state-approved waiver by December 31, 2007.

Various treatment technologies were also investigated as compliance alternatives for treatment of arsenic to regulatory levels (*i.e.*, MCL). According to a recent USEPA report for small water systems with less than 10,000 customers (EPA/600/R-05/001) a number of drinking water treatment technologies are available to reduce arsenic concentrations in source water to below the new MCL of 0.01 mg/L, including:

• Ion exchange (IX);

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- Reverse osmosis (RO);
- Electrodialysis reversal (EDR);
- Adsorption; and
- Coagulation/filtration.

1.4.5 Description of Treatment Technologies

Many of the most effective arsenic removal processes available are iron-based treatment technologies such as chemical coagulation/filtration with iron salts and adsorptive media with iron-based products. These processes are particularly effective at removing arsenic from aqueous systems because iron surfaces have a strong affinity for adsorbing arsenic. Other arsenic removal processes such as activated alumina and enhanced lime softening are more applicable to larger water systems because of their operational complexity and cost. A description and discussion of arsenic removal technologies applicable to smaller systems follow.

1.4.5.1 Ion Exchange

<u>Process</u> – In solution, salts separate into positively charged cations and negatively charged anions. Ion exchange is a reversible chemical process in which ions attached to an insoluble, permanent, solid resin bed are exchanged for ions in water. The process relies on the fact that certain ions are preferentially adsorbed on the ion exchange resin. Operation begins with a fully charged cation or anion bed, having enough positively or negatively charged ions to carry out the cation or anion exchange. Usually a polymeric resin bed is composed of millions of spherical beads about the size of medium sand grains. As water passes the resin bed, the charged ions are released into the water, being substituted or replaced with the contaminants in the water (IX). When the resin becomes exhausted of positively or negatively charged ions, the bed must be regenerated by passing a strong, sodium chloride solution over the resin bed, displacing the contaminant ions with sodium ions for cation exchange and chloride ion for anion exchange. Many different types of resins can be used to reduce dissolved contaminant concentrations. The IX treatment train for groundwater typically includes cation or anion resin

beds with a regeneration system, chlorine disinfection, and clear well storage. Treatment trains for surface water may also include raw water pumps, debris screens, and filters for pretreatment. Additional treatment or management of the concentrate and the removed solids will be necessary prior to disposal. For arsenic removal, an anion exchange resin in the chloride form is used to remove arsenate [As(V)]. Because arsenite [As(III)] occurs in water below pH 9 with no ionic charge, As(III) is not consistently removed by the anionic exchange process.

<u>Pretreatment</u> – Pretreatment guidelines are available on accepted limits for pH, organics, turbidity, and other raw water characteristics. Pretreatment may be required to reduce excessive amounts of total suspended solids (TSS), iron, and manganese, which could plug the resin bed, and typically includes media or carbon filtration. In addition, chlorination or oxidation may be required to convert As(III) to As(V) for effective removal.

<u>Maintenance</u> – The IX resin requires regular on-site regeneration, the frequency of which depends on raw water characteristics, the contaminant concentration, and the size and number of IX vessels. Many systems have undersized the IX vessels only to realize higher than necessary operating costs. Preparation of the sodium chloride solution is required. If used, a pretreatment filter would require filter replacement and/or backwashing.

<u>Waste Disposal</u> – Approval from local authorities is usually required for disposal of concentrate from the regeneration cycle (highly concentrated salt solution); occasional solid waste (in the form of broken resin beads) that are backwashed during regeneration and, if used, spent filters and backwash wastewater.

Advantages (IX)

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- Well established process for arsenic removal.
- Fully automated and highly reliable process.
- Suitable for small and large installations.

Disadvantages (IX)

- Requires salt storage; regular regeneration.
- Disposal of spent regenerate containing high salt and arsenic levels.
- Resins are sensitive to the presence of competing ions such as sulfate.
- Oxidation via pre-chlorination required if source water arsenic occurs as the arsenite [As(III)] species.

In considering application of IX for inorganics removal, it is important to understand what the effect of competing ions will be, and to what extent the brine can be recycled. Similar to activated alumina, IX exhibits a selectivity sequence, which refers to an order in which ions are preferred. Sulfate competes with both nitrate and arsenic, but more aggressive with arsenic in anion exchange. Source waters with total dissolved solids (TDS) levels above 500 mg/L or 120 mg/L sulfate are not amenable to IX treatment for arsenic removal. Spent regenerant is produced during IX bed regeneration, and this spent regenerant may have high concentrations of sorbed contaminants that can be expensive to treat and/or dispose. Research was conducted

to minimize this effect; recent research on arsenic removal shows that the brine can be reduced as many as 25 times.

1.4.5.2 Reverse Osmosis

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Process – RO is a pressure-driven membrane separation process capable of removing dissolved solutes from water by means of molecule size and electrical charge. The raw water is typically called feed; the product water is called permeate, and the concentrated reject is called concentrate. Common RO membrane materials include asymmetric cellulose acetate and polyamide thin film composite. Common RO membrane configurations include spiral wound hollow fine fiber, but most of RO systems to date are of the spiral wound type. A typical RO installation includes a high pressure feed pump with chemical feed; parallel first and second stage membrane elements in pressure vessels; and valves and piping for feed, permeate, and concentrate streams. Factors influencing membrane selection are cost, recovery, rejection, raw water characteristics, and pretreatment. Factors influencing performance are raw water characteristics, pressure, temperature, and regular monitoring and maintenance. RO is capable of achieving over 97 percent removal of As(V). Reported removals of As(III) have varied greatly, some being as low as only 5 percent. The treatment process is relatively insensitive to pH. Water recovery is typically 60-80 percent, depending on the raw water characteristics. The concentrate volume for disposal can be significant.

<u>Pretreatment</u> – RO requires careful review of raw water characteristics and pretreatment needs to prevent membranes from fouling, scaling or other membrane degradation. Removal or sequestering of suspended and colloidal solids is necessary to prevent fouling, and removal of sparingly soluble constituents such as calcium, magnesium, silica, sulfate, barium, etc. may be required to prevent scaling. Pretreatment can include media filters, ion exchange softening, acid and antiscalant feed, activated carbon of bisulfite feed to dechlorinate, and cartridge filters to remove any remaining suspended solids to protect membranes from upsets.

<u>Maintenance</u> – Monitoring rejection percentage is required to ensure contaminant removal below MCL. Regular monitoring of membrane performance is necessary to determine fouling, scaling, or other membrane degradation. Acidic or caustic solutions are regularly flushed through the system at high volume/low pressure with a cleaning agent to remove foulants and scalants. Frequency of membrane replacement is dependent on raw water characteristics, pretreatment, and maintenance. With good operation and pretreatment, membranes can last three to five years.

<u>Waste Disposal</u> – Pretreatment waste streams, concentrate flows, spent filters, and membrane elements all require approved disposal methods.

Advantages (RO)

- Can remove As(V) effectively; and in some cases As(III).
- Can remove other undesirable dissolved constituents and excessive TDS, if required.

Disadvantages (RO)

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- Relatively expensive to install and operate.
- Need sophisticated monitoring systems.
- Need to handle multiple chemicals.
 - Waste of water because of the significant concentrate flows
 - High silica concentrations (>35 mg/L) may limit water recovery rate
 - Concentrate disposal required.

RO is a relatively expensive alternative to remove arsenic and is usually not economically competitive with other processes unless nitrate and/or TDS removal is also required. The biggest drawback for using RO to remove arsenic is the waste of water through concentrate disposal, which is also difficult or expensive because of the large volumes involved.

1.4.5.3 Electrodialysis Reversal

Process. EDR is an electrochemical process in which ions migrate through ion-selective semi-permeable membranes as a result of their attraction to two electrically charged electrodes. A typical EDR system includes a membrane stack with a number of cell pairs, each consisting of a cation transfer membrane, a demineralized flow spacer, an anion transfer membrane, and a concentrate flow spacer. Electrode compartments are at opposite ends of the stack. The influent feed water (chemically treated to prevent precipitation) and the concentrated reject flow in parallel across the membranes and through the demineralized and concentrate flow spaces, respectively. The electrodes are continually flushed to reduce fouling or scaling. Careful consideration of flush feed water is required. Typically, the membranes are cation or anion exchange resins cast in sheet form; the spacers are high-density polyethylene; and the electrodes are inert metal. EDR stacks are tank-contained and often staged. Membrane selection is based on review of raw water characteristics. A single-stage EDR system usually removes 40-50 percent of arsenic and TDS. Additional stages are required to achieve higher removal efficiency if necessary. EDR uses the technique of regularly reversing the polarity of the electrodes, thereby freeing accumulated ions on the membrane surface. This process requires additional plumbing and electrical controls, but it increases membrane life, may require less added chemicals, and eases cleaning. The conventional EDR treatment train typically includes EDR membranes, chlorine disinfection, and clearwell storage. Treatment of surface water may also require pretreatment steps such as raw water pumps, debris screens, rapid mix with addition of a coagulant, slow mix flocculator, sedimentation basin or clarifier, and gravity filters. Microfiltration could be used in placement of flocculation, sedimentation, and filtration. Additional treatment or management of the concentrate and the removed solids would be necessary prior to disposal.

<u>Pretreatment.</u> There are pretreatment requirements for pH, organics, turbidity, and other raw water characteristics. EDR typically requires chemical feed to prevent scaling, acid addition for pH adjustment, and a cartridge filter for prefiltration. If arsenite [As(III)] occurs,

oxidation via pre-chlorination is required since the arsenite specie at pH below 9 has no ionic charge and will not be removed by EDR.

Maintenance. EDR membranes are durable, can tolerate a pH range from 1 to 10, and temperatures to 115 degrees Fahrenheit (°F) for cleaning. They can be removed from the unit and scrubbed. Solids can be washed off by turning the power to the electrodes off and letting water circulate through the stack. Electrode washes flush out byproducts of electrode reaction. The byproducts are hydrogen, formed in the cathode space, and oxygen and chlorine gas, formed in the anode space. If the chlorine is not removed, toxic chlorine gas may form. Depending on raw water characteristics, the membranes would require regular maintenance or replacement (four to six years). EDR requires reversing the polarity. Flushing at high volume/low pressure continuously is required to clean electrodes. If used, pretreatment filter replacement and backwashing would be required. The EDR stack must be disassembled, mechanically cleaned, and reassembled at regular intervals.

<u>Waste Disposal</u>. Highly concentrated reject flows, electrode cleaning flows, and spent membranes require approved disposal methods. Pretreatment processes and spent materials also require approved disposal methods.

Advantages (EDR)

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- EDR can operate with minimal fouling or scaling or chemical addition.
- Low pressure requirements; typically quieter than RO.
- Long membrane life expectancy; EDR extends membrane life and reduces maintenance.
 - More flexible than RO in tailoring treated water quality requirements.
 - Removes many constituents in addition to arsenic.

24 Disadvantages (EDR)

- Not suitable for high levels of iron, manganese, and hydrogen sulfide.
- High energy usage at higher TDS water.
- Waste of water because of the significant concentrate flows.
 - Generates relatively large saline waste stream requiring disposal.
 - Pre-oxidation required for arsenite (if present).

EDR can be quite expensive to run because of the energy it uses. However, EDR is generally automated, which allows for easier use by small systems. It can be used to simultaneously reduce arsenic and TDS.

1.4.5.4 Adsorption

<u>Process</u> – The adsorptive media process is a fixed-bed process by which ions in solution, such as arsenic, are removed by available adsorptive sites on an adsorptive media. When the

available adsorptive sites are filled, spent media may be regenerated or simply thrown away and Granular activated alumina was the first adsorptive media replaced with new media. successfully applied for the removal of arsenic from water supplies. More recently, other adsorptive media (mostly iron-based) have been developed and marketed for arsenic removal. Recent USEPA studies demonstrated that iron-based adsorption media typically have much higher arsenic removal capacities compared to alumina-based media. In the USEPA-sponsored Round 1 full-scale demonstration of arsenic removal technologies for small water systems program, the selected arsenic treatment technologies included nine adsorptive media systems, one IX system, one coagulation/filtration system, and one process modification.

The selected adsorptive media systems used four different adsorptive media, including three iron-based media (e.g., ADI's G2, Severn Trent and AdEdge's E33, and U.S. Filter's GFH), and one iron-modified AA media (e.g., Kinetico's AAFS50, a product of Alcan). The G2 media is a dry powder of diatomaceous earth impregnated with a coating of ferric hydroxide, developed by ADI specifically for arsenic adsorption. ADI markets G2 for both As(V) and As(III) removal, but it preferentially removes As(V). G2 media adsorbs arsenic most effectively at pH values within the 5.5 to 7.5 range, and less effectively at a higher pH value.

The Bayoxide E33 media was developed by Bayer AG for removal of arsenic from drinking water supplies. It is a dry granular iron oxide media designed to remove dissolved arsenic via adsorption onto its ferric oxide surface. Severn Trent markets the media in the United States for As(III) and As(V) removal as Sorb-33, and offers several arsenic package units (APU) with flowrates ranging from 150 to 300 gallons per minute (gpm). Another company, AdEdge, provides similar systems using the same media (marketed as AD-33) with flowrates ranging from 5 to 150 gpm. E33 adsorbs arsenic and other ions, such as antimony, cadmium, chromate, lead, molybdenum, selenium, and vanadium. The adsorption is effective at pH values ranging between 6.0 and 9.0. At greater than 8.0 to 8.5, pH adjustment is recommended to maintain its adsorption capacity. Two competing ions that can reduce the adsorption capacity are silica (at levels greater than 40 mg/L) and phosphate (at levels greater than 1 mg/L).

GFH is a moist granular ferric hydroxide media produced by GFH Wasserchemie GmbH of Germany and marketed by U.S. Filter under an exclusive marketing agreement. GFH is capable of adsorbing both As(V) and As(III). GFH media adsorb arsenic with a pH range of 5.5 to 9.0, but less effectively at the upper end of this range. Competing ions such as silica and phosphate in source water can adsorb onto GFH media, thus reducing the arsenic removal capacity of the media.

The AAFS50 is a dry granular media of 83 percent alumina and a proprietary iron-based additive to enhance the arsenic adsorption performance. Standard AA was the first adsorptive media successfully applied for the removal of arsenic from water supplies. However, it often requires pH adjustment to 5.5 to achieve optimum arsenic removal. The AAFS50 product is modified with an iron-based additive to improve its performance and increase the pH range within which it can achieve effective removal. Optimum arsenic removal efficiency is achieved

with a pH of the feed water less than 7.7. Competing ions such as fluoride, sulfate, silica, and phosphate can adsorb onto AAFS50 media, and potentially reduce its arsenic removal capacity. The adsorption capacity of AAFS50 can be impacted by both high levels of silica (>40 mg/L) and phosphate (>1 mg/L). The vendor recommended that the system be operated in a series configuration to minimize the chance for arsenic breakthrough to impact drinking water quality.

All iron-based or iron-modified adsorptive media are of the single use or throwaway type after exhaustion. The operations of these adsorption systems are quite similar and simple. Some of the technologies such as the E33 and GFH media have been operated successfully on large scale plants in Europe for several years.

<u>Pretreatment</u> – The adsorptive media are primarily used to remove dissolved arsenic and not for suspended solids removal. Pretreatment to remove TSS may be required if raw water turbidity is >0.3 NTU. However, most well waters are low in turbidity and hence, pre-filtration is usually not required. Pre-chlorination may be required to oxidize As(III) to As(V) if the proportion of As(III) is high. No pH adjustment is required unless pH is relatively high.

Maintenance – Maintenance for the adsorption media system is minimal if no pretreatment is required. Backwash is required infrequently (monthly) to remove silt and sediments that occur in source waters and replacement and disposal of the exhausted media occur between one to three years, depending on average water consumption, the concentrations of arsenic and competing ions in the raw water, the media bed volume and the specific media used.

<u>Waste Disposal</u> – If no pretreatment is required there is minimal waste disposal involved with the adsorptive media system. Disposal of backwash wastewater is required especially during startup. Regular backwash is infrequent, and disposal of the exhausted media occurs once every one to three years, depending on operating conditions. The exhausted media are usually considered non-hazardous waste.

Advantages (Adsorption)

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- Some adsorbents can remove both As(III) and As(V); and
- Very simple to operate.
 - Selective to arsenic.
 - Long media lives.
 - Spent media generally not classified as hazardous.

Disadvantages (Adsorption)

- Relatively new technology; and
- Need replacement of adsorption media when exhausted.

The adsorption media process is the most simple and requires minimal operator attention compared to other arsenic removal processes. The process is most applicable to small wellhead systems with low or moderate arsenic concentrations with no treatment process in place (e.g., iron and manganese removal; if treatment facilities for iron and/or manganese removal are

already in place, incorporating ferric chloride coagulation in the existing system would be a more cost-effective alternative for arsenic removal). The choice of media will depend on raw water characteristics, life cycle cost, and experience of the vendor. Many of the adsorption media have been demonstrated at the field-trial stage, while others are in full-scale applications throughout Europe and the United States. Pilot testing may or may not be necessary prior to implementation depending on the experience of the vendor with similar water characteristics.

1.4.5.5 Coagulation/Filtration and Iron Removal Technologies

<u>Process</u> – Iron oxides have an affinity for arsenic and iron removal processes can be used to removal arsenic from drinking water supplies. The iron filtration can be accomplished with granular media filter or microfilter. For effective arsenic removals, there needs to be a minimum amount of iron present in the source water. When iron in the source water is inadequate, an iron salt such as ferric chloride is added to the water to form ferric hydroxide. The iron removal process is commonly called coagulation/filtration because iron in the form of ferric chloride is a common coagulant. The actual capacity to remove arsenic during iron removal depends on a number of factors, including the amount of arsenic present, arsenic speciation, pH, amount and form of iron present, and existence of competing ions, such as phosphate, silicate, and natural organic matter. The filters used in groundwater treatment are usually pressure filters fed directly by the well pumps. The filter media can be regular dual media filters or proprietary media such as the engineered ceramic filtration media, Macrolite, developed by Kinetico. Macrolite is a low-density, spherical media designed to allow for filtration rates up to 10 gpm/ft², which is a higher loading rate than commonly used for conventional filtration media.

<u>Pretreatment</u> – Pre-chlorination to oxidize As(III) to As(V) is usually required for most groundwater sources since As(V) adsorbs to the iron much more strongly than As(III). The adjustment of pH is required only for relatively high pH value. Coagulation with the feed of ferric chloride is required for this process. Sometimes a 5-minute contact tank is required ahead the filters if the pH is high.

<u>Maintenance</u> – Maintenance is mainly to handle ferric chloride chemical and feed system, and for regular backwash of the filters. No filter replacement is required for this process.

<u>Waste Disposal</u> – The waste from the coagulation/filtration process is mainly the iron hydroxide sludge with adsorbed arsenic in the backwash water. The backwash water can be discharged to a public sewer if it is available. If a sewer is not available, the backwash water can be discharged to a storage and settling tank from where the supernatant is recycled in a controlled rate to the front of the treatment system and the settled sludge can be disposed periodically to a landfill. The iron hydroxide sludge is usually not classified as hazardous waste.

Advantages (Coagulation/Filtration)

- Very established technology for arsenic removal; and
- Often an economical process for arsenic removal.

Disadvantages (Coagulation/Filtration)

• Need to handle chemical:

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- Need to dispose of regular backwash wastewater; and
- Need to dispose of sludge.

The coagulation/filtration process is usually the most economical arsenic removal alternative, especially if a public sewer is available for accepting the discharge of the backwash water. However, because of the regular filter backwash requirements, more operation and maintenance attention is required from the utilities. Because of potential interference by competing ions bench-scale or pilot scaling testing may be required to ensure that the arsenic MCL can be met with this process alternative

1.4.6 Point-of-Entry and Point-of-Use Treatment Systems

Point-of-entry (POE) and Point-of-use (POU) treatment devices or systems rely on many of the same treatment technologies used in central treatment plants. However, while central treatment plants treat all water distributed to consumers to the same level, POU and POE treatment devices are designed to treat only a portion of the total flow. POU devices treat only the water intended for direct consumption, typically at a single tap or limited number of taps, while POE treatment devices are typically installed to treat all water entering a single home, business, school, or facility. POU and POE treatment systems may be an option for PWSs where central treatment is not affordable. Updated USEPA guidance on use of POU and POE treatment devices is provided in "Point-of-Use or Point-of-Entry Treatment Options for Small Drinking Water Systems," EPA 815-R-06-010, April 2006 (USEPA 2006).

Point-of-entry and POU treatment systems can be used to provide compliant drinking water. These systems typically use small adsorption or reverse osmosis treatment units installed "under the sink" in the case of POU, and where water enters a house or building in the case of POE. It should be noted that the POU treatment units would need to be more complex than units typically found in commercial retail outlets to meet regulatory requirements, making purchase and installation more expensive. Point-of-entry and POU treatment units would be purchased and owned by the PWS. These solutions are decentralized in nature, and require utility personnel entry into houses or at least onto private property for installation, maintenance, and testing. Due to the large number of treatment units that would be employed and would be largely out of the control of the PWS, it is very difficult to ensure 100 percent compliance. Prior to selection of a POE or POU program for implementation, consultation with TCEQ would be required to address measurement and determination of level of compliance.

The National Primary Drinking Water Regulations (NPDWR), 40 CFR Section 141.100, covers criteria and procedures for PWSs using POE devices and sets limits on the use of these devices. According to the regulations (July 2005 Edition), the PWS must develop and obtain TCEQ approval for a monitoring plan before POE devices are installed for compliance with an MCL. Under the plan, POE devices must provide health protection equivalent to central water treatment meaning the water must meet all NPDWR and would be of acceptable quality similar

to water distributed by a well-operated central treatment plant. In addition, monitoring must include physical measurements and observations such as total flow treated and mechanical condition of the treatment equipment. The system would have to track the POE flow for a given time period, such as monthly, and maintain records of device inspection. The monitoring plan should include frequency of monitoring for the contaminant of concern and number of units to be monitored. For instance, the system may propose to monitor every POE device during the first year for the contaminant of concern and then monitor one-third of the units annually, each on a rotating schedule, so each unit would be monitored every three years. To satisfy the requirement that POE devices must provide health protection, the water system may be required to conduct a pilot study to verify the POE device can provide treatment equivalent to central treatment. Every building connected to the system must have a POE device installed, maintained, and properly monitored. Additionally, TCEQ must be assured that every building is subject to treatment and monitoring, and that the rights and responsibilities of the PWS customer convey with title upon sale of property.

Effective technology for POE devices must be properly applied under the monitoring plan approved by TCEQ and the microbiological safety of the water must be maintained. TCEQ requires adequate certification of performance, field testing, and, if not included in the certification process, a rigorous engineering design review of the POE devices. The design and application of the POE devices must consider the tendency for increase in heterotrophic bacteria concentrations in water treated with activated carbon. It may be necessary to use frequent backwashing, post-contactor disinfection, and Heterotrophic Plate Count monitoring to ensure that the microbiological safety of the water is not compromised.

The SDWA [§1412(b)(4)(E)(ii)] regulates the design, management and operation of POU and POE treatment units used to achieve compliance with an MCL. The requirements associated with these regulations, relevant to MCL compliance are:

- POU and POE treatment units must be owned, controlled, and maintained by the water system, although the utility may hire a contractor to ensure proper O&M and MCL compliance. The water system must retain unit ownership and oversight of unit installation, maintenance and sampling; the utility ultimately is the responsible party for regulatory compliance. The water system staff need not perform all installation, maintenance, or management functions, as these tasks may be contracted to a third party-but the final responsibility for the quality and quantity of the water supplied to the community resides with the water system, and the utility must monitor all contractors closely. Responsibility for O&M of POU or POE devices installed for SDWA compliance may not be delegated to homeowners.
- POU and POE units must have mechanical warning systems to automatically notify customers of operational problems. Each POU or POE treatment device must be equipped with a warning device (e.g., alarm, light) that would alert users when their unit is no longer adequately treating their water. As an alternative, units may be equipped with an automatic shut-off mechanism to meet this requirement.

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 • If the American National Standards Institute issued product standards for a specific type of POU or POE treatment unit, only those units that have been independently certified according to those standards may be used as part of a compliance strategy.

The following observations with regard to using POE and POU devices for SDWA compliance were made by Raucher, *et al.* (2004):

- If POU devices are used as an SDWA compliance strategy, certain consumer behavioral changes will be necessary (e.g., encouraging people to drink water only from certain treated taps) to ensure comprehensive consumer health protection.
- Although not explicitly prohibited in the SDWA, USEPA indicates that POU treatment devices should not be used to treat for radon or for most volatile organic contaminants to achieve compliance, because POU devices do not provide 100 percent protection against inhalation or contact exposure to those contaminants at untreated taps (e.g., showerheads).
- Liability PWSs considering unconventional treatment options (POU, POE, or bottled water) must address liability issues. These could be meeting drinking water standards, property entry and ensuing liabilities, and damage arising from improper installation or improper function of the POU and POE devices.

1.4.7 Water Delivery or Central Drinking Water Dispensers

Current USEPA regulations 40 Code of Federal Regulations (CFR) 141.101 prohibit the use of bottled water to achieve compliance with an MCL, except on a temporary basis. State regulations do not directly address the use of bottled water. Use of bottled water at a non-compliant PWS would be on a temporary basis. Every 3 years, the PWSs that employ interim measures are required to present the TCEQ with estimates of costs for piping compliant water to their systems. As long as the projected costs remain prohibitively high, the bottled water interim measure is extended. Until USEPA amends the noted regulation, the TCEQ is unable to accept water delivery or central drinking water dispensers as compliance solutions.

Central provision of compliant drinking water would consist of having one or more dispensers of compliant water where customers could come to fill containers with drinking water. The centralized water source could be from small to medium-sized treatment units or could be compliant water delivered to the central point by truck.

Water delivery is an interim measure for providing compliant water. As an interim measure for a small impacted population, providing delivered drinking water may be cost effective. If the susceptible population is large, the cost of water delivery would increase significantly.

 Water delivery programs require consumer participation to a varying degree. Ideally, consumers would have to do no more than they currently do for a piped-water delivery system. Least desirable are those systems that require maximum effort on

the part of the customer (*e.g.*, customer has to travel to get the water, transport the water, and physically handle the bottles).

SECTION 2 EVALUATION METHOD

2.1 DECISION TREE

The decision tree is a flow chart for conducting feasibility studies for a non-compliant PWS. The decision tree is shown in Figures 2.1 through 2.4. The tree guides the user through a series of phases in the design process. Figure 2.1 shows Tree 1, which outlines the process for defining the existing system parameters, followed by optimizing the existing treatment system operation. If optimizing the existing system does not correct the deficiency, the tree leads to six alternative preliminary branches for investigation. The groundwater branch leads through investigating existing wells to developing a new well field. The treatment alternatives address centralized and on-site treatment. The objective of this phase is to develop conceptual designs and cost estimates for the six types of alternatives. The work done for this report follows through Tree 1 and Tree 2, as well as a preliminary pass through Tree 4.

Tree 3, which begins at the conclusion of the work for this report, starts with a comparison of the conceptual designs, selecting the two or three alternatives that appear to be most promising, and eliminating those alternatives that are obviously infeasible. It is envisaged that a process similar to this would be used by the study PWS to refine the list of viable alternatives. The selected alternatives are then subjected to intensive investigation, and highlighted by an investigation into the socio-political aspects of implementation. Designs are further refined and compared, resulting in the selection of a preferred alternative. The steps for assessing the financial and economic aspects of the alternatives (one of the steps in Tree 3) are given in Tree 4 in Figure 2.4.

2.2 DATA SOURCES AND DATA COLLECTION

2.2.1 Data Search

2.2.1.1 Water Supply Systems

The TCEQ maintains a set of files on public water systems, utilities, and districts at its headquarters in Austin, Texas. The files are organized under two identifiers: a PWS identification number and a CCN number. The PWS identification number is used to retrieve four types of files:

- CO Correspondence,
- CA Chemical analysis,
- MOR Monthly operating reports (quality/quantity), and
- FMT Financial, managerial and technical issues.

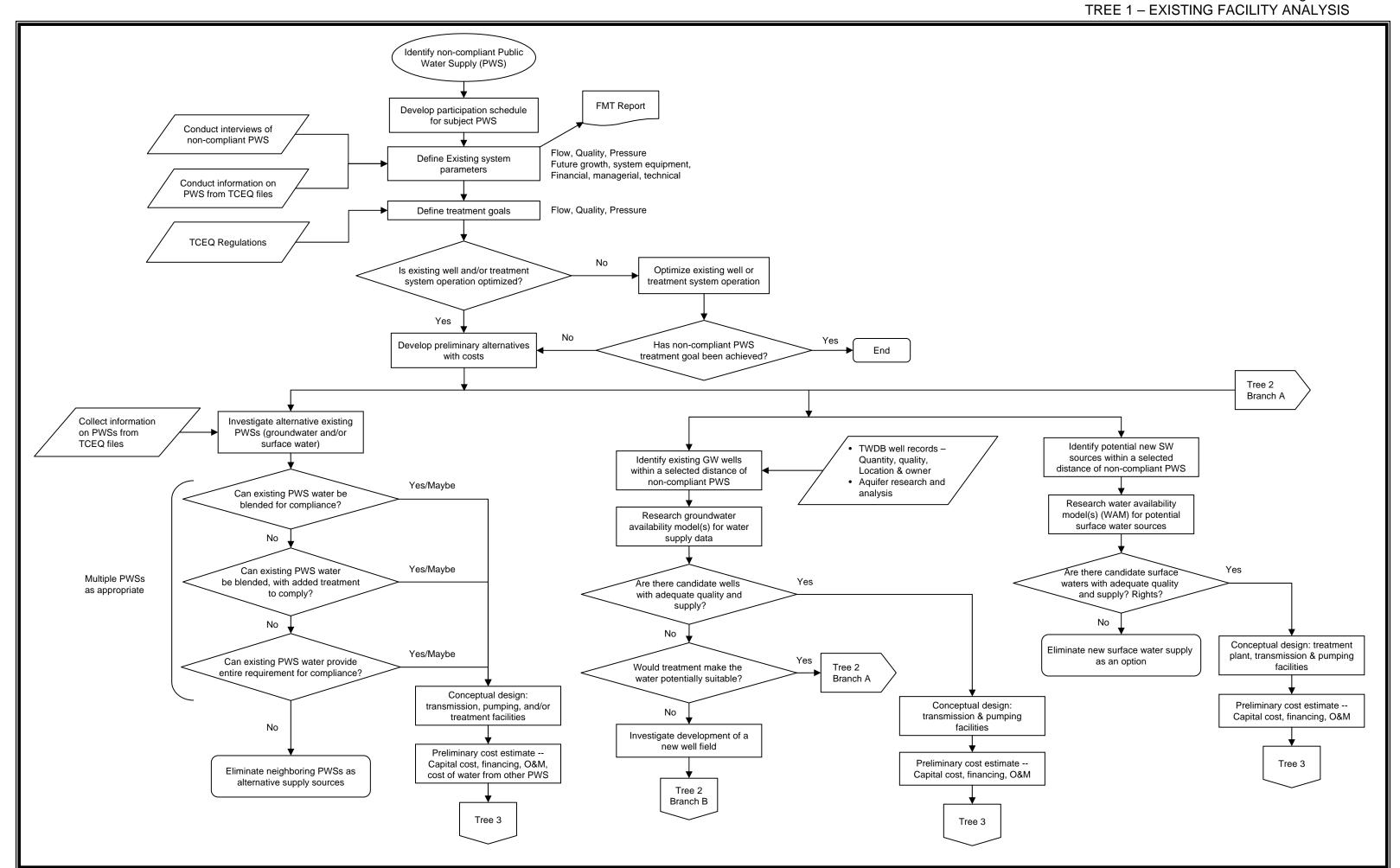
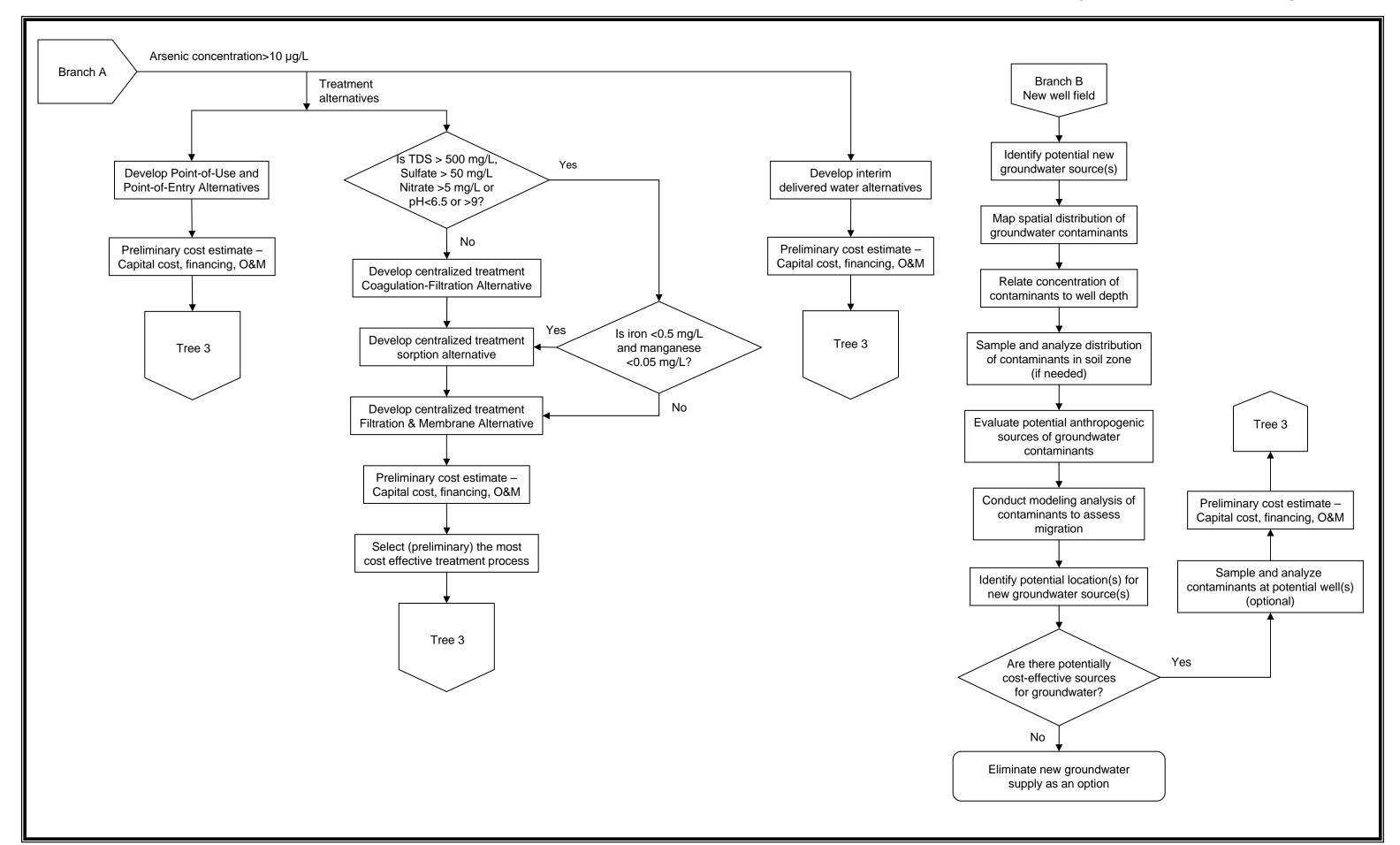


Figure 2.2 TREE 2 – DEVELOP TREATMENT ALTERNATIVES



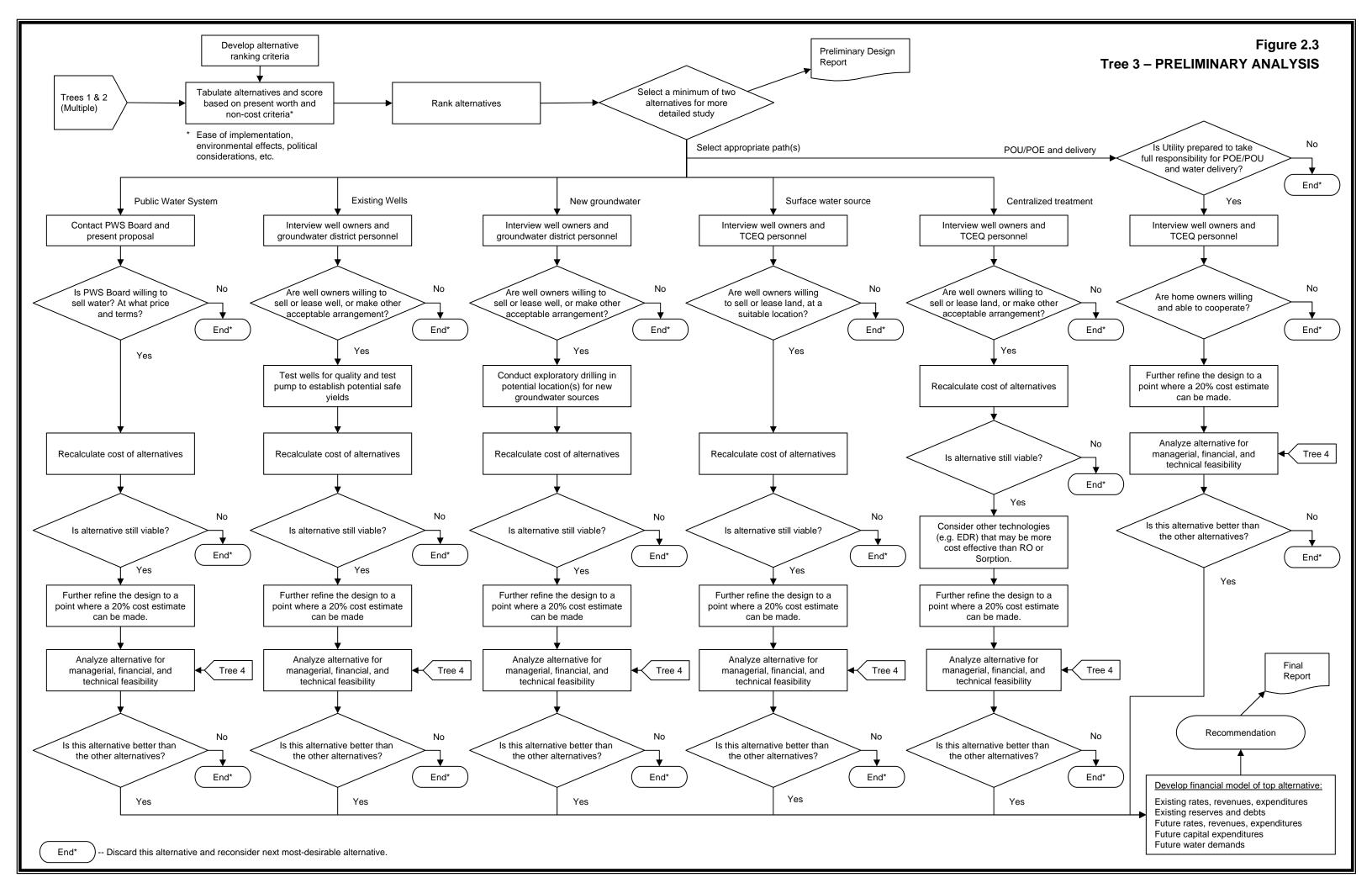
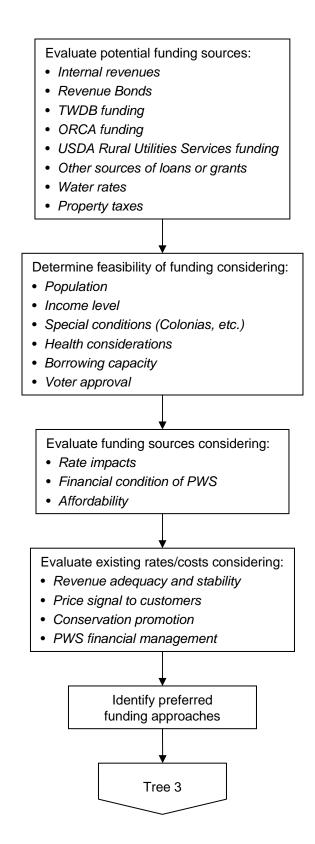


Figure 2.4 TREE 4 – FINANCIAL



- The CCN files generally contain a copy of the system's Certificate of Convenience and Necessity, along with maps and other technical data.
- These files were reviewed for the PWS and surrounding systems.
- 4 The following websites were consulted to identify the water supply systems in the area:
 - Texas Commission on Environmental Quality www3.tceq.state.tx.us/iwud/.
 - USEPA Safe Drinking Water Information System <u>www.epa.gov/safewater/data/getdata.html</u>

Groundwater Control Districts were identified on the TWDB web site, which has a series of maps covering various groundwater and surface water subjects. One of those maps shows groundwater control districts in the State of Texas.

2.2.1.2 Existing Wells

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The TWDB maintains a groundwater database available at www.twdb.state.tx.us that has two tables with helpful information. The "Well Data Table" provides a physical description of the well, owner, location in terms of latitude and longitude, current use, and for some wells, items such as flow rate, and nature of the surrounding formation. The "Water Quality Table" provides information on the aquifer and the various chemical concentrations in the water.

18 2.2.1.3 Surface Water Sources

Regional planning documents were consulted for lists of surface water sources.

2.2.1.4 Groundwater Availability Model

GAMs are numerical computer models of the major and minor Texas aquifers developed by the TWDB to assess groundwater availability over a 50-year planning period, and the possible effects of various proposed water management strategies on the aquifer systems. The GAM for the Gulf Coast aquifer was investigated as a potential tool for identifying available and suitable groundwater resources for the PWS.

2.2.1.5 Water Availability Model

The WAM is a computer-based simulation predicting the amount of water that would be in a river or stream under a specified set of conditions. WAMs are used to determine whether water would be available for a newly requested water right or amendment. If water is available, these models estimate how often the applicant could count on water under various conditions (*e.g.*, whether water would be available only one month out of the year, half the year, or all year, and whether that water would be available in a repeat of the drought of record).

WAMs provide information that assist TCEQ staff in determining whether to recommend the granting or denial of an application.

2.2.1.6 Financial Data

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An evaluation of existing data will yield an up-to-date assessment of the financial condition of the water system. As part of a site visit, financial data were collected in various forms such as electronic files, hard copy documents, and focused interviews. Data sought included:

- 8 Annual Budget
 - Audited Financial Statements
- 10 o Balance Sheet
- o Income & Expense Statement
- o Cash Flow Statement
- o Debt Schedule
- Water Rate Structure
- Water Use Data
- o Production
- o Billing
- 18 Customer Counts

2.2.1.7 Demographic Data

- Basic demographic data were collected from the 2000 Census to establish incomes and eligibility for potential low cost funding for capital improvements. Median household income
- 22 (MHI) and number of families below poverty level were the primary data points of significance.
- 23 If available, MHI for the customers of the PWS should be used. In addition, unemployment
- data were collected from current U.S. Bureau of Labor Statistics. These data were collected for
- 25 the following levels: national, state, and county.

2.2.2 PWS Interviews

2.2.2.1 PWS Capacity Assessment Process

- Capacity assessment is the industry standard term for evaluation of a water system's FMT capacity to effectively deliver safe drinking water to its customers now and in the future at a
- reasonable cost, and to achieve, maintain and plan for compliance with applicable regulations.
- 31 The assessment process involves interviews with staff and management who have a
- responsibility in the operations and management of the system.

 Financial, managerial, and technical capacity are individual yet highly interrelated components of a system's capacity. A system cannot sustain capacity without maintaining adequate capability in all three components.

Financial capacity is a water system's ability to acquire and manage sufficient financial resources to allow the system to achieve and maintain compliance with SDWA regulations. Financial capacity refers to the financial resources of the water system, including but not limited to, revenue sufficiency, credit worthiness, and fiscal controls.

Managerial capacity is the ability of a water system to conduct its affairs so the system is able to achieve and maintain compliance with SDWA requirements. Managerial capacity refers to the management structure of the water system, including but not limited to, ownership accountability, staffing and organization, and effective relationships with customers and regulatory agencies.

Technical capacity is the physical and operational ability of a water system to achieve and maintain compliance with SDWA regulations. It refers to the physical infrastructure of the water system, including the adequacy of the source water, treatment, storage and distribution infrastructure. It also refers to the ability of system personnel to effectively operate and maintain the system and to otherwise implement essential technical knowledge.

Many aspects of water system operations involve more than one component of capacity. Infrastructure replacement or improvement, for example, requires financial resources, management planning and oversight, and technical knowledge. A deficiency in any one area could disrupt the entire operation. A system that is able to meet both its immediate and long-term challenges demonstrates that it has sufficient FMT capacity.

Assessment of FMT capacity of the PWS was based on an approach developed by the New Mexico Environmental Finance Center (NMEFC), which is consistent with the TCEQ FMT assessment process. This method was developed from work the NMEFC did while assisting USEPA Region 6 in developing and piloting groundwater comprehensive performance evaluations. The NMEFC developed a standard list of questions that could be asked of water system personnel. The list was then tailored slightly to have two sets of questions – one for managerial and financial personnel, and one for operations personnel (the questions are included in Appendix A). Each person with a role in the FMT capacity of the system was asked the applicable standard set of questions individually. The interviewees were not given the questions in advance and were not told the answers others provided. Also, most of the questions are open-ended type questions so they were not asked in a fashion to indicate what would be the "right" or "wrong" answer. The interviews lasted between 45 minutes to 75 minutes depending on the individual's role in the system and the length of the individual's answers.

In addition to the interview process, visual observations of the physical components of the system were made. A technical information form was created to capture this information. This form is also contained in Appendix A. This information was considered supplemental to the

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interviews because it served as a check on information provided in the interviews. For example, if an interviewee stated he or she had an excellent preventative maintenance schedule and the visit to the facility indicated a significant amount of deterioration (more than would be expected for the age of the facility) then the preventative maintenance program could be further investigated or the assessor could decide that the preventative maintenance program was inadequate.

Following interviews and observations of the facility, answers that all personnel provided were compared and contrasted to provide a clearer picture of the true operations at the water system. The intent was to go beyond simply asking the question, "Do you have a budget?" to actually finding out if the budget was developed and being used appropriately. For example, if a water system manager was asked the question, "Do you have a budget?" he or she may say, "yes" and the capacity assessor would be left with the impression that the system is doing well in this area. However, if several different people are asked about the budget in more detail, the assessor may find that although a budget is present, operations personnel do not have input into the budget, the budget is not used by the financial personnel, the budget is not updated regularly, or the budget is not used in setting or evaluating rates. With this approach, the inadequacy of the budget would be discovered and the capacity deficiency in this area would be noted.

Following the comparison of answers, the next step was to determine which items noted as a potential deficiency truly had a negative effect on the system's operations. If a system had what appeared to be a deficiency, but this deficiency was not creating a problem in terms of the operations or management of the system, it was not considered critical and may not have needed to be addressed as a high priority. As an example, the assessment may have revealed an insufficient number of staff members to operate the facility. However, it may also have been revealed that the system was able to work around that problem by receiving assistance from a neighboring system, so no severe problems resulted from the number of staff members. Although staffing may not be ideal, the system does not need to focus on this particular issue. The system needs to focus on items that are truly affecting operations. As an example of this type of deficiency, a system may lack a reserve account which can then lead the system to delay much-needed maintenance or repair on its storage tank. In this case, the system needs to address the reserve account issue so proper maintenance can be completed.

The intent was to develop a list of capacity deficiencies with the greatest impact on the system's overall capacity. Those were the most critical items to address through follow-up technical assistance or by the system itself.

2.2.2.2 Interview Process

PWS personnel were interviewed by the project team, and each was interviewed separately. Interview forms were completed during each interview.

2.3 ALTERNATIVE DEVELOPMENT AND ANALYSIS

The initial objective for developing alternatives to address compliance issues is to identify a comprehensive range of possible options that can be evaluated to determine the most promising for implementation. Once the possible alternatives are identified, they must be defined in sufficient detail so a conceptual cost estimate (capital and O&M costs) can be developed. These conceptual cost estimates are used to compare the affordability of compliance alternatives, and to give a preliminary indication of rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. The basis for the unit costs used for the compliance alternative cost estimates is summarized in Appendix B. Other non-economic factors for the alternatives, such as reliability and ease of implementation, are also addressed.

2.3.1 Existing PWS

 The neighboring PWSs were identified, and the extents of their systems were investigated. PWSs farther than 35 miles from the non-compliant PWSs were not considered because the length of the pipeline required would make the alternative cost prohibitive. The quality of water provided was also investigated. For neighboring PWSs with compliant water, options for water purchase and/or expansion of existing well fields were considered. The neighboring PWSs with non-compliant water were considered as possible partners in sharing the cost for obtaining compliant water either through treatment or developing an alternate source.

The neighboring PWSs were investigated to get an idea of the water sources in use and the quantity of water that might be available for sale. They were contacted to identify key locations in their systems where a connection might be made to obtain water, and to explore on a preliminary basis their willingness to partner or sell water. Then, the major system components that would be required to provide compliant water were identified. The major system components included treatment units, wells, storage tanks, pump stations, and pipelines.

Once the major components were identified, a preliminary design was developed to identify sizing requirements and routings. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.2 New Groundwater Source

It was not possible in the scope of this project to determine conclusively whether new wells could be installed to provide compliant drinking water. To evaluate potential new groundwater source alternatives, three test cases were developed based on distance from the PWS intake point. The test cases were based on distances of 10 miles, 5 miles, and 1 mile. It was assumed a pipeline would be required for all three test cases, and a storage tank and pump station would be required for the 10-mile and 5-mile alternatives. It was also assumed that new wells would be installed, and that their depths would be 300 feet deep or other existing drinking water wells in the area.

A preliminary design was developed to identify sizing requirements for the required system components. A capital cost estimate was then developed based on the preliminary design of the required system components. An annual O&M cost was also estimated to reflect the change (*i.e.*, from current expenditures) in O&M expenditures that would be needed if the alternative was implemented.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increase in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.3.3 New Surface Water Source

New surface water sources were also considered. Availability of adequate quality water from rivers and major reservoirs in the surrounding area were investigated. TCEQ WAMs were inspected, and the WAM was run, where appropriate.

2.3.4 Treatment

Treatment technologies considered potentially applicable to arsenic removal are IX, RO, EDR, iron-based adsorption, and coagulation/filtration. However, because of high TDS in the well water (985 mg/L), IX may not be economically feasible. RO and EDR have the advantage of reducing TDS. Adsorption and coagulation/filtration processes remove arsenic only without significantly affecting TDS. RO treatment is considered for central treatment alternatives, as well as POU and POE alternatives. Both RO and EDR treatments produce a liquid reject waste stream. As a result, more water needs to be pumped than that which is introduced into the distribution system. This disadvantage is somewhat offset by split treatment of the raw water wherein a fraction of the water is treated through the RO unit, and is then blended back to the raw source water. RO has an advantage over EDR in that, in some cases, RO will remove As(III) without pre-oxidation. Since the arsenic speciation is not known at this time [As(III) or As(IV)] EDR is not considered further. Adsorption and coagulation filtration treatments produce periodic backwash wastewater for disposal. For this analysis RO treatment and iron-based adsorption treatment are considered. The treatment units were sized based on flow rates,

and capital and annual O&M cost estimates were made based on the size of the treatment equipment required. Neighboring non-compliant PWSs were identified to look for opportunities where the costs and benefits of central treatment could be shared between systems.

Non-economic factors were also identified. Ease of implementation was considered, as well as the reliability for providing adequate quantities of compliant water. Additional factors were whether implementation of an alternative would require significant increases in the management or technical capability of the PWS, and whether the alternative had the potential for regionalization.

2.4 COST OF SERVICE AND FUNDING ANALYSIS

The primary purpose of the cost of service and funding analysis is to determine the financial impact of implementing compliance alternatives, primarily by examining the required rate increases, and also the fraction of household income that water bills represent. The current financial situation of the non-compliant PWS is also reviewed to determine what rate increases are necessary to achieve or maintain long-term financial viability.

2.4.1 Financial Feasibility

A key financial metric is the comparison of average annual household water bill for a PWS customer to the MHI for the area. MHI data from the 2000 Census are used, at the most detailed level available for the community. Typically, county level data are used for small rural water utilities due to small population sizes. Annual water bills are determined for existing, base conditions, including consideration of additional rate increases needed under current conditions. Annual water bills are also calculated after adding incremental capital and operating costs for each of the alternatives to determine feasibility under several potential funding sources. It has been suggested by agencies such as USEPA that federal and state programs consider several criteria to determine "disadvantaged communities" with one based on the typical residential water bill as a percentage of MHI.

Additionally, the use of standard ratios provides insight into the financial condition of any business. Three ratios are particularly significant for water utilities:

- Current Ratio = current assets (liquid assets that could be readily converted to cash) divided by current liabilities (accounts payable, accrued expenses, and other short-term financial obligations) provides insight into the ability to meet short-term payments. For a healthy utility, the value should be greater than 1.0.
- Debt to Net Worth Ratio = total debt (total amount of long-term debt) divided by net worth (total assets minus total liabilities) shows to what degree assets of the company have been funded through borrowing. A lower ratio indicates a healthier condition.
- Operating Ratio = total operating revenues divided by total operating expenses show the degree to which revenues cover ongoing expenses. The value is greater than 1.0 if the utility is covering its expenses.

2.4.2 Median Household Income

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The 2000 U.S. census is used as the basis for MHI. In addition to consideration of affordability, the annual MHI may also be an important factor for sources of funds for capital programs needed to resolve water quality issues. Many grant and loan programs are available to lower income rural areas, based on comparisons of local income to statewide incomes. In the 2000 Census, MHI for the State of Texas was \$39,927, compared to the U.S. level of \$41,994. The census broke down MHIs geographically by block group and ZIP code. The MHIs can vary significantly for the same location, depending on the geographic subdivision chosen. The MHI for each PWS was estimated by selecting the most appropriate value based on block group or ZIP code based on results of the site interview and a comparison with the surrounding area.

2.4.3 Annual Average Water Bill

The annual average household water bill was calculated for existing conditions and for future conditions incorporating the alternative solutions. Average residential consumption is estimated and applied to the existing rate structure to estimate the annual water bill. The estimates are generated from a long-term financial planning model that details annual revenue, expenditure, and cash reserve requirements over a 30-year period.

2.4.4 Financial Plan Development

The financial planning model uses available data to establish base conditions under which the system operates. The model includes, as available:

- Accounts and consumption data
- Water tariff structure
- Beginning available cash balance
- Sources of receipts:
- 24 o Customer billings
- o Membership fees
- o Capital Funding receipts from:
- 27 **❖** Grants
- 28 Proceeds from borrowing
- Operating expenditures:
- o Water purchases
- o Utilities
- 32 o Administrative costs
- o Salaries

- Capital expenditures
- Debt service:
- O Existing principal and interest payments
 - Future principal and interest necessary to fund viable operations
- Net cash flow

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- Restricted or desired cash balances:
 - Working capital reserve (based on 1-4 months of operating expenses)
 - Replacement reserves to provide funding for planned and unplanned repairs and replacements

From the model, changes in water rates are determined for existing conditions and for implementing the compliance alternatives.

2.4.5 Financial Plan Results

Results from the financial planning model are summarized in two areas: percentage of household income and total water rate increase necessary to implement the alternatives and maintain financial viability.

2.4.5.1 Funding Options

Results are summarized in a table that shows the following according to alternative and funding source:

- Percentage of the median annual household income the average annual residential water bill represents.
- The first year in which a water rate increase would be required
- The total increase in water rates required, compared to current rates

Water rates resulting from the incremental capital costs of the alternative solutions are examined under a number of funding options. The first alternative examined is always funding from existing reserves plus future rate increases. Several funding options were analyzed to frame a range of possible outcomes.

- Grant funds for 100 percent of required capital. In this case, the PWS is only responsible for the associated O&M costs.
- Grant funds for 75 percent of required capital, with the balance treated as if revenue bond funded.
- Grant funds for 50 percent of required capital, with the balance treated as if revenue bond funded.

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- State revolving fund loan at the most favorable available rates and terms applicable to the communities.
 - If local MHI > 75 percent of state MHI, standard terms, currently at 3.8 percent interest for non-rated entities. Additionally:
 - o If local MHI = 70-75 percent of state MHI, 1 percent interest rate on loan.
 - o If local MHI = 60-70 percent of state MHI, 0 percent interest rate on loan.
 - o If local MHI = 50-60 percent of state MHI, 0 percent interest and 15 percent forgiveness of principal.
 - o If local MHI less than 50 percent of state MHI, 0 percent interest and 35 percent forgiveness of principal.
 - Terms of revenue bonds assumed to be 25-year term at 6.0 percent interest rate.

2.4.5.2 General Assumptions Embodied in Financial Plan Results

- The basis used to project future financial performance for the financial plan model includes:
- No account growth (either positive or negative).
 - No change in estimate of uncollectible revenues over time.
- Average consumption per account unchanged over time.
 - No change in unaccounted for water as percentage of total (more efficient water use would lower total water requirements and costs).
 - No inflation included in the analyses (although the model has provisions to add escalation of O&M costs, doing so would mix water rate impacts from inflation with the impacts from the alternatives being examined).
 - Minimum working capital fund established for each district, based on specified months of O&M expenditures.
 - O&M for alternatives begins 1 year after capital implementation.
 - Balance of capital expenditures not funded from primary grant program is funded through debt (bond equivalent).
- Cash balance drives rate increases, unless provision chosen to override where current net cash flow is positive.

2.4.5.3 Interpretation of Financial Plan Results

Results from the financial plan model are presented in a Table 4.4, which shows the percentage of MHI represented by the annual water bill that results from any rate increases necessary to maintain financial viability over time. In some cases, this may require rate increases even without implementing a compliance alternative (the no action alternative). The

table shows any increases such as these separately. The results table shows the total increase in rates necessary, including both the no-action alternative increase and any increase required for the alternative. For example, if the no action alternative requires a 10 percent increase in rates and the results table shows a rate increase of 25 percent, then the impact from the alternative is an increase in water rates of 15 percent. Likewise, the percentage of household income in the table reflects the total impact from all rate increases.

2.4.5.4 Potential Funding Sources

A number of potential funding sources exist for water supply corporations, which typically provide service to less than 50,000 people. Both state and federal agencies offer grant and loan programs to assist rural communities in meeting their infrastructure needs. Most are available to "political subdivisions" such as counties, municipalities, school districts, special districts, or authorities of the state with some programs providing access to private individuals. Grant funds and lower interest rates are made more available with demonstration of economic stress, typically indicated with MHI below 80 percent that of the state. The funds may be used for planning, design, and construction of water supply construction projects including, but not limited to, line extensions, elevated storage, purchase of well fields, and purchase or lease of rights to produce groundwater. Interim financing of water projects and water quality enhancement projects such as wastewater collection and treatment projects are also eligible. Some funds are used to enable a rural water utility to obtain water or wastewater service supplied by a larger utility or to finance the consolidation or regionalization of neighboring utilities. Three Texas agencies that offer financial assistance for water infrastructure are:

- Texas Water Development Board has several programs that offer loans at interest rates lower than the market offers to finance projects for public drinking water systems that facilitate compliance with primary drinking water regulations. Additional subsidies may be available for disadvantaged communities. Low interest rate loans with short and long-term finance options at tax exempt rates for water or water-related projects give an added benefit by making construction purchases qualify for a sales tax exemption. Generally, the program targets customers with eligible water supply projects for all political subdivisions of the state (at tax exempt rates) and Water Supply Corporations (at taxable rates) with projects.
- Texas Department of Rural Affairs (TDRA) is a Texas state agency with a focus on rural Texas by making state and federal resources accessible to rural communities. Funds from the U.S. Department of Housing and Urban Development Community Development Block Grants (CDBG) are administered by TDRA for small, rural communities with populations less than 50,000 that cannot directly receive federal grants. These communities are known as non-entitlement areas. One of the program objectives is to meet a need having a particular urgency, which represents an immediate threat to the health and safety of residents, principally for low- and moderate-income persons.
- U.S. Department of Agriculture Rural Development Texas (Texas Rural Development) coordinates federal assistance to rural Texas to help rural Americans improve their

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quality of life. The Rural Utilities Service (RUS) programs provide funding for water and wastewater disposal systems.

The application process, eligibility requirements, and funding structure vary for each of these programs. There are many conditions that must be considered by each agency to determine eligibility and ranking of projects. The principal factors that affect this choice are population, percent of the population under the state MHI, health concerns, compliance with standards, Colonia status, and compatibility with regional and state plans. Technical assistance is available to assist local entities with the preparation of funding request applications from each agency.

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SECTION 3 UNDERSTANDING SOURCES OF CONTAMINANTS

3.1 OVERVIEW OF THE STUDY AREA

The regional study area is defined by seven Texas counties, including La Salle, McMullen, Webb, Duval, Zapata, Jim Hoggs, and Brooks (Figure 3.1) and is located in the Texas Southern Gulf Coast. Duval, Jim Hoggs, and Brooks Counties are located over the Gulf Coast aquifer; McMullen and Webb Counties are located above the Gulf Coast, Yegua Jackson, and Carrizo Wilcox aquifers; Zapata is located Yegua Jackson aquifer; and La Salle is located over the Carrizo Wilcox and Yegua Jackson aquifers. The Mirando PWS operates two wells completed in the Gulf Coast aquifer.

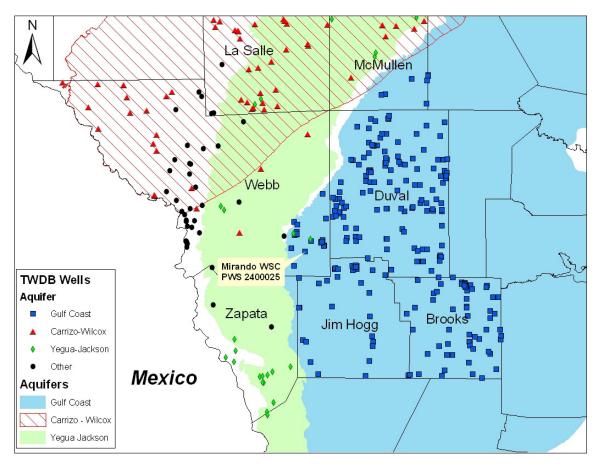


Figure 3.1 Regional Study Area, Major and Minor Aquifers, Groundwater Well Locations, and Location of the Mirando City PWS.

Aquifers in the study area include the Gulf Coast, Carrizo Wilcox, and Yegua Jackson aquifers. The Gulf Coast and Carrizo Wilcox are classified as major aquifers by the State of Texas. There are 261 wells in the study area completed in the Gulf Coast aquifer that have

- water quality analyses in the Texas Water Development Board (TWDB) database; and 58
- 2 Carrizo Wilcox wells. The Yegua Jackson aquifer is classified as a minor aquifer by the State
- 3 of Texas. There are only 26 wells in the study area that are completed in the Yegua Jackson
- 4 aquifer that have water quality analyses in the TWDB database.
- 5 Data used for this study include information come from two sources:
 - TWDB groundwater database available at www.twdb.state.tx.us. The database includes information on the location and construction of wells throughout the state as well as historical measurements of water chemistry and levels in the wells.
 - Texas Commission on Environmental Quality (TCEQ) Public Water Supply database (not publicly available). The database includes information on the location, type, and construction of water sources used by public water systems in Texas, along with historical measurements of water levels and chemistry.

13 3.2 CONTAMINANTS OF CONCERN IN THE STUDY AREA

The primary contaminant of concern in the region is arsenic. The maximum contaminant level (MCL) concentration allowed in the drinking water of a public water supply system by the U.S. Environmental Protection Agency is $10 \mu g/L$ (ppb).

Arsenic

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- Arsenic concentrations exceed the MCL (10 μ g/L) in 35 percent of the wells throughout the study area, with generally higher concentrations in the Gulf Coast aquifer (Figure 3.2).
- 20 Approximately 51 percent of Gulf Coast aguifer wells in the study area have arsenic
- 21 concentrations above the MCL (Table 3.1). Only three wells from other aguifers (two in the
- Yegua Jackson, and one in an undefined aquifer) exceed the MCL.

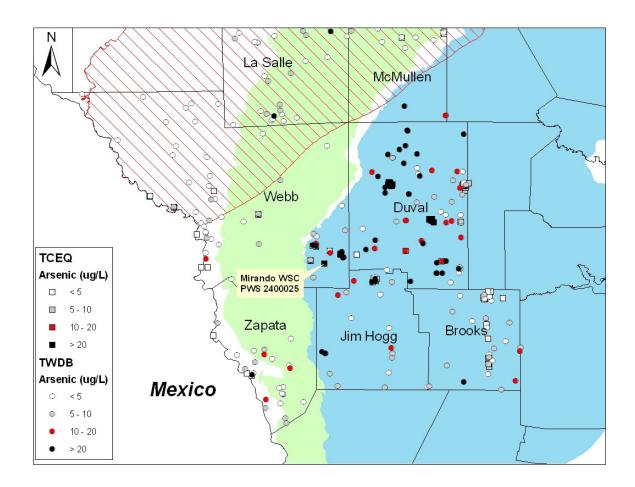


Figure 3.2 Spatial Distribution of Arsenic Concentrations in the Study Area.

Data represent the latest sample for wells in the TWDB and TCEQ databases. Data for TWDB wells represent values for single wells. For TCEQ wells, locations shown represent the spatial average for all wells associated with a PWS system entry point and concentrations may represent blended water from multiple wells and/or treated water.

Table 3.1 Summary of Arsenic Concentrations in Groundwater Well Samples based on the Most Recent Sample Data from the TWDB Database.

| Aquifer | Wells with Measurements | Median (μg/L) | Range (µg/L) | Wells that exceed MCL | % of wells that exceed MCL |
|----------------|----------------------------|------------------|-----------------|-----------------------------|----------------------------------|
| Gulf Coast | 146 | 10.45 | 1.5-160.0 | 74 | 51 |
| Carrizo Wilcox | 41 | 2.00 | 1.0-10.0 | 0 | 0 |
| Yegua Jackson | 14 | 5.05 | 2.0-28.5 | 2 | 14 |
| Other | 25 | 2.00 | 2.0-16.0 | 1 | 4 |

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When examining the relation of arsenic concentrations with depth, no association is found for the Gulf Coast or Carrizo-Wilcox aquifers (Figure 3.3). Yegua-Jackson and wells described as "other" exhibit an association of arsenic concentrations with depth, decreasing to below the MCL at approximately 1,000 ft.

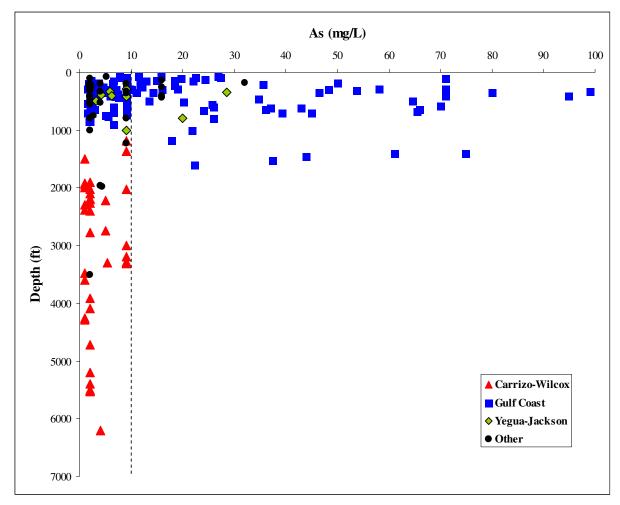


Figure 3.3 Arsenic Concentrations versus Well Depth.

Total Dissolved Solids

Total dissolved solids (TDS) concentrations exceed the secondary MCL (1000 mg/L) in 60 percent of the wells throughout the study area (Figure 3.4). High TDS values are found in all aquifers in the region (Table 3.2).

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Data represent the latest sample for wells in the TWDB and TCEQ databases. Data for TWDB wells represent values for single wells. For TCEQ wells, locations shown represent the spatial average for all wells associated with a PWS system entry point and concentrations may represent blended water from multiple wells and/or treated water.

Table 3.2 Summary of TDS Concentrations in Groundwater Well Samples Based on the most Recent Sample Data from the TWDB Database.

| Aquifer | Wells with measurements | Median (mg/L) | Range (mg/L) | Wells that Exceed SMCL | % of Wells that Exceed SMCL |
|----------------|-------------------------|------------------|-----------------|------------------------------|-----------------------------------|
| Gulf Coast | 182 | 1078 | 454 - 8062 | 98 | 54 |
| Carrizo Wilcox | 64 | 830 | 403 - 9707 | 27 | 42 |
| Yegua Jackson | 17 | 2588 | 924 - 11256 | 16 | 94 |
| Other | 61 | 2104 | 704 - 11710 | 54 | 89 |

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TDS concentrations are associated with depth, but generally exceed the SMCL in the region (Figure 3.5).

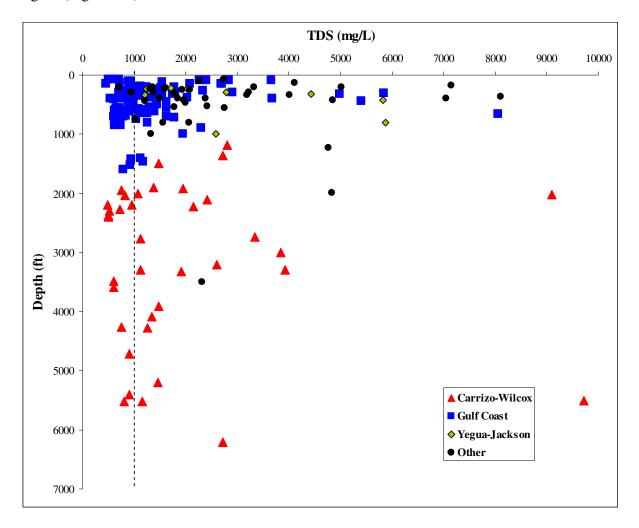


Figure 3.5 TDS Concentrations versus Well Depth.

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3.3 REGIONAL GEOLOGY

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Webb County represents a large area (8,743 km²) with geology similar to the rest of the Gulf Coast. The Gulf Coast aquifer system consists of several progradation wedges of Tertiary age consisting of alternating sandstone and claystone corresponding to variations in sea level and in inland sediment input as well as other factors. Those wedges are approximately parallel to the current shoreline, and the deposition process is still active today (e.g., Mississippi River). Sediment layers are progressively younger from northwest to southeast through Webb County. Formations cropping out in the vicinity of the Dimmit County line have a regional dip of a couple of degrees and can be followed to a depth of approximately 8,000 feet at the Zapata – Jim Hogg County line. Similar to other Gulf Coast counties, the oldest formations covering the mostly carbonate Cretaceous platform are predominantly clayey formations of the Midway Group. They crop out in Dimmit County and form the low-permeability base of the Tertiary Gulf Coast aquifer system. The subsequent sediment input cycles have been grouped into the following hydrostratigraphic units, approximately valid from the Mexican border to the Louisiana State line. They are, starting with the oldest: (1) the Carrizo - Wilcox aquifer (Eocene), (2) the Queen City - Sparta aquifer (Eocene), (3) the Yegua - Jackson aquifer (Eocene), and (4) the Gulf Coast aquifers sensu stricto (Miocene to recent) present in the coastal plains. Each of these units is separated by clayey aquitards and may also contain layers of lower permeability.

The Carrizo-Wilcox and Gulf Coast aquifers are recognized as major aquifers in the State of Texas (Ashworth and Hopkins 1995). In the Tertiary Gulf Coast system, the general flow system consists of water infiltrating in the outcrop areas of the more permeable formations, some of it discharging into rivers and springs along short flow paths, and some of it flowing downdip into the deeper sections of the aquifers. The fate of that slowly moving water is to slowly percolate up by cross-formational flow and discharge into the ocean. This step is necessary to maintain the mass balance of the regional flow system although, because of sometimes heavy pumping, the natural upward flow has been locally reversed. The Carrizo-Wilcox aquifer is mainly present in the northern half of Webb County and may provide a significant amount of water, although little pumping currently occurs in this region (*e.g.*, Deeds, *et al.* 2003). The aquifer consists of fluvio-deltaic sediments of the Wilcox Group and of the Carrizo Sand. The Carrizo Sand produces high quality water, especially in the extreme northwest of the county.

The Queen City Formation is separated from the Carrizo Sand by a marine clay: the Reclaw formation and from the overlying Sparta Sands by another marine clay: the Weches formation. Queen City and Sparta formations are often grouped together and are of fluvio-deltaic origin. The Queen City – Sparta aquifers do not strictly exist south of the Frio River, but equivalent water-bearing formations have been recognized (Bigford and Laredo formations) (Kelley, *et al.* 2004). In Webb County, wells drilled in the formations equivalent to the Queen City – Sparta Formations yield slightly saline water.

The Yegua Formation overlies another shaly layer (the Cook Mountain formation) separating it from the Sparta Sand and equivalent formations. The Yegua formation and

Jackson Group yield relatively saline water in Webb County, except perhaps in the outcrop and shallow downdip areas (Mace, *et al.* 2006). The top formation of the Jackson Group, the Frio Clay, and the Catahoula formation, form the regional confining unit between the Yegua – Jackson aquifer and the Gulf Coast aquifers in south Texas. Both are of Oligocene age, and the latter consists mainly of volcanic tuffs and their derived sediments. The Frio Clay is not to be confused with the Frio formation or Frio Sandstones of the deep subsurface that host multiple oil and gas reservoirs. Locally, the permeability of the Catahoula tuffs and sediments is high enough to qualify as an aquifer. A recognized geologic source of arsenic in groundwater is volcanic ash such as the tuffs of the Catahoula formation (Scanlon, *et al.* 2005). Arsenic is often associated with other chemical elements, such as fluoride, vanadium, molybdenum, selenium, and uranium. The association is, in general, seen at the subregional level, although not necessarily at the well level because of different geochemical behavior of individual elements.

Only the base of the Gulf Coast aquifers is present in the extreme southeast of Webb County (Bruni area). The sediments are mainly of fluvio-deltaic or shallow marine in origin. In most of south Texas, the base of the Gulf Coast aquifers consists of the Jasper aquifer, approximately equivalent to the Oakville Sandstones of Miocene age and of the Evangeline aquifer corresponding to the Goliad Sand of Pliocene age. A complex lower permeability system, called the Burkeville Confining System, which primarily includes the upper part of the Fleming formation, separates the Jasper and Evangeline aquifers. Although the Oakville Sandstone is thick in the middle sections of the Gulf Coast, it pinches out or has been eroded toward the Mexico border. This generates confusion in the definition of the Jasper aquifer in Webb County and surrounding counties. This hydrostratigraphic unit could also include Catahoula sandstone (e.g., Chowdhury and Mace 2003), Oakville sandstone, and permeable formations of the Fleming formation or could be altogether lumped into the Evangeline aquifer (Adidas 1991). The PWS well of concern in Webb County is identified by the TWBD as being in the Jasper aquifer (540 feet). The younger Chicot aquifer, uppermost subunit of the Gulf Coast aquifers and composed of Pleistocene sediment, is not present in Webb County.

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3.4 DETAILED ASSESSMENT

Mirando City (PWS 2400025)

The Mirando PWS has two wells, G2400025A (540 ft deep) and G2400025B (500 ft deep). Well A was tested for compliance in November 2008 (Table 3.3). The analytic data revealed an arsenic concentration that exceeded the MCL (10 μ g/L), and the TDS concentration exceeded the secondary MCL (1000 mg/L). The well was not tested for combined uranium. On April 30, 2010, TCEQ approved Well B for use; however, analytical data for Well B is not available in the TCEQ PWS database.

Table 3.3 Arsenic, Gross Alpha, and TDS Concentrations in Mirando PWS Entry Point Samples (2008 data from the TCEQ PWS database; 2009 data from SDWIS database).

| Sample | As | Gross Alpha | TDS | |
|----------------------|------------|-------------|--------|--|
| | μg/L | pCi/L | mg/L | |
| MCL (SMCL) | 10 | 15 | (1000) | |
| 11/25/08 | 23.8 | 4.6 | 985 | |
| 01/01/09 to 03/31/09 | 11.5 (avg) | | | |
| 04/01/09 to 06/30/09 | 15.9 (avg) | - | - | |

There are three PWS system located within 10 km of Mirando PWS (Table 3.4, Figures 3.6 through 3.8). There are nine groundwater wells listed in the TWDB data base that are within 10 km of Mirando PWS that have been analyzed for one or more of the contaminants of concern (Table 3-4, Figures 3.6 through 3.8). Six wells are TCEQ PWS wells, and three are privately owned. The TCEQ database lists blend values measured at the entry point of the PWS, whereas TWDB data are single well analysis; therefore, all TWDB wells are discussed here, including TCEQ wells listed in the TWDB database.

Arsenic concentrations for TWDB wells located within 10 km of Mirando PWS range from 2 to 100 μ g/L (median 12 μ g/L). Four wells out of nine (44%) were compliant with the arsenic MCL (10 μ g/L) (Figure 3.6). TDS concentrations range from 532 to 1240 mg/L (median 833 mg/L) and none of the wells were compliant with the secondary TDS MCL (1,000 mg/L) (Figure 3.7). Only three TWDB wells were tested for gross alpha, and all were found to be noncompliant with the MCL (15 pCi/L), with a range of 19 to 25 pCi/L (median 20 pCi/L). Combined uranium concentrations for TWDB wells located within 10 km of Mirando PWS range from 10 to 61 μ g/L (median 24 μ g/L). Three of these wells (75%) were compliant with the total uranium MCL (30 μ g/L) (Figure 3.8).

Table 3.4 Arsenic, Gross Alpha, Total Uranium, and TDS Concentrations in Potential Alternative Groundwater Sources within 10 km of Mirando PWS

Non-compliant values for MCLs are highlighted.

| PWD ID / | System / Oyyman | Sample | As | α | U | TDS | |
|---------------|-----------------------|------------|------|-------|------|------|--|
| Well ID | System / Owner | date | μg/L | pCi/L | μg/L | mg/L | |
| TCEQ Database | | | | | | | |
| 2400025 | Mirando PWS * | 11/25/2008 | 23.8 | 4.6 | - | 985 | |
| 2400017 | Los Ojuelos Water Co. | 10/25/2006 | 7.89 | 3.6 | - | 984 | |
| 2400006 | Oilton Rural WSC | 5/17/2006 | 2.91 | 2.0 | - | 965 | |
| 2400003 | City of Bruni | 2/14/2006 | 81.0 | 19.5 | 23.5 | 798 | |
| TWDB Database | | | | | | | |
| 8433204 | Oilton Rural WSC | 4/3/2001 | 2 | - | - | 532 | |
| 8433601 | Robert Marshall | 5/15/1990 | 12 | 1 | 10 | 833 | |
| 8433603 | Oilton Rural WSC | 5/24/2005 | 4.2 | 1 | - | 1104 | |
| 8433701 | Los Ojuelos Water Co. | 4/3/2001 | 6.4 | 20 | - | 1240 | |
| 8434401 | City of Bruni | 3/15/2005 | 95 | - | - | 788 | |
| 8434403 | City of Bruni | 3/25/1986 | 71 | 25 | 25 | 771 | |
| 8434404 | City of Bruni | 4/20/1989 | 100 | 19 | 22 | 832 | |
| 8434413 | Jose A. Solano | 5/16/1990 | 71 | - | 61 | 860 | |
| 8441401 | Violeta Ranch | 5/7/1997 | 9.2 | - | - | 1069 | |
| Median | | 1997 | 12 | 20 | 24 | 833 | |

^{* -} SDWIS reports arsenic results ranging from 0.0115 to 0.015975 mg/l from 01/01/2009 to 06/30/2009

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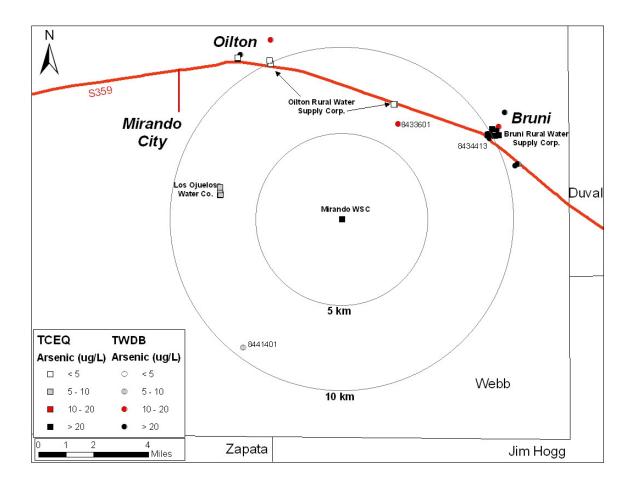


Figure 3.6 Arsenic Concentrations in Groundwater near Mirando PWS

Sample data shown represent the most recent sample. Data in the TCEQ PWS database may represent entry point samples that combine water from multiple wells and may also reflect post-treatment concentrations. Samples from the TWDB database represent samples from single wells and represent raw water concentrations.

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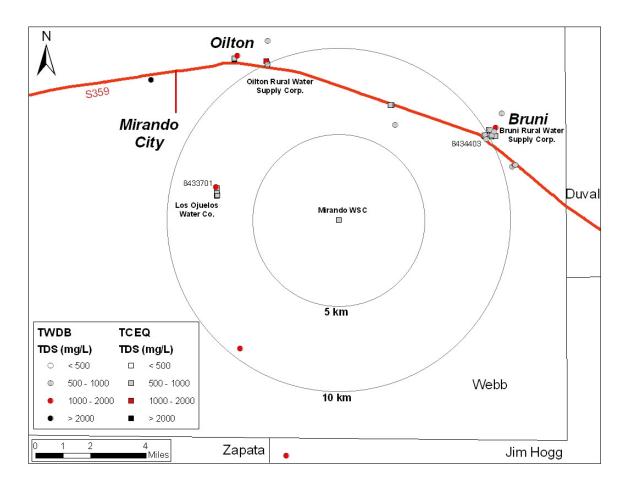


Figure 3.7 Total Dissolved Solids Concentrations in Groundwater near Mirando PWS

Sample data shown represent the most recent sample. Data in the TCEQ PWS database may represent entry point samples that combine water from multiple wells and may also reflect post-treatment concentrations. Samples from the TWDB database represent samples from single wells and represent raw water concentrations.

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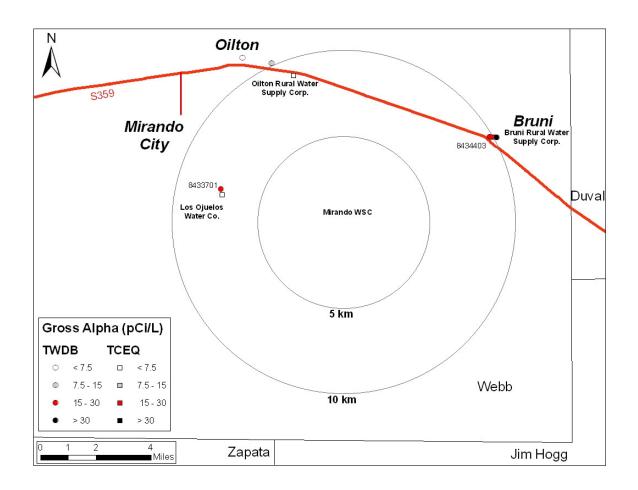


Figure 3.8 Gross Alpha Concentrations in Groundwater near Mirando PWS.

Sample data shown represent the most recent sample. Data in the TCEQ PWS database may represent entry point samples that combine water from multiple wells and may also reflect post-treatment concentrations. Samples from the TWDB database represent samples from single wells and represent raw water concentrations.

3.5 SUMMARY OF ALTERNATIVE GROUNDWATER SOURCES FOR MIRANDO PWS

There are several alternative groundwater sources within 10 km of Mirando PWS. Unfortunately, lack of data (combined uranium and gross alpha measurements) for many of these sites prevents us from identifying a suitable alternative source. Los Ojuelos Water Co. and Oilton Rural PWS show compliant values for arsenic and gross alpha, but lack combined uranium values. Violeta Ranch is compliant with arsenic values, but lacks data for combined uranium and gross alpha values. More tests are required in this region to assess suitable alternative groundwater sources.

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SECTION 4 2 ANALYSIS OF THE MIRANDO CITY PWS

4.1 DESCRIPTION OF EXISTING SYSTEM

4.1.1 Existing System

The location of the Mirando City PWS is shown in Figure 4.1. Mirando City is a community water system serving a population of 500 with 250 active connections. The PWS is located approximately 12 miles west of Bruni, Texas and 34 miles east of Laredo, Texas.

The Mirando City PWS previously purchased water from Los Ojuelos Water Company until August 31, 2008 and now obtains its water from two groundwater wells, Well #1 (G2400025A) and Well #2 (G2400025B). Well #1, completed to a depth of 540 feet, is rated at 60 gallons per minute (gpm) and Well #2, completed to a depth of 500 feet, is rated at 58 gpm. The Mirando City PWS is a regional supplier of water to Los Ojuelos, Los Veteranos, and Augilares. Groundwater from the wells, located approximately 2 miles southeast from Mirando City, is pumped into a 6 inch transmission line to a standpipe (capacity 0.193 mg). The size of the discharge line to Los Ojuelos, Los Veteranos, and Augilares is 4-inches.

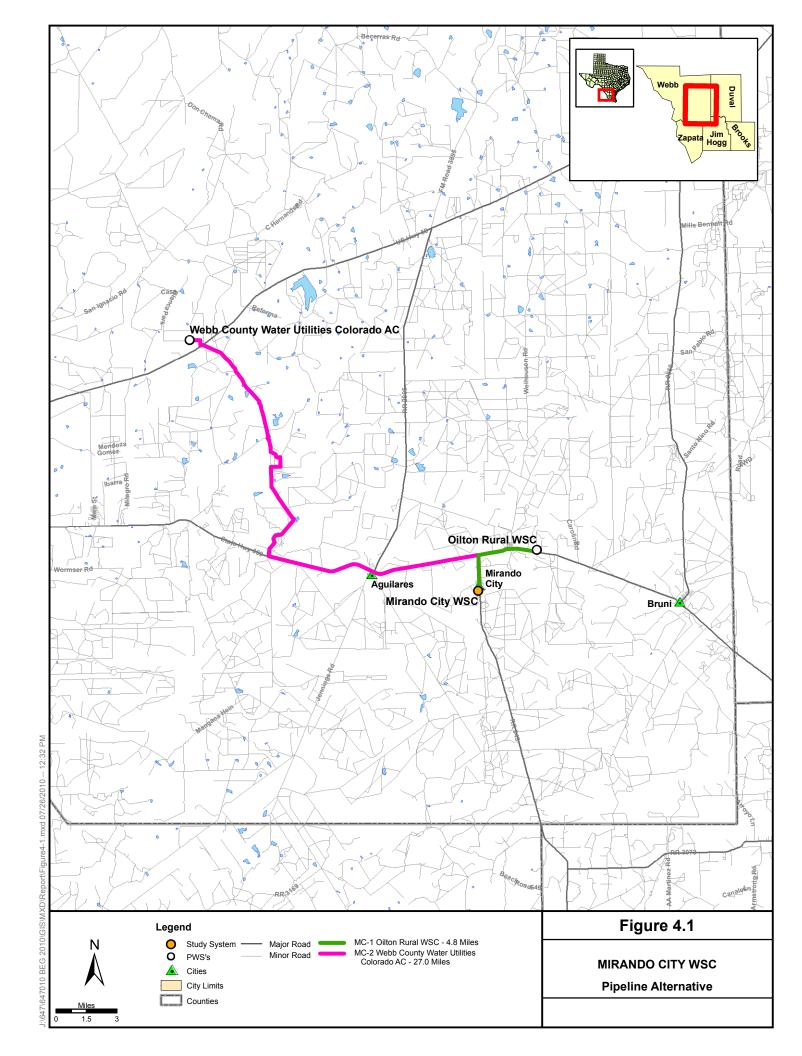
Previous TCEQ inspection reports noted the system service connections are located at the same elevation as the base of the storage tank so only the top 10 feet of tank (approximate capacity 0.025 mg) can be counted as elevated storage even though the standpipe is approximately 90 feet high. The wells, both of which are completed in the Jasper formation of the Gulf Coast aquifer, are located in Webb County. A third well, completed to a depth of 300 feet, is used for non-potable purposes and is rated at 35 to 40 gpm. Sample results from Well #3 indicate that the concentration of arsenic is less than the MCL. The total production of the two wells is 0.187 mgd. Disinfection (gas chlorination) is performed on the well discharge line before water is pumped into the distribution system.

In November 2008 arsenic was detected at 0.0238 mg/L. During January 2009 to June 2009, arsenic was detected at levels between 0.0115 mg/L to 0.0159 mg/L. The average concentration exceeds the MCL of 0.010 mg/. The Mirando City PWS has not encountered any other water quality issues except for having marginally high total dissolved solids (TDS). Typical TDS concentrations average around 985 mg/L.

The treatment employed for disinfection is not appropriate or effective for removal of arsenic, so optimization is not expected to be effective in increasing removal of this contaminant. There is, however, a potential opportunity for system optimization to reduce arsenic concentration. There are lower arsenic concentrations (although not less than $10 \,\mu\text{g/L}$) in water bearing units at depths to 300 feet. The existing wells could be rescreened to block groundwater from water-bearing units below 300 feet. The PWS could also drill an additional well in the sand units to 300 feet deep, to blend water with the existing wells. It is possible that

- 1 re-screening the existing wells and drilling a new well would produce water with an arsenic
- 2 concentration below 10 μg/L.
- 3 Basic system information is as follows:
- Population served: 500
- Connections: 250
- Average daily flow: 0.080 mgd
- 7 Total production capacity: 0.187 mgd
- Typical arsenic range: 0.0115 mg/L to 0.0238 mg/L
- Typical total dissolved solids range: 985 mg/L (one sample result)
- Typical combined radium range: 1.5 pCi/L
- Typical selenium range: 0.004 mg/L
- Typical sulfate range: 141 mg/L
- Typical nitrate range: 2.94 mg/L
- Typical bicarbonate (CaCO₃) range: 227 mg/L
- Typical fluoride range: 0.94 mg/L
- Typical iron range: 0.207 mg/L

- Typical manganese range: 0.0058 mg/L
- The typical ranges for water quality data listed above are based on a TCEQ database that contains data updated through the beginning of 2010.



4.1.2 Capacity Assessment for the Mirando City PWS

The project team conducted a capacity assessment of the Mirando City PWS on July 29, 2010. The results of this evaluation are separated into four categories: general assessment of capacity, positive aspects of capacity, capacity deficiencies, and capacity concerns. The general assessment of capacity describes the overall impression of the technical, managerial, and financial capability of the water system. The positive aspects of capacity describe the strengths of the system. These factors can provide the building blocks for the system to improve capacity deficiencies. The capacity deficiencies noted are those aspects that are creating a particular problem for the system related to long-term sustainability. Primarily, these problems are related to the system's ability to meet current or future compliance, ensure proper revenue to pay the expenses of running the system, and to ensure the proper operation of the system. The last category, capacity concerns, includes items that are not causing significant problems for the system at this time. However, the system may want to address them before they become problematic.

- To complete this analysis, the project team interviewed the following people:
- Lorenzo Mata, General Manager (GM) and Operator
 - Patricia Mata, Office Manager
- Judy Perez, Office Assistant

4.1.2.1 General Structure of the Water System

Mirando City PWS is governed by a 5-member Board of Directors. The system provides water and is in the process of constructing a wastewater treatment lagoon system and will eventually discontinue use of all septic tanks. The wastewater project is being funded by USDA and the county.

The water system includes two wells and one standpipe. The system has a long lease on 1,000 acres of land about 12 miles outside of the city. The system pays the landowner for water produced each month, as well as for the costs of drilling the wells. The system has 250 service connections including the nearby communities of Aguilares, Ranchitos Los Veteranos, and Ojuelos.

The current GM/operator was hired in August 2009, but because the former operator was still employed, the new manager did not obtain full control of the system until about January 2010. He has taken the TCEQ Operator Certification test to obtain a Class D license, and the results are pending. In the meantime, he is being supervised by two certified operators in the neighboring town of Oilton. After obtaining his Class D license, and two years of experience, he will test for a Class C license. The system also employs an office manager, who has been there for 7 months, and an office assistant who has been there about a month. Contract laborers

are hired as needed, and the manager is in the process of training his two sons as backup operators.

Previously, the system did not have an office and records were kept at the former operator's home. The current GM/operator negotiated a 50 year lease with the local church on a piece of property and the local school district donated a building for an office. All available records have been transferred from the former operator's home to the new office.

The current customer rates are \$40 per month and include 2,000 gallons of water. An average water bill is \$60 - \$65 a month. It is unclear when these rates went into effect, but the manger thinks it was about 3-4 years ago. The prior rates were \$35 per month. They do not charge for wastewater services at this time but will need a rate structure when the new wastewater treatment system is operational. Monthly expenses include salaries, \$350 to the landowner (Mr. Marshall) for checking the wells on his property, \$900 for debt service to USDA (total debt is \$128,000), \$1,000 to Marshall to repay well #1 drilling, and \$0.50 per 1,000 gallons produced by his well. Currently, the system revenues are covering monthly expenses. At this time, the system is able to maintain an average remaining balance of \$60,000 a month for emergencies and additional expenses.

4.1.2.2 General Assessment of Capacity

Based on the team's assessment, this system has an inadequate level of capacity at this time. The people interviewed were enthusiastic about receiving the report and very receptive to receiving any type of technical assistance that is available. The system has the willingness and the potential to become a well-managed system, but there are several areas of present concern. The deficiencies noted could prevent the water system from being able to achieve compliance now or in the future and may also impact the water system's long-term sustainability.

4.1.2.3 Positive Assessment of Capacity

In assessing a system's overall capacity, it is crucial to look at all aspects – positive and negative. It is important for systems to understand those characteristics that are working well, so that those activities can be continued or strengthened. In addition, these positive aspects can assist the system in addressing the capacity deficiencies or concerns. The factors that were particularly important for the Mirando City Water Supply Corporation are listed below.

• **Dedicated Operator and Staff:** The GM/operator has made significant progress during the past 7 months. He is a hard worker and dedicated to improving the water system. He has built a good rapport with the Board, and they now trust him to make critical decisions. He has established important relationships with water supply vendors in Laredo, and technical assistance providers, and communicates with TCEQ (Laredo) on a regular basis. In addition, he has relied on operators in the Laredo utility department for assistance. The relationship between the PWS and its customers has improved since the General Manager was hired and he now has support from the community.

4.1.2.4 Capacity Deficiencies

The following capacity deficiencies were noted in conducting the assessment and seriously impact the ability of the water system to meet compliance with current and future regulations and to ensure long-term sustainability.

- Lack of Sufficient Sources: The two wells produce only about 100 gallons per minute combined. According to TCEQ regulations, this is insufficient, and the wells need to provide at least 140 gpm combined. In addition, TCEQ has indicated the current standpipe does not contain sufficient storage capacity above the required pressure plain. The system plans to develop a third well, which should resolve the pumping capacity issue. The arsenic level of the proposed third well is unknown at this time. However, they believe that if the new well is drilled to less than 300 ft, there is a chance the arsenic concentration may not exceed the MCL of 0.010 mg/L. In addition, the system indicated that they have access to a local rancher's well for emergencies. However, it has not been approved for potable use by TCEQ.
- Lack of Budget and Other Financial Records: All of the system's records are in boxes that were moved from the former operator's house. These records have not been reviewed. The office manager stated that there are no financial reports for 2008 and 2009. The last one was completed in 2007. The system does not have an operating budget but is currently working on one. However, without tracking expenses and revenues on a monthly basis, it is not possible to know if the revenue collected through user charges is sufficient to cover the cost of current operations, repair and replacement, compliance with the arsenic regulations, and provide a reserve fund.

4.1.2.5 Potential Capacity Concerns

The following items were a concern regarding capacity but no specific operational, managerial, or financial problems can be attributed to these items at this time. The system should consider the items listed below to further improve technical, managerial, and financial capabilities and to improve the system's long-term sustainability.

- Sustainability of Water Supply: The system is both paying for water and paying for drilling of additional wells on the private property. While the price for water produced is confirmed in writing and is reviewed every 10 years, in the interest of maintaining a long-term water supply, the system should look into purchasing the groundwater wells.
- Board of Directors: The former operator of the system believed he was still employed when the board decided to hire a general manager/operator. It was several months before the records were transferred and the new manager/operator was able to take over the system. This lack of direction and clarification of roles by the board has created confusion in the past. The board members should consider attending training on their roles and responsibilities and improving their communication skills.

- Operations and Maintenance: Routine and preventative maintenance of the system was neglected by the former operator. The GM/operator has implemented a flushing program and is working on replacing valves, as he has time. However, it is important that enough staff time is available to perform preventive maintenance to avoid problems and prolong the life of equipment. This is especially important as the system begins wastewater treatment.
- Rate Review: None of the current staff were involved in the last rate increase. It will be necessary for the staff and the board members to fully understand the cost of supplying water to customers and to review current rates and develop a rate structure that will provide for the system's long term needs. In addition, it will be important to track expenses and revenues separately for water and wastewater to ensure that all costs are covered and to maintain sufficient reserves.
- Outstanding Collections: The system has approximately \$33,000 in uncollected accounts. While they have made progress in collecting past due accounts, this is a substantial amount of their revenues. It would be important to review policies on late fees and disconnections for non-payment to ensure that the system has adequate revenues to operate

4.2 ALTERNATIVE WATER SOURCE DEVELOPMENT

4.2.1 Identification of Alternative Existing Public Water Supply Sources

Using data drawn from the TCEQ drinking water and TWDB groundwater well databases, the PWSs surrounding the Mirando City PWS were reviewed with regard to their reported drinking water quality and production capacity. PWSs that appeared to have water supplies with water quality issues were ruled out from evaluation as alternative sources, while those without identified water quality issues were investigated further. Small systems were only considered if they were established residential or non residential systems within 5 miles of the Mirando City PWS. Large systems or systems capable of producing greater than four times the daily volume produced by the study system were considered if they were within 35 miles of the study system. A distance of 35 miles was considered to be the upper limit of economic feasibility for constructing a new water line. Table 4.1 is a list of the selected PWSs based on these criteria for large and small PWSs within 35 miles of Mirando City. If it was determined these PWSs had excess supply capacity and might be willing to sell the excess, or might be a suitable location for a new groundwater well, the system was taken forward for further consideration and identified with "EVALUATE FURTHER" in the comments column of Table 4.1.

Selected Public Water Systems within 35 Miles of the Mirando City **Table 4.1**

| PWS ID | PWS Name | Distance from Mirando City (miles) | Comments/Other Issues |
|--------------------|--|------------------------------------|--|
| 2400006 | OILTON RURAL WSC | 2.79 | Small GW System. WQ Issues: TDS. Evaluate Further |
| 2400003 | BRUNI RURAL WSC | 7.37 | Small GW System. WQ Issues: Arsenic, Gross Alpha, Total Uranium, Gross Alpha. |
| 2400009 | WEBB CONSOLIDATED SCHOOLS BRUNI | 7.47 | Small GW System. WQ Issues: Arsenic |
| 1240001 | JIM HOGG COUNTY WCID 2 | 19.08 | Large GW System. WQ Issues: Arsenic, TDS |
| 2400029 | WEBB COUNTY WATER UTILITES COLORADO ACRES | 21.41 | Small GW/Purchase Water System. WQ Issues: TDS. Evaluate Further |
| 0660014 | REALITOS-DUVAL COUNTY CONSERVATION & RECLAMATION D | 26.48 | Small GW System. WQ Issues: TDS. Associated with Benavides-Duval County Conservation and Reclamation PWS. Evaluate Further |
| 2400022 | WEBB COUNTY WATER UTILITIES | 27.25 | Large Surface Water System. WQ Issues: None. Evaluate Further. |
| 2530023 | ZAPATA COUNTY WCID HWY 16 EAST | 30.66 | Small Purchase Water System. WQ Issues: None. Systems currently purchasing water are not considered. |
| 0660002 | FREER WCID | 32.03 | Large GW System. WQ Issues: Arsenic, Iron, TDS, Gross Alpha. |
| 0000001 | BENAVIDES-DUVAL COUNTY CONSERVATION & | 24.5 | Lavge CW System WO leavest Argenia TDS Cross Alpha |
| 2400033 | RECLAMATION TXDOT TRAVEL INFORMATION CENTER | 34.5 36.4 | Large GW System. WQ Issues: Arsenic, TDS, Gross Alpha. Small GW System. WQ Issues: Iron, Sulfate, TDS |
| 2400033 | CITY OF LAREDO | 36.4 | Large GW/Surface Water System. WQ Issues: None |
| | CONCEPCION-DUVAL COUNTY CONSERVATION & | | |
| 0660015 | RECLAMATION EL PRIMERO TRAINING | 36.99 | Small GW System. WQ Issues: Arsenic, TDS |
| 2400024 | CENTER | 37.94 | Small GW System. WQ Issues: Sulfate, TDS |
| 2400027 | SANTA ISABEL RO UNIT | 38.45 | Small GW System. WQ Issues: None |
| 2530022 | RAMIRENO WSC | 38.91 | Small Purchase Water System. WQ Issues: None |
| 2530003 2530004 | SAN YGNACIO MUD SIESTA SHORES WCID | 39.68 43.47 | Large Surface Water System. WQ Issues: None Small Surface Water System. WQ Issues: None |
| | ZAPATA COUNTY | | |
| 2530002 0660020 | WATERWORKS GONZALITOZ GROCERY STORE | 43.47 43.87 | Large Surface Water System. WQ Issues: None Small GW System. WQ Issues: Iron, Manganese, TDS |
| 2400028 | CITY OF COLUMBIA BRIDGE LAREDO | 45.58 | Small Surface Water System. WQ Issues: None |
| 1250005 | PALITO BLANCO ELEMENTARY SCHOOL | 48.65 | Small GW System. WQ Issues: TDS |
| 0660017 | CITY OF SAN DIEGO GLOSSBRENNER UNIT | 48.77 | Small Purchased Water System. WQ Issues: None |
| 1420002 0660003 | ENCINAL WSC SAN DIEGO MUD 1 | 49.01 49.29 | Small System. WQ Issues: None Large GW System. WQ Issues: Arsenic, Iron |
| 1110 | = water quality | | |

WQ = water quality

2 3 4 5 GW = groundwater

 $SW = surface \ water$

 $WSC = water\ supply\ corporation$

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8 9 After the PWSs in Table 4.1 with water quality problems were eliminated from further consideration, the remaining PWSs were screened by proximity to Mirando City PWS and sufficient total production capacity for selling or sharing water. Based on the initial screening summarized in Table 4.1, four alternatives were selected for further evaluation. These alternatives are summarized in Table 4.2. The four alternatives are connections to the Oilton Rural PWS, the Webb County Water Utilities Colorado Acres Water System, the Realitos-Duval County Conservation and Reclamation District, and the Webb County Water Utilities. Descriptions of the water systems that could potentially supply water follow Table 4.2.

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Table 4.2 Public Water Systems within the Vicinity of the Mirando City PWS Selected for Further Evaluation

| PWS ID | PWS Name | Pop | Connections | Total Production (mgd) | Avg Daily Usage (mgd) | Approx. Dist. from Mirando City | Comments/Other Issues |
|-----------|---|----------|-------------|------------------------------|--------------------------------|---------------------------------|---|
| 2400006 | Oilton Rural Water Supply Corp | 350 | 184 | 0.13 | 0.047 | 4.8 | System with available water to purchase or area can be considered as a new well option. Recent results indicate As and TDS below limits. |
| 2400029 | Webb County Water Utilities Colorado Acres Dispenser | 200 0 | 1 | 0.173 | 0.031 | 21.4 | Dispenser meeting 25% of demand through RO treatment of on-site GW well and remaining 75% demand is met through purchase of water from City of Laredo. |
| 0660014 | Realitos-Duval County Conservation & Reclamation District | 400 | 92 | 0.1 | 0.03 | 26.5 | Water source is a single groundwater well completed to a depth of 800 feet. WQ is fine. PWS is part of the network associated with the Benavides-Duval PWS. |
| 2400022 | Webb County Water Utilities | 635 6 | 1816 | 2.5 | 1.354 | 27.3 | Surface water supplier which treats water from the Rio Grande |

 $^{1\}overline{2}$ GW = groundwater

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4.2.1.1 Oilton Rural Water Supply Corp (2400006)

Oilton Rural PWS is located approximately 3 miles north of Mirando City. It serves a population of 350 with 184 metered connections. The PWS is supplied by three local groundwater wells drilled to depths of 460 feet, 395 feet, and 400 feet. The total system production is 0.130 mgd. The average daily consumption is 0.047 mgd which means that the Oilton PWS is utilizing less than 50 percent of the total system production. The water is used primarily for residential purposes. The water is chlorinated for disinfection before distribution. The system has total tank storage of 0.14 MG. Oilton PWS already sells water to the Webb

 $^{1\}overline{3}$ WSC = water supply corporation

- 1 County ISD, and has discussed selling water to Mirando City in the past. No water quality
- 2 issues are reported for the Oilton PWS system in the TCEQ database.

4.2.1.2 Webb County Water Utilities Colorado Acres Dispenser (2400029)

4 Webb County Water Utilities Colorado Acres Dispenser is located approximately 21 miles north of Mirando City or about 23 miles northeast of Laredo along Highway 59. Water is 5 6 provided to about 2000 people who bring their containers to the dispenser as needed. It should 7 be noted that the population in 2006 served by this PWS was approximately 600. The PWS 8 pumps water from their well and the water is treated through an RO unit. The water treated 9 through the on-site RO unit meets approximately 25% of the demand. The remaining 75% is transported from the City of Laredo and stored in an on-site 50,000-gallon storage tank. 10 11 Customers are charged \$1.00 per 300 gallons.

12 4.2.1.3 Realitos-Duval County Conservation and Reclamation District (0660014)

The Realitos-Duval County Conservation and Reclamation District is located approximately 26 miles east from Mirando City. The PWS operates a single water supply well at an average rate of 0.003 MGD and serves a population of 209 or 85 connections. The Realitos-Duval County Conservation and Reclamation District is part of the same District that also serves the towns of Concepcion (PWS #0660015) and Benavides (PWS #0660001). A Feasibility Study under the BEG program was completed for Benavides in 2008. As a result of the BEG Study, information was obtained for both Realitos and Concepcion which both had noncompliant water due to arsenic and nitrate levels. In 2005, a 350-foot deep well was replaced by a 634-foot deep well at Realitos and Concepcion replaced a 210-foot deep well with a 780-foot deep well. Installation of a pipeline to Realitos may be cost prohibitive for Mirando City; however, installation of a deeper well (eastward towards Realitos) may be a viable alternative.

4.2.1.4 Webb County Water Utilities (2400022)

Webb County Water Utilities operates a water treatment plant which treats water pumped from the Rio Grande River. The plant provides compliant water to a population of 6360 or 1820 connections and is located approximately 27 miles from Mirando City. The capability of this PWS to provide compliant water to Mirando City was not determined during this study.

4.2.2 Potential for New Groundwater Sources

4.2.2.1 Installing New Compliant Wells

Developing new wells or well fields is recommended, provided good quality groundwater available in sufficient quantity can be identified. Since a number of water systems in the area have water quality problems, it should be possible to share in the cost and effort of identifying compliant groundwater and constructing well fields.

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Installation of a new well in the vicinity of the system intake point is likely to be an attractive option provided compliant groundwater can be found, since the PWS is already familiar with operation of a water well. As a result, existing nearby wells with good water quality should be investigated. Re-sampling and test pumping would be required to verify and determine the quality and quantity of water at those wells.

The use of existing wells should probably be limited to use as indicators of groundwater quality and availability. If a new groundwater source is to be developed, it is recommended that a new well or wells be installed instead of using existing wells. This would ensure well characteristics are known and meet standards for drinking water wells.

Some of the alternatives suggest new wells be drilled in areas where existing wells have acceptable water quality. In developing the cost estimates, Parsons assumed the aquifer in these areas would produce the required amount of water with only one well. Site investigations and geological research, which are beyond the scope of this study, could indicate whether the aquifer at a particular site and depth would provide the amount of water needed or if more than one well would need to be drilled in separate areas.

4.2.2.2 Results of Groundwater Availability Modeling

The southeastern section of Webb County, where the Mirando City PWS is located, is a transition area between the Gulf Coast aquifer and the Yegua-Jackson aquifer. The single well operated by the Mirando City PWS is completed at a depth of 540 feet in the Jasper Formation of the Gulf Coast Aquifer.

A search of registered wells was conducted using TCEQ's Public Water Supply database to assess groundwater sources utilized within a 10-mile radius of the PWS. The search indicated that most wells in operation within the search are utilized for public supply and domestic use, with stock watering as a secondary use. For the most part, the water source for register wells is the Catahoula Sandstone Formation of the Gulf Coast aquifer that underlies the Jasper Formation. West of the Mirando City PWS, a few wells completed in the Yegua-Jacson aquifer are registered as public water supplies.

Groundwater Supply

The Gulf Coast Aquifer is a high-yield aquifer composed of discontinuous sand, silt, clay and gravel beds that extends over the entire Texas coastal region. It is composed of five hydrogeologic units, from the land surface downward, the Chicot Aquifer, the Evangeline Aquifer, the Burkenville Formation, the Jasper Aquifer, and the Catahoula Sandstone Formation. Only the base of the Gulf Coast aquifer reaches southeast Webb County. Detailed regional geology has been previously discussed in Section 3.

In the southern section of the Gulf Coast Aquifer underlying the Mirando City PWS, the groundwater yield is relatively low compared to the north section and central sections of the aquifer, and of lower water quality due to a high content of total dissolved solids (TWDB 2007b).

Groundwater Availability

Regional groundwater withdrawal in the Mirando City PWS area is extensive, and likely to increase over current levels over the next decades. For Webb County, the 2007 State Water Plan projected that water needs during the 2010-2060 period will substantially increase, from 12,363 AFY to 102,857 AFY. Nearly all of the anticipated increase in water demand will be associated with municipal water supply. The Yegua-Jackson aquifer has only minor utilization in the PWS vicinity, and is not expected to become a significant source of groundwater.

A GAM was developed by TWDB for the southern section of the Gulf Coast aquifer. On a regional basis, the GAM model predicted that by the year 2050, current aquifer utilization would increase more than 10 percent (Chowdhury and Mace, 2003). A GAM evaluation was not run for the Mirando City PWS. Water use by the system would represent a minor addition to regional withdrawal conditions, making potential changes in aquifer levels beyond the spatial resolution of the regional GAM model.

4.2.3 Potential for New Surface Water Sources

The Mirando City PWS is located in the northwest margin of the Nueces-Rio Grande Coastal Basin, in close proximity to the Rio Grande Basin. The surface water demand in the Nueces-Rio Grande Coastal Basin is expected to increase over the next 50 years due increased population, and decline in the groundwater supply due to overpumping and salinization. The 2007 Texas State Water Plan estimated the basin's surface water availability in 2010 to be approximately 8,900 AFY.

There is a minimum potential for development of new surface water sources for Mirando City PWS as indicated by results of surface water availability model for the Nueces-Rio Grande Coastal Basin. Surface water availability maps developed by TCEQ for the basin --illustrating percent of months of flow per year during the drought of record-- indicate that in the southeast section of Webb County, unappropriated flows for new applications are typically available less than 50 percent of the time. This availability is inadequate for development of new municipal water supplies as a 100 percent year-round availability is required by TCEQ for new surface water source permit applications.

4.2.4 Options for Detailed Consideration

The initial review of alternative sources of water results in the following options for moredetailed consideration:

- 1. Oilton Rural PWS. Treated water would be purchased from the Oilton Rural PWS to be used by the Mirando City PWS. A pipeline would be constructed to convey water from the Oilton Rural PWS to the Mirando City PWS (MC-1).
- 2. Webb County Water Utilities Colorado Acres Water System. Treated water would be purchased from the Webb County Water Utilities Colorado Acres Water System to be used by the Mirando City PWS. A pipeline would be constructed to convey

- water from the Webb County Water Utilities Colorado Acres Water System to Mirando City PWS (Alternative MC-2).
 - 3. Mirando City PWS. New shallower groundwater wells would be completed at Mirando City PWS that would produce compliant water in place of the water produced by the existing wells. (Alternative MC-3).
 - 4. New Wells at 10, 5, and 1 mile. Installing a new well within 10, 5, or 1 mile of the Mirando City PWS may produce compliant water in place of the water produced by the existing active well. A pipeline and pump station would be constructed to transfer the water to the Mirando City PWS (Alternatives MC-4, MC-5, and MC-6).

4.3 TREATMENT OPTIONS

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11 4.3.1 Centralized Treatment Systems

- 12 Centralized treatment of the well water is identified as a potential option. Reverse osmosis
- 13 (RO), and iron-based adsorption treatment, could be potentially applicable processes. Both
- processes can reduce arsenic to produce compliant water. The central RO treatment alternative
- is Alternative MC-7 and the adsorption treatment is Alternative MC-8.

16 **4.3.2 Point-of-Use Systems**

POU treatment using RO technology is valid for arsenic removal. The POU treatment alternative is MC-9.

19 4.3.3 Point-of-Entry Systems

POE treatment using RO technology is valid for arsenic removal. The POE treatment alternative is MC-10.

22 4.4 BOTTLED WATER

- Providing bottled water is considered an interim measure to be used until a compliance
- 24 alternative is implemented. Even though the community is small and people know each other;
- 25 it would be reasonable to require a quarterly communication advising customers of the need to
- 26 take advantage of the bottled water program. An alternative to providing delivered bottled
- 27 water is to provide a central, publicly accessible dispenser for treated drinking water.
- Alternatives addressing bottled water are MC-11, MC-12, and MC-13.

4.5 ALTERNATIVE DEVELOPMENT AND ANALYSIS

- A number of potential alternatives for compliance with the MCL for arsenic have been
- 31 identified. Each of the potential alternatives is described in the following subsections. It
- 32 should be noted that the cost information given is the capital cost and change in O&M costs
- associated with implementing the particular alternative. Appendix C contains cost estimates for

- the compliance alternatives. These compliance alternatives represent a range of possibilities,
- and a number of them are likely not feasible. However, all have been presented to provide a
- complete picture of the range of alternatives considered. It is anticipated that a PWS will be
- 4 able to use the information contained herein to select the most attractive alternative(s) for more
- 5 detailed evaluation and possible subsequent implementation.

4.5.1 Alternative MC-1: Purchase Treated Water from the Oilton Rural Water Supply Corporation

This alternative involves purchasing potable water from the Oilton Rural PWS, which will be used to supply the Mirando City PWS. The Oilton Rural PWS currently has sufficient excess capacity for this alternative to be feasible. For purposes of this report, to allow direct and straightforward comparison with other alternatives, this alternative assumes that water would be purchased from the Oilton Rural PWS. Also, it is assumed that Mirando City would obtain all its water from the Oilton Rural PWS.

This alternative would require construction of a 5,000-gallon feed tank at a point adjacent to an Oilton Rural PWS water main, and a new pipeline from the feed tank to the existing storage tank located at the Mirando City PWS. One pump station would also be required to overcome pipe friction and the elevation differences between the two systems. The required pipeline would be constructed of 6-inch pipe and would follow south on J.C. Perez Road to State Hwy 359, then west to Farm-to-Market (FM) Rd 649, then south on FM 649 to Dr JW Edgar Street to the Mirando City PWS. Using this route, the pipeline required would be approximately 4.8 miles long. The pipeline would terminate at the existing storage tank owned by the Mirando City PWS.

The pump station would include two pumps, including one standby, and would be housed in a building. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Mirando City PWS, since the incremental cost would be relatively small, and it would provide operational flexibility.

This alternative has the potential to provide a regional solution, as there are several PWSs in the vicinity that have a need for compliant water. PWSs located close to the proposed pipeline route could share the cost of drilling the new well and pipeline construction.

By definition this alternative involves regionalization, since Mirando City would be obtaining drinking water from an existing larger supplier. Also, other PWSs near Mirando City are in need of compliant drinking water and could share in implementation of this alternative.

The estimated capital cost for this alternative includes completing the new well, and constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$1.17 million, with an estimated annual O&M cost of \$22,500. If the purchased water was used for blending rather than for the full water supply, the annual O&M cost for this alternative could be reduced because of reduced

pumping costs and reduced water purchase costs. However, additional costs would be incurred for equipment to ensure proper blending, and additional monitoring to ensure the finished water is compliant.

The reliability of adequate amounts of compliant water under this alternative should be good. From Mirando City PWS's perspective, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood, and Mirando City PWS personnel currently operate pipelines and a pump station. If the decision was made to perform blending then the operational complexity would increase.

The feasibility of this alternative would be dependent on Mirando City PWS being able to reach an agreement with Oilton Rural PWS to install a new groundwater well.

4.5.2 Alternative MC-2: Purchase Treated Water from the Webb County Water Utilities Colorado Acres Dispenser

This alternative involves purchasing potable water from the Webb County Water Utilities Colorado Acres Water System, which will be used to supply the Mirando City PWS. The Webb County Water Utilities Colorado Acres Water System currently has sufficient excess capacity for this alternative to be feasible. For purposes of this report, to allow direct and straightforward comparison with other alternatives, this alternative assumes that water would be purchased from the Webb County Water Utilities Colorado Acres Water System. Also, it is assumed that Mirando City would obtain all its water from the Webb County Water Utilities Colorado Acres Water System.

This alternative would require construction of two 5,000-gallon feed tanks at a point adjacent to a Webb County Water Utilities Colorado Acres Water System water main, and a new pipeline from the feed tank to the existing storage tank located at the Mirando City PWS. Two pump stations would also be required to overcome pipe friction and the elevation differences between Webb County Water Utilities and Mirando City PWS. The required pipeline would be constructed of 6-inch pipe and would cross Rural Route 2805 and follow numerous minor roads between US Hwy 59 and State Hwy 359 to the Mirando City PWS. Using this route, the required pipeline would be approximately 27 miles in length. The pipeline would terminate at the existing storage tank owned by the Mirando City PWS.

The pump stations would include four pumps, including two standby, and would be housed in a building. A tank would also be constructed for the pumps to draw from. It is assumed the pumps and piping would be installed with capacity to meet all water demand for the Mirando City, since the incremental cost would be relatively small, and would provide operational flexibility.

By definition this alternative involves regionalization, since Mirando City would be obtaining drinking water from an existing larger supplier. Also, other PWSs near Mirando City are in need of compliant drinking water and could share in implementation of this alternative.

The estimated capital cost for this alternative includes constructing the pipeline and pump station. The estimated O&M cost for this alternative includes the purchase price for the treated water minus the cost related to current operation of the Mirando City wells, plus maintenance cost for the pipeline, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$5.56 million, with an estimated annual O&M cost of \$119,500.

The reliability of adequate amounts of compliant water under this alternative should be good. Webb County Water Utilities Colorado Acres Water System provides treated surface water on a large scale, facilitating adequate O&M resources. From Mirando City's perspective, this alternative would be characterized as easy to operate and repair, since O&M and repair of pipelines and pump stations is well understood. If the decision was made to perform blending then the operational complexity would increase.

The feasibility of this alternative is dependent on an agreement being reached with the Webb County Water Utilities Colorado Acres Water System to purchase treated drinking water.

4.5.3 Alternative MC-3: New Wells at the Current Mirando City PWS Location

This alternative would require completing three new shallower wells at the current Mirando City PWS site and tying them into an existing water system. The new wells would be approximately 300 feet deep. Base on the water quality data collected from a nearby non-potable well, it is expected that groundwater from 300 feet deep may be compliant with drinking water MCLs.

Since the new wells would be on site and close to the existing system, a pump station would not be necessary. The required pipeline would be constructed of 6-inch pipe, approximately 100 feet in length, and would follow the existing pipeline route to the Mirando City PWS. The pipeline would terminate at the existing storage tank owned by the Mirando City PWS.

This alternative would provide a regional solution since there are several PWSs in the vicinity that currently rely on Mirando City PWS to provide them compliant water. PWSs located close to the proposed pipeline route could share the cost of drilling the new wells and pipeline construction.

The estimated capital cost for this alternative includes completing the new wells, constructing the connection piping, and set the feed pump to supply water to the existing system. The estimated O&M cost for this alternative includes the maintenance cost for the connection piping, and power and O&M labor and materials for the pump station. The estimated capital cost for this alternative is \$302,600, with an estimated annual O&M cost of \$8,500.

The reliability of adequate amounts of compliant water under this alternative should be good. From Mirando City PWS's perspective, this alternative would be characterized as easy to

- 1 operate and repair, since O&M and repair of pipelines and pump stations is well understood,
- 2 and Mirando City PWS personnel currently operate pipelines and a pump station. If the
- 3 decision was made to perform blending then the operational complexity would increase.

4.5.4 Alternative MC-4: New Well at 10 miles

 This alternative consists of installing three new wells within 10 miles of the Mirando City PWS that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify existing wells or the location where new wells could be installed.

This alternative would require constructing three new 300-foot wells, a new pump station with a 5,000-gallon feed tank near the new wells, and a pipeline from the new well/feed tank to the existing intake point for the Mirando City PWS system. The pump station and feed tank would be necessary to overcome pipe friction and changes in land elevation. For this alternative, the pipeline is assumed to be approximately 10 miles long, and would be a 6-inches in diameter and discharge to the existing storage tank at the Mirando City PWS. The pump station would include a feed tank, two transfer pumps, including one standby, and would be housed in a building.

The estimated capital cost for this alternative includes installing the wells, constructing the pipeline, pump station, feed tank, service pumps, and pump house. The estimated O&M cost for this alternative includes O&M for the pipeline and pump stations. The estimated capital cost for this alternative is \$2.37 million, and the estimated annual O&M cost for this alternative is \$39,100.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of the Mirando City PWS, this alternative would be similar to operate as the existing system. Mirando City PWS personnel have experience with O&M of wells, pipelines, and pump stations.

The feasibility of this alternative is dependent on the ability to find adequate existing wells or success in installing wells that produces an adequate supply of compliant water.

4.5.5 Alternative MC-5: New Well at 5 miles

This alternative consists of installing three new wells within 5 miles of the Mirando City that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify existing wells or the location where new wells could be installed.

This alternative would require constructing three new 300-foot wells, a new pump station with a 5,000 gallon feed tank near the new well, and a pipeline from the new well/feed tank to the existing intake point for the Mirando City PWS system. The pump station and feed tank would be necessary to overcome pipe friction and changes in land elevation. For this

alternative, the pipeline is assumed to be 6-inches in diameter, approximately 5 miles long, and would discharge to the existing storage tank at the Mirando City PWS. The pump station would include two transfer pumps, including one standby, and would be housed in a building.

The estimated capital cost for this alternative includes installing the wells, and constructing the pipeline and pump station. The estimated O&M cost for this alternative includes O&M for the pipeline and pump station. The estimated capital cost for this alternative is \$1.41 million, and the estimated annual O&M cost for this alternative is \$36,900.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells, pump stations and pipelines are commonly employed. From the perspective of the Mirando City PWS, this alternative would be similar to operate as the existing system. Mirando City PWS personnel have experience with O&M of wells, pipelines and pump stations.

The feasibility of this alternative is dependent on the ability to find adequate existing wells or success in installing wells that produces an adequate supply of compliant water.

4.5.6 Alternative MC-6: New Well at 1 mile

This alternative consists of installing three new wells within 1 mile of the Mirando City PWS that would produce compliant water in place of the water produced by the existing wells. At this level of study, it is not possible to positively identify existing wells or the location where new wells could be installed.

This alternative would require constructing three new 300-foot wells and a pipeline from the new wells to the existing intake point for the Mirando City PWS system. Since the new wells are relatively close, a pump station would not be necessary. For this alternative, the pipeline is assumed to be 6 inches in diameter, approximately 1 mile long, and would discharge to the existing storage tank at the Mirando City PWS.

The estimated capital cost for this alternative includes installing the wells, and constructing the pipeline. The estimated O&M cost for this alternative includes O&M for the pipeline. The estimated capital cost for this alternative is \$457,100, and the estimated annual O&M cost for this alternative is \$8,600.

The reliability of adequate amounts of compliant water under this alternative should be good, since water wells and pipelines are commonly employed. From the perspective of the Mirando City PWS, this alternative would be similar to operate as the existing system. Mirando City personnel have experience with O&M of wells, pipelines and pump stations.

The feasibility of this alternative is dependent on the ability to find adequate existing wells or success in installing wells that produces an adequate supply of compliant water. It is possible an alternate groundwater source would not be found on land owned by Mirando City PWS, so landowner cooperation may be required.

4.5.7 Alternative MC-7: Central RO Treatment

This system would continue to pump water from the Mirando City PWS wells, and would treat the water through an RO system prior to distribution. For this option, 73 percent of the raw water would be treated in a slip stream to obtain compliant water. TO obtain the peak flow requirement of 130 gpm, a total well flow of 160 gpm needs to be produced. The current two wells are not capable of producing this flow. It is estimated the total RO reject generation would be approximately 20,000 gallons per day (gpd) when the system is operated at the average daily consumption 80,000 gpd.

This alternative consists of constructing the RO treatment plant near the existing wells. The plant comprises a 960 square foot building with a paved driveway; a skid with the preconstructed RO plant; three transfer pumps, a 26,000-gallon tank for storing the treated water. The cost estimate assumes that the RO reject will be stored in a lined pond and trucked periodically to a neighboring WWTP with the capacity to absorb the additional flow. The assumed roundtrip distance is 10 miles. The treated water would be chlorinated and stored in the new treated water tank prior to being pumped into the distribution system. The entire facility is fenced.

The estimated capital cost for this alternative is \$1,160,000, and the estimated annual O&M cost is \$121,800.

The reliability of adequate amount of compliant water under this alternative is good, since RO treatment is a common and well-understood treatment technology. However, O&M efforts required for the central RO treatment plant may be significant, and O&M personnel would require training with RO. The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.8 Alternative MC-8: Central Iron Adsorption Treatment

The system would treat groundwater from the existing wells using an iron-based adsorption system prior to distribution. This alternative consists of constructing the adsorption treatment plant near the well. The plant comprises a 960 ft² building with a paved driveway, the preconstructed adsorption system on a skid (*e.g.*, one AdEdge APU-100 package units), and a 3,600-gallon backwash tank. The water would be pre-chlorinated to oxidize AS(III) to AS(V) and post chlorinated for disinfection prior to pumping to the existing standpipe. Backwash would be required every three weeks with raw well water supplied directly by the well pump. The backwash wastewater would be discharged to a 4,400 gallon earthen pond. The adsorption media are expected to pass 40,000 bed volumes before replacement and disposal.

The estimated capital cost for this alternative is \$591,400, and the estimated annual O&M cost is \$48,700, which includes the annual media replacement cost of \$10,000. Reliability of supply of adequate amounts of compliant water under this alternative is good as the adsorption technology has been demonstrated effective in full-scale and pilot-scale facilities. The technology is simple and requires minimal O&M effort.

 Coagulation/Filtration was not considered in this analysis, because of the need to add iron salts to obtain an iron/arsenic ratio of 20:1, with need for follow up iron removal and disposal, was considered to be equal in cost to Alternative MC-8.

4.5.9 Alternative MC-9: Point-of-Use Treatment

This alternative consists of the continued operation of the Mirando City PWS well field, plus treatment of water to be used for drinking or food preparation at the point of use to remove arsenic. The purchase, installation, and maintenance of POU treatment systems to be installed "under the sink" would be necessary for this alternative. Blending is not an option in this case.

This alternative would require installing the POU treatment units in residences and other buildings that provide drinking or cooking water. Mirando City PWS staff would be responsible for purchase and maintenance of the treatment units, including membrane and filter replacement, periodic sampling, and necessary repairs. In houses, the most convenient point for installation of the treatment units is typically under the kitchen sink, with a separate tap installed for dispensing treated water. Installation of the treatment units in kitchens will require the entry of Mirando City PWS or contract personnel into the houses of customers. As a result, cooperation of customers would be important for success implementing this alternative. The treatment units could be installed for access without house entry, but that would complicate the installation and increase costs.

Treatment processes would involve RO. Treatment processes produce a reject waste stream. The reject waste streams result in a slight increase in the overall volume of water used. POU systems have the advantage that only a minimum volume of water is treated (only that for human consumption). This minimizes the size of the treatment units, the increase in water required, and the waste for disposal. For this alternative, it is assumed the increase in water consumption is insignificant in terms of supply cost, and that the reject waste stream can be discharged to the house septic or sewer system.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the POU treatment systems. The estimated O&M cost for this alternative includes the purchase and replacement of filters and membranes, as well as periodic sampling and record keeping as required by the Texas Administrative Code (Title 30, Part I, Chapter 290, Subchapter F, Rule 290.106). The estimated capital cost for this alternative is \$189,800, and the estimated annual O&M cost for this alternative is \$183,300. For the cost estimate, it is assumed that one POU treatment unit will be required for each of the 250 connections in the Mirando City PWS system. It should be noted that the POU treatment units would need to be more complex than units typically found in commercial retail outlets in order to meet regulatory requirements, making purchase and installation more expensive. Additionally, capital cost would increase if POU treatment units are placed at other taps within a home, such as refrigerator water dispensers, ice makers, and bathroom sinks. In school settings, all taps where children and faculty receive water may need POU treatment units or clearly mark those taps suitable for

human consumption. Additional considerations may be necessary for preschools or other establishments where individuals cannot read.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of the customers for system installation, use, and maintenance, and only provides compliant water to single tap within a house. Additionally, the O&M efforts (including monitoring of the devices to ensure adequate performance) required for the POU systems will be significant, and the current personnel are inexperienced in this type of work. From the perspective of the Mirando City PWS, this alternative would be characterized as more difficult to operate owing to the in-home requirements and the large number of individual units.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.10 Alternative MC-10: Point-of-Entry Treatment

This alternative consists of the continued operation of the Mirando City PWS well field, plus treatment of water as it enters residences to remove arsenic. The purchase, installation, and maintenance of the treatment systems at the point of entry to a household would be necessary for this alternative. Blending is not an option in this case.

This alternative would require the installation of the POE treatment units at houses and other buildings that provide drinking or cooking water. Every building connected to the system must have a POE device installed, maintained, and adequately monitored. TCEQ must be assured the system has 100 percent participation of all property and or building owners. A way to achieve 100 percent participation is through a public announcement and education program. Example public programs are provided in the document "Point-of-Use or Point-of-Entry" Treatment Options for Small Drinking Water Systems" published by USEPA. The property owner's responsibilities for the POE device must also be contained in the title to the property and "run with the land" so subsequent property owners understand their responsibilities (USEPA 2006).

Mirando City PWS would be responsible for purchase, operation, and maintenance of the treatment units, including membrane and filter replacement, periodic sampling, and necessary repairs. It may also be desirable to modify piping so water for non-consumptive uses can be withdrawn upstream of the treatment unit. The POE treatment units would be installed outside the residences, so entry would not be necessary for O&M. Some cooperation from customers would be necessary for installation and maintenance of the treatment systems.

POE treatment for arsenic would involve RO. Treatment processes produce a reject stream that requires disposal. The reject water stream results in a slight increase in overall volume of water used. POE systems treat a greater volume of water than POU systems. For this alternative, it is assumed the increase in water consumption is insignificant in terms of supply cost, and that the backwash reject waste stream can be discharged to the house septic or sewer system.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the POE treatment systems. The estimated O&M cost for this alternative includes the purchase and replacement of filters and membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is \$3.92 million, and the estimated annual O&M cost for this alternative is \$553,800. For the cost estimate, it is assumed that one POE treatment unit will be required for each of the 250 existing connections to the Mirando City PWS system.

The reliability of adequate amounts of compliant water under this alternative are fair, but better than POU systems since it relies less on the active cooperation of the customers for system installation, use, and maintenance, and compliant water is supplied to all taps within a house. Additionally, the O&M efforts required for the POE systems will be significant, and the current personnel are inexperienced in this type of work. From the perspective of the Mirando City PWS, this alternative would be characterized as more difficult to operate owing to the on-property requirements and the large number of individual units.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.11 Alternative MC-11: Public Dispenser for Treated Drinking Water

This alternative consists of the continued operation of the Mirando City PWS wells, plus dispensing treated water for drinking and cooking at a publicly accessible location. Implementing this alternative would require purchasing and installing a treatment unit where customers would be able to come and fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of water requires treatment, but customers would be required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

Mirando City PWS personnel would be responsible for maintenance of the treatment unit, including media or membrane replacement, periodic sampling, and necessary repairs. The spent media or membranes will require disposal. This alternative relies on a great deal of cooperation and action from the customers in order to be effective.

This alternative does not present options for a regional solution.

The estimated capital cost for this alternative includes purchasing and installing the treatment system to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes purchasing and replacing filters and media or membranes, as well as periodic sampling and record keeping. The estimated capital cost for this alternative is \$18,400, and the estimated annual O&M cost for this alternative is \$34,900.

The reliability of adequate amounts of compliant water under this alternative is fair, because of the large amount of effort required from the customers and the associated inconvenience. Mirando City PWS has not provided this type of service in the past. From Mirando City PWS's perspective this alternative would be characterized as relatively easy to operate, since these types of treatment units are highly automated, and there is only one unit.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.12 Alternative MC-12: 100 Percent Bottled Water Delivery

This alternative consists of the continued operation of the Mirando City PWS wells, but compliant drinking water will be delivered to customers in containers. This alternative involves setting up and operating a bottled water delivery program to serve all customers in the system. It is expected that Mirando City PWS would find it most convenient and economical to contract a bottled water service. The bottle delivery program would have to be flexible enough to allow the delivery of smaller containers should customers be incapable of lifting and manipulating 5-gallon bottles. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

This alternative does not involve capital cost for construction, but would require some initial costs for system setup, and then ongoing costs to have the bottled water furnished. It is assumed for this alternative that bottled water is provided to 100 percent of the Mirando City PWS customers.

This alternative does not present options for a regional solution.

The estimated initial capital cost is for setting up the program. The estimated O&M cost for this alternative includes program administration and purchase of the bottled water. The estimated capital cost for this alternative is \$27,600, and the estimated annual O&M cost for this alternative is \$309,700. For the cost estimate, it is assumed that each person requires one gallon of bottled water per day.

The reliability of adequate amounts of compliant water under this alternative is fair, since it relies on the active cooperation of customers to order and utilize the water. Management and administration of the bottled water delivery program will require attention from Mirando City PWS.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.13 Alternative MC-13: Public Dispenser for Trucked Drinking Water

This alternative consists of continued operation of the Mirando City PWS wells, plus dispensing compliant water for drinking and cooking at a publicly accessible location. The compliant water would be purchased from the City of Oilton, and delivered by truck to a tank at

a central location where customers would be able to fill their own containers. This alternative also includes notifying customers of the importance of obtaining drinking water from the dispenser. In this way, only a relatively small volume of water requires treatment, but customers are required to pick up and deliver their own water. Blending is not an option in this case. It should be noted that this alternative would be considered an interim measure until a compliance alternative is implemented.

Mirando City PWS would purchase a truck suitable for hauling potable water, and install a storage tank. It is assumed the storage tank would be filled once a week, and that the chlorine residual would be tested for each truckload. The truck would have to meet requirements for potable water, and each load would be treated with bleach. This alternative relies on a great deal of cooperation and action from the customers for it to be effective.

This alternative presents limited options for a regional solution if two or more systems share the purchase and operation of the water truck.

The estimated capital cost for this alternative includes purchasing a water truck and construction of the storage tank to be used for the drinking water dispenser. The estimated O&M cost for this alternative includes O&M for the truck, maintenance for the tank, water quality testing, record keeping, and water purchase, The estimated capital cost for this alternative is \$189,400, and the estimated annual O&M cost for this alternative is \$30,700.

The reliability of adequate amounts of compliant water under this alternative is fair because of the large amount of effort required from the customers and the associated inconvenience. Current personnel have not provided this type of service in the past. From the perspective of Mirando City PWS, this alternative would be characterized as relatively easy to operate, but the water hauling and storage would have to be done with care to ensure sanitary conditions.

The feasibility of this alternative is not dependent on the cooperation, willingness, or capability of other water supply entities.

4.5.14 Summary of Alternatives

Table 4.3 provides a summary of the key features of each alternative for Mirando City PWS.

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Table 4.3 Summary of Compliance Alternatives for Mirando City PWS

| Alt No. | Alternative Description | Major Components | Capital Cost ¹ | Annual O&M Cost | Total Annualized Cost | Reliability | System Impact | Remarks |
|---------|---|--|---------------------------|--------------------|-----------------------------|------------------------------|------------------|---|
| MC-1 | Purchase treated water from Oilton Rural WSC | - Pump station/feed tank - 4.8-mile pipeline | \$1,171,500 | \$22,500 | \$124,700 | Good | N | Agreement must be successfully negotiated with Oilton Rural Water Supply Corp Water System. Blending may be possible. Costs could possibly be shared with small systems along pipeline route. |
| MC-2 | Purchase treated water from Webb County Water Utilities Colorado Acres | - 2 Pump stations/feed tanks - 27-mile pipeline | \$5,559,800 | \$119,500 | \$604,200 | Good | N | Agreement must be successfully negotiated with Webb County Water Utilities Colorado Acres. Blending may be possible. Costs could possibly be shared with small systems along pipeline route. |
| MC-3 | Drill New shallower wells at Mirando City WSC | - 3 New wells - 100-feet piping | \$302,600 | \$8,500 | \$34,800 | Good | N | New, deeper wells on-site. Sharing cost with neighboring systems may be possible. |
| MC-4 | Install new compliant well within 10 miles | - 3 New wells - Pump station/feed tank - 10-mile pipeline | \$2,372,200 | \$39,100 | \$245,900 | Good | N | May be difficult to find well with good water quality. Costs could possibly be shared with small systems along pipeline route. |
| MC-5 | Install new compliant well within 5 miles | - 3 New wells - Pump station/feed tank - 5-mile pipeline | \$1,409,500 | \$36,900 | \$159,800 | Good | N | May be difficult to find well with good water quality. Costs could possibly be shared with small systems along pipeline route. |
| MC-6 | Install new compliant well within 1 mile | - 3 New wells - Pump station/feed tank - 1-mile pipeline | \$457,100 | \$8,600 | \$48,500 | Good | N | May be difficult to find well with good water quality. |
| MC-7 | Continue operation of Mirando City PWS well field with central RO treatment | - Central RO treatment plant | \$1,159,800 | \$121,800 | \$222,900 | Good | Т | Costs could possibly be shared with nearby small systems. |
| MC-8 | Continue operation of Mirando City PWS well field with central iron adsorption treatment | - Central iron-based adsorption treatment plant | \$591,400 | \$48,700 | \$100,300 | Good | Т | Costs could possibly be shared with nearby small systems. |
| MC-9 | Continue operation of Mirando City well field, and POU treatment | - POU treatment units. | \$189,800 | \$183,300 | \$199,800 | Fair | T, M | Only one compliant tap in home. Cooperation of residents required for installation, maintenance, and testing. |
| MC-10 | Continue operation of Mirando City well field, and POE treatment | - POE treatment units. | \$3,916,700 | \$553,800 | \$895,200 | Fair (better than POU) | T, M | All home taps compliant and less resident cooperation required. |

| Alt No. | Alternative Description | Major Components | Capital Cost ¹ | Annual O&M Cost | Total Annualized Cost | Reliability | System Impact | Remarks | | |
|---------|---|--|---------------------------|--------------------|-----------------------------|-------------------------|------------------|---|--|--|
| MC-11 | Continue operation of Mirando City well field, but furnish public dispenser for treated drinking water | - Water treatment and dispenser unit | \$18,400 | \$34,900 | \$36,500 | Fair/interim measure | Т | Does not provide compliant water to all taps, and requires a lot of effort by customers. | | |
| MC-12 | Continue operation of Mirando City well field, but furnish bottled drinking water for all customers | - Set up bottled water system | \$27,600 | \$309,700 | \$312,100 | Fair/interim measure | М | Does not provide compliant water to all taps, and requires customers to order and use. Management of program may be significant. | | |
| MC-13 | Continue operation of Mirando City well field, but furnish public dispenser for trucked drinking water. | - Construct storage tank and dispenser - Purchase potable water truck | \$189,400 | \$30,700 | \$47,200 | Fair/interim measure | М | Does not provide compliant water to all taps, and requires a lot of effort by customers. | | |

| Notes: | N-No significant increase required in technical or management capability |
|--------|--|
| | T – Implementation of alternative will require increase in technical capability |
| | M – Implementation of alternative will require increase in management capability |
| | 1 – See cost breakdown in Appendix C |

4.6 COST OF SERVICE AND FUNDING ANALYSIS

To evaluate the financial impact of implementing the compliance alternatives, a 30-year financial planning model was developed. This model can be found in Appendix D. The financial model is based on estimated cash flows, with and without implementation of the compliance alternatives. Data for such models are typically derived from established budgets, audited financial reports, published water tariffs, and consumption data. Mirando City PWS operates a PWS with 250 connections, serving a population of approximately 500. Information that was used to complete the financial analysis was based interviews with PWS personnel. The water usage rate for Mirando City PWS was estimated to be 160 gpd per capita based on average daily use and current population. This analysis will need to be performed in a more detailed fashion and applied to alternatives deemed attractive and worthy of more detailed evaluation. A more detailed analysis should include additional factors such as:

Cost escalation,

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- Price elasticity effects where increased rates may result in lower water consumption,
- Costs for other system upgrades and rehabilitation needed to maintain compliant operation.

17 **4.6.1 Financial Plan Development**

Since financial records for Mirando City PWS were not available, revenues and expenses were estimated for this PWS. Annual revenue was estimated based on an average monthly bill of \$62.50, as reported by PWS personnel, resulting in annual revenue of \$187,500. In the absence of data, the expenses were assumed equal to revenues. The average annual water bill is \$750, which is 2.8 percent of the MHI. The Mirando City PWS MHI is less than 75% of the median state household income, which may result in eligibility for some grants and low interest rate loans.

25 4.6.2 Current Financial Condition

26 **4.6.2.1 Cash Flow Needs**

Although expenses are not tracked for Mirando City PWS, it appears revenues may be sufficient to cover expenses based.

4.6.2.2 Ratio Analysis

The Current Ratio for the Mirando City PWS could not be determined due to lack of necessary financial data to determine this ratio.

32 Debt to Net Worth Ratio

A Debt-to-Net-Worth Ratio also could not be determined owing to lack of the necessary financial data to determine this ratio.

Operating Ratio

Because of the lack of complete separate financial data on expenses specifically related to the Mirando City PWS, the Operating Ratio could not be accurately determined.

4.6.3 Financial Plan Results

Each compliance alternative for the Mirando City PWS was evaluated, with emphasis on the impact on affordability (expressed as a percentage of household income), and the overall increase in water rates necessary to pay for the improvements. Each alternative was examined under the various funding options described in Section 2.4.

Results of the financial impact analysis are provided in Table 4.4 and Figure 4.2. Table 4.4 and Figure 4.2 present rate impacts assuming that revenues match expenses, without funding reserve accounts, and that operations and implementation of compliance alternatives are funded with revenue and are not paid for from reserve accounts. Figure 4.2 provides a bar chart that, in terms of the yearly billing to an average customer, shows the following:

- Current annual average bill,
- Projected annual average bill including rate increase, if needed, to match existing expenditures, and
- Projected annual bill including rate increases needed to fund implementation of a compliance alternative (this does not include funding for reserve accounts).

The two bars shown for each compliance alternative represent the rate changes necessary for revenues to match total expenditures assuming 100 percent grant funding and 100 percent loan/bond funding. Most funding options will fall between 100 percent grant and 100 percent loan/bond funding, with the exception of 100 percent revenue financing. Establishing or increasing reserve accounts would require an increase in rates. If existing reserves are insufficient to fund a compliance alternative, rates would need to be raised before implementing the compliance alternative. This would allow for accumulation of sufficient reserves to avoid larger but temporary rate increases during the years the compliance alternative was being implemented.

4.6.4 Evaluation of Potential Funding Options

There are a variety of funding programs available to entities as described in Section 2.4. Mirando City PWS is most likely to obtain funding from programs administered by the TWDB, TDRA, and Rural Development. This report contains information that would be used for an application for funding. Information such as financial analyses, water supply assessment, and records demonstrating health concerns, failing infrastructure, and financial need, may be required by these agencies. This section describes the candidate funding agencies and their appropriate programs as well as information and steps needed to begin the application process.

This report should serve to document the existing water quality issues, infrastructure need and costs, and water system information needed to begin the application process with the TWDB. Although this report is at the conceptual level, it demonstrates that significant funding will be needed to meet Safe Drinking Water Standards. The information provided in this report may serve as the needed documentation to justify a project that may only be possible with significant financial assistance.

4.6.4.1 TWDB Funding Options

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TWDB programs include the Drinking Water State Revolving Fund (DWSRF), Rural Water Assistance Fund (RWAF), State Loan Program (Development Fund II), and Economically Distressed Areas Program (EDAP). Additional information on these programs can be found online at the TWDB website under the Assistance tab, Financial Assistance section, under the Public Works Infrastructure Construction subsection.

Drinking Water State Revolving Fund

The DWSRF offers net long-term interest lending rates below the rate the borrower would receive on the open market for a period no longer than 20 years. A cost-recovery loan origination charge is imposed to cover the administrative costs of operating the DWSRF, but an additional interest rate subsidy is offered to offset the charge. The terms of the loan typically require a revenue or tax pledge. The DWSRF program can provide funds from State sources or Federal capitalization grants. State loans provide a net long-term interest rate of 0.7 percentage points below the rate the borrower would receive on the open market at the time of loan closing and Federal Capitalization Grants provide a lower net long-term interest rate of 1.2 percentage points. "Disadvantaged communities" may obtain loans at even greater subsidies and up to a 30-year loan term.

The loan application process has several steps: pre-application, application and commitment, loan closing, funding and construction monitoring, and any other special requirements. In the pre-application phase, prospective loan applicants are asked to submit a brief DWSRF Information Form to the TWDB that describes the applicant's existing water facilities, additional facility needs and the nature of projects being considered for meeting those needs, project cost estimates, and "disadvantaged community" status. The TCEQ assigns a priority rating that includes an applicant's readiness to proceed. TWDB staff notifies prospective applicants of their priority rating and encourage them to schedule a pre-planning conference for guidance in preparing the engineering, planning, environmental, financial, and water conservation portions of the DWSRF application.

Rural Water Assistance Fund

Small rural water utilities can finance water projects with attractive interest rate loans with short and long-term finance options at tax exempt rates. Funding through this program gives an added benefit to nonprofit water supply corporations as construction purchases qualify for a sales tax exemption. Rural Political Subdivisions are eligible (nonprofit water supply corporations; water districts or municipalities serving a population of up to 10,000; and counties in which no urban area has a population exceeding 50,000). A nonprofit water supply

corporation is eligible to apply these funds for design and construction of water projects. Projects can include line extensions, elevated storage, the purchase of well fields, the purchase or lease of rights to produce groundwater, and interim financing of construction projects. The fund may also be used to enable a rural water utility to obtain water service supplied by a larger utility or to finance the consolidation or regionalization of a neighboring utility.

A maximum financing life is 50 years for projects. The average financing period is 20 to 23 years. System revenues and/or tax pledges are typically required. The lending rate is set in accordance with the TWDB rules in 31 Texas Administrative Code (TAC) 384.5 and the scale varies according to the length of the loan and several factors. The TWDB seeks to provide reasonable rates for its customers with minimal risk to the state. The TWDB posts rates for comparison for applicants, and in August 2010 the TWDB showed its rates for a 22-year, taxable loan at 7.07 percent, where the market was at 8.47 percent. Funds in this program are not restricted.

The TWDB's Office of Project Finance and Construction Assistance staff can discuss the terms of the loan and assist applicants during preparation of the application, and this is encouraged. The application materials must include an engineering feasibility report, environmental information, rates and customer base, operating budgets, financial statements, and project information. The TWDB considers the needs of the area; benefits of the project; the relationship of the project to the overall state water needs; relationship of the project to the State Water Plan; and availability of all sources of revenue to the rural utility for the ultimate repayment of the water supply project cost. The board considers applications monthly.

State Loan Program (Development Fund II)

The State Loan Program is a diverse lending program directly from state funding sources. As it does not receive federal subsidies, it is more streamlined. The loans can incorporate more than one project under the umbrella of one loan. Water supply corporations are eligible, but will have taxable rates. Projects can include purchase of water rights, treatment plants, storage and pumping facilities, transmission lines, well development, and acquisitions.

The loan requires that the applicant pledge revenue or taxes, as well as some collateral for Mirando City PWS. The maximum financing life is 50 years. The average financing period is 20 to 23 years. The interest rate is set in accordance with the TWDB rules in 31 TAC 363.33(a). The TWDB seeks to provide reasonable rates with minimal risk to the state. The TWDB post rates for comparison for applicants and in August 2010, the TWDB showed their rates for a 22-year, taxable loan at 7.07 percent where the market was at 8.47 percent.

The TWDB staff can discuss the terms of the loan and assist applicants during preparation of the application, and a preapplication conference is encouraged. The application materials must include an engineering feasibility report, environmental information, rates and customer base, operating budgets, financial statements, and project information. The board considers applications monthly.

Economically Distressed Areas Program

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The EDAP Program was designed to assist areas along the U.S./Mexico border in areas that were economically distressed. In 2008, this program was extended to apply to the entire state so long as requirements are met. This program provides financial assistance through the provision of grants and loans to communities where present facilities are inadequate to meet minimal residential needs. Eligible communities are those that have median household income less than 75 percent of the state household income. The applicant must be capable of maintaining and operating the completed system, and hold or be in the process of obtaining a Certificate of Convenience and Necessity. The county where the project is located must adopt model rules for the regulation of subdivisions prior to application for financial assistance. If the applicant is a city, the city must also adopt Model Subdivision Rules of TWDB (31 TAC Chapter 364). The program funds planning, design, construction, and acquisition. Up to 75 percent funding is available for facility plans with certain hardship cases 100 percent funding may be available. Projects must complete the planning, acquisition, and design phase before applying for second phase construction funds. The TWDB works with the applicant to find ways to leverage other state and federal financial resources. For grant fund above 50 percent, the Texas Department of State Health Services must determine if there is a health and safety nuisance.

The loan requires that the applicant pledge revenue or taxes, as well as some collateral for Mirando City PWS. The maximum financing life is 50 years. The average financing period is 20 to 23 years. The lending rate scale varies according to several factors but is set by the TWDB in accordance with the TWDB rules in 31 TAC 363.33(a). The TWDB seeks to provide reasonable rates with minimal loss to the state. The TWDB posts rates for comparison for applicants and in August 2010 the TWDB showed its rates for a 22-year, tax exempt loan at 5.05 percent where the market was at 6.05 percent. Most projects have a financial package with the majority of the project financed with grants. Many have received 100 percent grants.

The first step in the application process is to meet with TWDB staff to discuss the terms of the loan and assist applicants during preparation of the application. Major components of the application materials must include an engineering feasibility report, environmental information, rates and customer base, operating budgets, financial statements, community information, project information, and other legal information.

4.6.4.2 TDRA Funding Options

Created in 2001, TDRA seeks to strengthen rural communities and assist them with community and economic development and healthcare by providing a variety of rural programs, services, and activities. Of their many programs and funds, the most appropriate programs related to drinking water are the Community Development (CD) Fund and the Texas Small Towns Environment Program (STEP). These programs offer attractive funding packages to help make improvements to potable water systems to mitigate potential health concerns. These programs are available to counties and cities, which have to submit an TDRA application on

behalf of the PWS. All program requirements would have to be met by the benefiting community receiving services by the PWS.

Colonia Economically Distressed Areas Program

In the event a community, which is designated as economically distressed, receives TWDB funding through EDAP for water and sewer system improvement projects, it may be eligible to receive TDRA grants that can be used to connect households to the improved system. Funding may be used for connection fees, plumbing improvements, taps and meters, distribution lines, and other connection projects to a TWDB improvement project. Applications are submitted at the time an EDAP project construction begins and should work with CDBG staff to complete the application. In addition to CD Fund requirements, the community must be within 150 miles of the border and be designated a Colonia. These funds are submitted by the county on behalf of the Colonia and can be part of a project taken on by a nearby city to provide services to a nearby Colonia. Awards are given based on utilization of grant funds in a timely manner, past CDBG contract performance, availability of other resources, and effectiveness of funds to make connections to improve systems. Awards are on a "first-come, first serve" basis with a maximum of award of \$500,000.

Community Development Fund

The CD Fund is a competitive grant program for water system improvements as well as other utility services (wastewater, drainage improvements, and housing activities). Funds are distributed between 24 state planning regions where funds are allocated to address each region's utility priorities. Funds can be used for various types of public works projects, including water system improvements. Communities with a population of less than 50,000 that are not eligible for direct CDBG funding from the U.S. Department of Housing and Urban Development are eligible. Funds are awarded on a competitive basis decided twice a year in each region by local elected officials, appointed by the Governor using a defined scoring system (past performance with CDBG is a factor). Awards are no less than \$75,000 and cannot exceed \$800,000. More information can be found at the Office of Community Affairs website under Community Development Fund.

Texas Small Towns Environment Program

Under special occasions some communities are invited to participate in grant programs when self-help is a feasible method for completing a water project, the community is committed to self-help, and the community has the capacity to complete the project. The purpose is to significantly reduce the cost of the project by using the communities' own human, material, and financial capital. Communities with a population of less than 50,000 that are not eligible for direct CDBG funding from the U.S. Department of Housing and Urban Development are eligible. Projects typically are repair, rehabilitation, improvements, service connections, and yard services. Reasonable associated administration and engineering cost can be funded. A letter of interest is first submitted, community meetings are held, and after CDBG staff determines eligibility with a written invitation to apply, an application may be submitted. Awards are only given twice per year on a priority basis so long as the project can be fully funded (\$350,000 maximum award). Ranking criteria are project impact, local effort, past performance, percent of savings, and benefit to low to medium-income persons.

4.6.4.3 Rural Development

The Rural Utilities Service's (RUS) agency of Rural Development established Water and Waste Disposal Program for public entities administered by the staff of the Water and Environment Program (WEP) to assist communities with water and wastewater systems. The purpose is to fund technical assistance and projects to help communities bring safe drinking water and sanitary, environmentally sound, waste disposal facilities to rural Americans in greatest need.

The Water and Waste Disposal Program provides loans, grants, and loan guarantees for drinking water, sanitary sewer, solid waste, and storm drainage facilities in rural areas and cities and towns with a population of 10,000 people and rural areas with no population limits. Recipients must be public entities such as municipalities, counties, special purpose districts, Indian tribes, and non-profit corporations. RUS has set aside direct loans and grants for several areas (e.g., empowerment zones). Projects include all forms of infrastructure improvement, acquisition of land and water rights, and design fees. Funds are provided on a first come, first serve basis; however, staff do evaluate need and assign priorities as funds are limited. Grant/loan mixes vary on a case by case basis and some communities may have to wait though several funding cycles until funds become available.

Entities must demonstrate that they cannot obtain reasonable loans at market rates, but have the capacity to repay loans, pledge security, and operate the facilities. Grants can be up to 75 percent of the project costs, and loan guarantees can be up to 90 percent of eligible loss. Loans are not to exceed a 40-year repayment period, require tax or revenue pledges, and are offered at three rates:

- Poverty Rate The lowest rate is the poverty interest rate of 4.5 percent. Loans must be used to upgrade or construct new facilities to meet health standards, and the MHI in the service area must be below the poverty line for a family of four or below 80 percent of the statewide MHI for non-metropolitan communities.
- Market Rate Where the MHI in the service exceeds the state MHI, the rate is based on the average of the "Bond Buyer" 11-Bond Index over a four week period.
- Intermediate Rate the average of the Poverty Rate and the Market Rate, but not to exceed seven percent.

Water and Waste Disposal Grants and Loans (Section 306C for Colonias)

Grant funds at 100 percent are provided for areas along the US/Mexico border known as colonias. Projects must construct basic drinking water, sanitary sewer, solid waste disposal and storm drainage to serve residents of Colonias. Also, the systems can obtain funds to provide grant assistance directly to individuals to install necessary indoor plumbing and pay other costs of connecting to the system. Residents of the rural area to be served must face significant health risks due to the fact that a significant proportion of the community's residents do not

have access to or are not served by adequate, affordable water and/or waste disposal systems. Colonias is a term used to describe subdivisions that exist outside incorporated areas located along the United States-Mexico border. Colonias are generally characterized as small communities with inadequate drinking water, poor sanitary waste disposal facilities, and substandard housing. Aside from demonstrating health risk, areas not designated as a colonia must show that (1) per capita income of the residents is not more than 70 percent of the most recent national average per capita income, as determined by the Department of Commerce; and (2) unemployment rate of the residents is not less than 125 percent of the most recent national average unemployment rate, as determined by the Bureau of Labor Statistics. Project are ranked according to eligibility, a state preapplication review, RUS administrative review, population, income, other matching funds, colonia status, and natural disaster effect.

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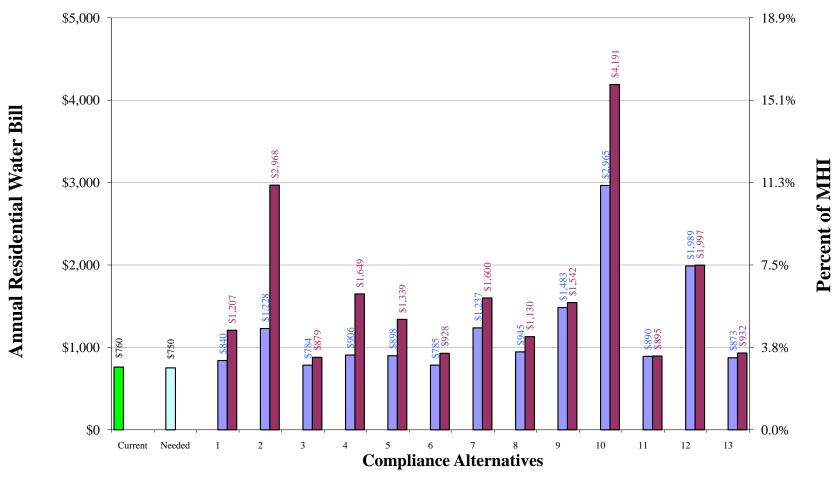
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Mirando City WSC

Table 4.4 Financial Impact on Households

| Alternative | Description | | All Revenue | 100% Grant | 75% Grant | 50% Grant | SRF | Bond |
|-------------|---|--|-------------|------------|-----------|-----------|---------|---------|
| | | Maximum % of MHI | 20.5% | 3.2% | 3.5% | 3.9% | 4.1% | 4.6% |
| 1 | Purchase Water from Oilton Rural | Percentage Rate Increase Compared to Current | 615% | 10% | 23% | 35% | 41% | 59% |
| | | Average Annual Water Bill | \$5,436 | \$840 | \$932 | \$1,023 | \$1,074 | \$1,207 |
| | | Maximum % of MHI | 86.8% | 4.6% | 6.3% | 7.9% | 8.8% | 11.2% |
| 2 | Purchase Water from Webb County | Percentage Rate Increase Compared to Current | 2924% | 62% | 119% | 176% | 208% | 290% |
| | | Average Annual Water Bill | \$22,989 | \$1,228 | \$1,663 | \$2,098 | \$2,340 | \$2,968 |
| | | Maximum % of MHI | 7.4% | 3.0% | 3.0% | 3.1% | 3.2% | 3.3% |
| 3 | New Well at Mirando | Percentage Rate Increase Compared to Current | 158% | 3% | 6% | 9% | 11% | 16% |
| | | Average Annual Water Bill | \$1,961 | \$784 | \$808 | \$831 | \$844 | \$879 |
| | | Maximum % of MHI | 38.6% | 3.4% | 4.1% | 4.8% | 5.2% | 6.2% |
| 4 | New Well at 10 Miles | Percentage Rate Increase Compared to Current | 1247% | 19% | 44% | 68% | 82% | 117% |
| | | Average Annual Water Bill | \$10,239 | \$906 | \$1,092 | \$1,278 | \$1,381 | \$1,649 |
| | | Maximum % of MHI | 24.1% | 3.4% | 3.8% | 4.2% | 4.5% | 5.1% |
| 5 | New Well at 5 Miles | Percentage Rate Increase Compared to Current | 740% | 18% | 33% | 47% | 55% | 76% |
| | | Average Annual Water Bill | \$6,388 | \$898 | \$1,008 | \$1,118 | \$1,180 | \$1,339 |
| | | Maximum % of MHI | 9.7% | 3.0% | 3.1% | 3.2% | 3.3% | 3.5% |
| 6 | New Well at 1 Mile | Percentage Rate Increase Compared to Current | 239% | 3% | 8% | 13% | 15% | 22% |
| | | Average Annual Water Bill | \$2,579 | \$785 | \$820 | \$856 | \$876 | \$928 |
| | | Maximum % of MHI | 20.3% | 4.7% | 5.0% | 5.4% | 5.5% | 6.0% |
| 7 | Central Treatment - RO | Percentage Rate Increase Compared to Current | 609% | 63% | 75% | 87% | 93% | 110% |
| | | Average Annual Water Bill | \$5,389 | \$1,237 | \$1,328 | \$1,419 | \$1,469 | \$1,600 |
| | | Maximum % of MHI | 11.8% | 3.6% | 3.7% | 3.9% | 4.0% | 4.3% |
| 8 | Central Treatment - Iron-Based Absorption | Percentage Rate Increase Compared to Current | 310% | 24% | 30% | 36% | 40% | 49% |
| | | Average Annual Water Bill | \$3,116 | \$945 | \$991 | \$1,037 | \$1,063 | \$1,130 |
| | | Maximum % of MHI | 5.7% | 5.6% | 5.7% | 5.7% | 5.7% | 5.8% |
| 9 | Point-of-Use Treatment | Percentage Rate Increase Compared to Current | 98% | 95% | 97% | 99% | 100% | 103% |
| | | Average Annual Water Bill | \$1,509 | \$1,483 | \$1,498 | \$1,513 | \$1,521 | \$1,542 |
| | | Maximum % of MHI | 62.0% | 11.2% | 12.3% | 13.5% | 14.1% | 15.8% |
| 10 | Point-of-Entry Treatment | Percentage Rate Increase Compared to Current | 2059% | 290% | 330% | 371% | 393% | 451% |
| | | Average Annual Water Bill | \$16,417 | \$2,965 | \$3,271 | \$3,578 | \$3,748 | \$4,191 |
| | | Maximum % of MHI | 3.4% | 3.4% | 3.4% | 3.4% | 3.4% | 3.4% |
| 11 | Public Dispenser for Treated Drinking Water | Percentage Rate Increase Compared to Current | 17% | 17% | 17% | 17% | 17% | 18% |
| | | Average Annual Water Bill | \$890 | \$890 | \$891 | \$893 | \$893 | \$895 |
| 12 | | Maximum % of MHI | 7.5% | 7.5% | 7.5% | 7.5% | 7.5% | 7.5% |
| | Supply Bottled Water to 100% of Population | Percentage Rate Increase Compared to Current | 162% | 162% | 162% | 162% | 162% | 163% |
| | | Average Annual Water Bill | \$1,989 | \$1,989 | \$1,991 | \$1,993 | \$1,994 | \$1,997 |
| | | Maximum % of MHI | 5.7% | 3.3% | 3.3% | 3.4% | 3.4% | 3.5% |
| 13 | Central Trucked Drinking Water - Oilton | Percentage Rate Increase Compared to Current | 98% | 15% | 17% | 19% | 20% | 23% |
| | | Average Annual Water Bill | \$1,508 | \$873 | \$888 | \$903 | \$911 | \$932 |

Figure 4.2 Alternative Cost Summary: Mirando City WSC



Current Average Monthly Bill = \$63.36 Median Household Income = \$26500 Average Monthly Residential Usage = 9733 gallons

 \blacksquare Current \blacksquare Needed \blacksquare With 100% Grant Funding \blacksquare With 100% Loan/Bond Funding

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5-1

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| 1 | APPENDIX A |
|---|--------------------|
| 2 | PWS INTERVIEW FORM |

CAPACITY DEVELOPMENT ASSESSMENT FORM

| Prepared By | |
|---|--------------------------------------|
| Section 1. Public Water System | Information |
| 1. PWS ID # 2. W | Vater System Name |
| 3. County | |
| 4. Owner | Address |
| Tele. | E-mail |
| Fax | Message |
| 5. Admin | Address |
| Tele. | E-mail |
| Fax | Message |
| 6. Operator | Address |
| Tele. | E-mail |
| Fax | Message |
| 7. Population Served | 8. No. of Service Connections |
| 9. Ownership Type | 10. Metered (Yes or No) |
| 11. Source Type | |
| 12. Total PWS Annual Water Used | |
| 13. Number of Water Quality Violations (Pri | ior 36 months) |
| Total Coliform | Chemical/Radiological |
| Monitoring (CCR, Public Notification | on, etc.) Treatment Technique, D/DBP |

A. Basic Information

Name of Water System:

7b. How long have you been certified?

Describe your water system related duties on a typical day.

1.

8.

| 2. | Name of Person Interviewed: |
|----|---|
| 3. | Position: |
| 4. | Number of years at job: |
| 5. | Number of years experience with drinking water systems: |
| 6. | Percent of time (day or week) on drinking water system activities, with current position (how much time is dedicated exclusively to the water system, not wastewater, solid waste or other activities): |
| 7. | Certified Water Operator (Yes or No): |
| | If Yes, 7a. Certification Level (water): |

B. Organization and Structure

1. Describe the organizational structure of the Utility. Please provide an organizational chart. (Looking to find out the governance structure (who reports to whom), whether or not there is a utility board, if the water system answers to public works or city council, etc.)

| 3. | Do all of the positions have a written job description? |
|----|---|
| | 3a. If yes, is it available to employees? |
| | 3b. May we see a copy? |
| | |
| | C. Personnel |
| 1. | What is the current staffing level (include all personnel who spend more than 10% of their time working on the water system)? |
| 2. | Are there any vacant positions? How long have the positions been vacant? |
| 2. | The there any vacant positions. How long have the positions been vacant. |
| 3. | In your opinion, is the current staffing level adequate? If not adequate, what are the issues or staffing needs (how many and what positions)? |
| 4. | What is the rate of employee turnover for management and operators? What are the major issues involved in the turnover (e.g., operator pay, working conditions, hours)? |
| 5. | Is the system staffed 24 hours a day? How is this handled (on-site or on-call)? Is there an alarm system to call an operator if an emergency occurs after hours? |
| | |

If not already covered in Question 1, to whom do you report?

2.

D. Communication

| 1. | Does the utility have a mission statement? If yes, what is it? |
|----|--|
| 2. | Does the utility have water quality goals? What are they? |
| 3. | How are your work priorities set? |
| 4. | How are work tasks delegated to staff? |
| 5. | Does the utility have regular staff meetings? How often? Who attends? |
| 6. | Are there separate management meetings? If so, describe. |
| 7. | Do management personnel ever visit the treatment facility? If yes, how often? |
| 8. | Is there effective communication between utility management and state regulators (e.g., NMED)? |
| 9. | Describe communication between utility and customers. |

E. Planning and Funding

| 1. | Describe the rate structure for the utility. |
|----|---|
| 2. | Is there a written rate structure, such as a rate ordinance? May we see it? |
| | 2a. What is the average rate for 6,000 gallons of water? |
| 3. | How often are the rates reviewed? |
| 4. | What process is used to set or revise the rates? |
| 5. | In general, how often are the new rates set? |
| 6. | Is there an operating budget for the water utility? Is it separate from other activities, such as wastewater, other utilities, or general city funds? |
| 7. | Who develops the budget, how is it developed and how often is a new budget created or the old budget updated? |
| 8. | How is the budget approved or adopted? |

| 9. | In the last 5 years, how many budget shortfalls have there been (i.e., didn't collect enough money to cover expenses)? What caused the shortfall (e.g., unpaid bills, an emergency repair, weather conditions)? |
|-----|---|
| | 9a. How are budget shortfalls handled? |
| 10. | In the last 5 years how many years have there been budget surpluses (i.e., collected revenues exceeded expenses? |
| | 10a. How are budget surpluses handled (i.e., what is done with the money)? |
| 11. | Does the utility have a line-item in the budget for emergencies or some kind of emergency reserve account? |
| 12. | How do you plan and pay for short-term system needs? |
| 13. | How do you plan and pay for long- term system needs? |
| 14. | How are major water system capital improvements funded? Does the utility have a written capital improvements plan? |
| 15. | How is the facility planning for future growth (either new hook-ups or expansion into new areas)? |
| 16. | Does the utility have and maintain an annual financial report? Is it presented to policy makers? |

| 17. | Has an independent financial audit been conducted of the utility finances? If so, how often? When was the last one? |
|-----|---|
| 18. | Will the system consider any type of regionalization with any other PWS, such as system interconnection, purchasing water, sharing operator, emergency water connection, sharing bookkeeper/billing or other? |
| | F. Policies, Procedures, and Programs |
| 1. | Are there written operational procedures? Do the employees use them? |
| 2. | Who in the utility department has spending authorization? What is the process for obtaining needed equipment or supplies, including who approves expenditures? |
| 3. | Does the utility have a source water protection program? What are the major components of the program? |
| 4. | Are managers and operators familiar with current SDWA regulations? |
| 5. | How do the managers and operators hear about new or proposed regulations, such as arsenic, DBP, Groundwater Rule? Are there any new regulations that will be of particular concern to the utility? |
| 6. | What are the typical customer complaints that the utility receives? |
| 7. | Approximately how many complaints are there per month? |

| 8. | How are customer complaints handled? Are they recorded? |
|-----|--|
| 9. | (If not specifically addressed in Question 7) If the complaint is of a water quality nature, how are these types of complaints handled? |
| 10. | Does the utility maintain an updated list of critical customers? |
| 11. | Is there a cross-connection control plan for the utility? Is it written? Who enforces the plan's requirements? |
| 12. | Does the utility have a written water conservation plan? |
| 13. | Has there been a water audit of the system? If yes, what were the results? |
| 14. | (If not specifically answered in 11 above) What is the estimated percentage for loss to leakage for the system? |
| 15. | Are you, or is the utility itself, a member of any trade organizations, such as AWWA or Rural Water Association? Are you an active member (i.e., attend regular meetings or participate in a leadership role)? Do you find this membership helpful? If yes, in what ways does it help you? |

G. Operations and Maintenance

1.

How is decision-making authority split between operations and management for the following items:

| | a. | Process Control |
|----|-----------------|--|
| | b. | Purchases of supplies or small equipment |
| | c. | Compliance sampling/reporting |
| | d. | Staff scheduling |
| 2. | Describe your | utility's preventative maintenance program. |
| 3. | Do the operate | ors have the ability to make changes or modify the preventative maintenance program? |
| 4. | | nagement prioritize the repair or replacement of utility assets? Do the operators play a role zation process? |
| 5. | Does the utilit | y keep an inventory of spare parts? |
| 6. | Where does st | aff have to go to buy supplies/minor equipment? How often? |
| | examp | w do you handle supplies that are critical, but not in close proximity (for le if chlorine is not available in the immediate area or if the components for a critical are not in the area) |

| 7. | Describe the system's disinfection process. Have you had any problems in the last few years with the disinfection system? |
|--------|--|
| | 7a. Who has the ability to adjust the disinfection process? |
| 8. Hov | w often is the disinfectant residual checked and where is it checked? |
| | 8a. Is there an official policy on checking residuals or is it up to the operators? |
| 9. | Does the utility have an O & M manual? Does the staff use it? |
| 10. | Are the operators trained on safety issues? How are they trained and how often? |
| 11. | Describe how on-going training is handled for operators and other staff. How do you hear about appropriate trainings? Who suggests the trainings – the managers or the operators? How often do operators, managers, or other staff go to training? Who are the typical trainers used and where are the trainings usually held? |
| 12. | In your opinion is the level of your on-going training adequate? |
| 13. | In your opinion is the level of on-going training for other staff members, particularly the operators, adequate? |

| 14. | Does the facility have mapping of the water utility components? Is it used on any routine basis by the operators or management? If so, how is it used? If not, what is the process used for locating utility components? |
|-----|--|
| 15. | In the last sanitary survey, were any deficiencies noted? If yes, were they corrected? |
| 16. | How often are storage tanks inspected? Who does the inspection? |
| | 16a. Have you experienced any problems with the storage tanks? |
| | H. SDWA Compliance |
| 1. | Has the system had any violations (monitoring or MCL) in the past 3 years? If so, describe. |
| 2. | How were the violations handled? |
| 3. | Does the system properly publish public notifications when notified of a violation? |
| 4. | Is the system currently in violation of any SDWA or state regulatory requirements, including failure to pay fees, fines, or other administrative type requirements? |
| 5. | Does the utility prepare and distribute a Consumer Confidence Report (CCR)? Is it done every year? What type of response does the utility get to the CCR from customers? |

I. Emergency Planning

| 1. | Does the system have a written emergency plan to handle emergencies such as water outages, weather issues, loss of power, loss of major equipment, etc? |
|----|---|
| 2. | When was the last time the plan was updated? |
| | |

Describe the last emergency the facility faced and how it was handled.

Do all employees know where the plan is? Do they follow it?

3.

4.

Attachment A

A. Technical Capacity Assessment Questions

| 1. | Based on available information of water rights on record and water pumped has the system exceeded its rights in the past year? YES NO | wate |
|----|--|------|
| | In any of the past 5 years? YES NO How many times? | |
| 2. | Does the system have the proper level of certified operator? (Use questions $a - c$ to answer.) YES \square NO \square | |
| | a. What is the Classification Level of the system by NMED? | |
| | b. Does the system have one or more certified operator(s)? [20 NMAC 7.4.20] | |
| | YES NO | |
| | c. If YES, provide the number of operators at each New Mexico Certification Level. [20 NMAC 7.4.12] | |
| | NM Small SystemClass 2 | |
| | NM Small System AdvancedClass 3 | |
| | Class 1Class 4 | |
| 3. | Did the system correct any sanitary deficiency noted on the most recent sanitary survey within 6 months | of |
| | receiving that information? [20 NMAC 7.20.504] | |
| | YES NO No Deficiencies | |
| | What was the type of deficiency? (Check all that are applicable.) | |
| | Source Storage | |
| | Treatment Distribution | |
| | Other | |
| | From the system's perspective, were there any other deficiencies that were not noted on the sanitary surv | ey? |
| | Please describe. | |
| 4. | Will the system's current treatment process meet known future regulations? | |
| | Radionuclides YES NO Doesn't Apply | |
| | Arsenic YES NO Doesn't Apply | |
| | Stage 1 Disinfectants and Disinfection By-Product (DBP) | |
| | YES NO Doesn't Apply | |
| | Surface Water Treatment Rule YES NO Doesn't Apply | |
| 5. | Does the system have a current site plan/map? [20 NMAC 7.10.302 A.1.] | |
| | YES NO | |

| o. Has | s me systen | i nau a wate | r suppry outag | e in the prior 24 month | is: | |
|---------------|---------------------|---------------|-----------------------|----------------------------|------------|----------------------------|
| | YES | | NO | | | |
| | What | were the cau | ses of the outa | age(s)? (Include numbe | er of outo | ages for each cause.) |
| | Droug | ht | | Limited Supply | - | |
| | Syster | n Failure | | Other | - | |
| | | | | | | |
| 7. Has | s the systen | n ever had a | water audit or | a leak evaluation? | | |
| | YES | | NO | ☐ Don't Know | | |
| | If YES | S, please con | nplete the follo | owing table. | | |
| Type of | | Date | Water Loss | What approach or | | Was any follow-up done? If |
| Investigation | on | Done | (%) | technology was used | to | so, describe |
| | | | | complete the investig | ation? | |
| | | | | | | |
| | | | | | | |
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| | | | | | | |
| 0 11 | 11 1 . 1 | | • , • | 1111000 | 10 | [20 ND 4 C 7 10 201] |
| 8. Hav | ve all drink YES | ing water pro | ojects received NO | l NMED review and ap | oproval? | [20 NMAC 7.10.201] |
| | If NO | , what types | of projects hav | ve not received NMED | review a | and approval. |
| | Source | e [|] | Storage | | |
| | Treatn | nent |] | Distribution | | |
| | Other | |] | | | |
| | | | | | | |
| 9. Wh | at are the t | ypical custor | ner complaint | s that the utility receive | es? | |
| | | | | | | |
| | | | | | | |
| 10. Apj | proximately | y how many | complaints are | e there per month? | | |
| 11. Ho | w are custo | mer complai | ints handled? | Are they recorded? | | |
| | | | | | | |

| | Pipe Material | Approximate Age | Percentage of the system | Comments |
|----|------------------|---------------------|---|--|
| | | 1.28 | | Sanitary Survey Distribution System Record Attached |
| | | | | |
| | | | | |
| | Are there any d | ead end lines in t | he system? NO | |
| | Does the system | n have a flushing | program? | |
| | | YES | NO | |
| | If YES, please | describe. | | |
| | Are there any p | ressure problems | within the system? | |
| | | YES | NO | |
| | If YES, please | describe. | | |
| | Does the system | n disinfect the fin | nished water? | |
| | | YES | NO | |
| | If yes, which di | sinfectant produc | et is used? | |
| ev | wer Comments on | Technical Capac | city: | |
| | | | sment Questions ear Infrastructure Capital Imp | rovement Plan (ICIP) plan? |
| | YES | | NO 🗌 | |
| | If YES, has the | plan been submi | tted to Local Government Di | vision? |
| | YES | | NO 🗌 | |
| | Does the system | m have written oj | perating procedures? | |
| | | | | |
| | YES | | NO | |
| | | | NO b descriptions for all staff? | |

| 20. | Does the system have: |
|-----|---|
| | A preventative maintenance plan? YES NO |
| | A source water protection plan? YES NO N/A |
| | An emergency plan? |
| | YES NO A cross-connection control program? |
| | YES NO |
| | An emergency source? |
| | YES NO |
| | System security measures? |
| | YES L NO L |
| 21. | Does the system report and maintain records in accordance with the drinking water regulations concerning: Water quality violations |
| | YES NO |
| | Public notification YES NO |
| | Sampling exemptions YES NO |
| 22. | Please describe how the above records are maintained: |
| 23. | Describe the management structure for the water system, including board and operations staff. Please include examples of duties, if possible. |
| 24. | Please describe type and quantity of training or continuing education for staff identified above. |
| 25. | Describe last major project undertaken by the water system, including the following: project in detail, positive aspects, negative aspects, the way in which the project was funded, any necessary rate increases, the public response to the project, whether the project is complete or not, and any other pertinent information. |

| 26. | Does the system have any debt? YES NO |
|-------|--|
| | If yes, is the system current with all debt payments? YES NO |
| | If no, describe the applicable funding agency and the default. |
| 27. | Is the system currently contemplating or actively seeking funding for any project? YES NO |
| | If yes, from which agency and how much? |
| | Describe the project? |
| | Is the system receiving assistance from any agency or organization in its efforts? |
| 28. | Will the system consider any type of regionalization with other PWS? (Check YES if the system has already regionalized.) |
| | YES NO |
| | If YES, what type of regionalization has been implemented/considered/discussed? (Check all that apply.) |
| | System interconnection |
| | Sharing operator |
| | Sharing bookkeeper |
| | Purchasing water |
| | Emergency water connection |
| | Other: |
| 29. | Does the system have any of the following? (Check all that apply.) |
| | Water Conservation Policy/Ordinance Current Drought Plan |
| | Water Use Restrictions |
| Inter | rviewer Comments on Managerial Capacity: |
| | |
| | |
| | |
| | |
| | |
| | |

| <u>C.</u> | Financial Capacity Assessment |
|-----------|--|
| 30. | Does the system have a budget? |
| | YES NO |
| | If YES, what type of budget? |
| | Operating Budget |
| | Capital Budget |
| 31. | Have the system revenues covered expenses and debt service for the past 5 years? |
| | YES NO |
| | If NO, how many years has the system had a shortfall? |
| 32. | Does the system have a written/adopted rate structure? |
| | YES NO |
| 33. | What was the date of the last rate increase? |
| 34. | Are rates reviewed annually? |
| | YES NO |
| | IF YES, what was the date of the last review? |
| 35. | Did the rate review show that the rates covered the following expenses? (Check all that apply.) |
| | Operation & Maintenance |
| | Infrastructure Repair & replacement |
| | Staffing |
| | Emergency/Reserve fund |
| | Debt payment |
| 36. | Is the rate collection above 90% of the customers? |
| | YES NO |
| 37. | Is there a cut-off policy for customers who are in arrears with their bill or for illegal connections? |
| | YES NO |
| | If yes, is this policy implemented? |
| 38. | What is the residential water rate for 6,000 gallons of usage in one month. |
| 39. | In the past 12 months, how many customers have had accounts frozen or dropped for non-payment? |
| | [Convert to % of active connections |
| | Less than 1% |
| | 11% - 20% |

| 40. | The following questions refer to the process of obtaining needed equipment and supplies. |
|-----|--|
| | a. Can the water system operator buy or obtain supplies or equipment when they are needed? |
| | YES NO |
| | b. Is the process simple or burdensome to the employees? |
| | c. Can supplies or equipment be obtained quickly during an emergency? |
| | YES NO |
| | d. Has the water system operator ever experienced a situation in which he/she couldn't purchase the needed supplies? |
| | YES NO |
| | e. Does the system maintain some type of spare parts inventory? |
| | YES NO |
| | If yes, please describe. |
| | ii yes, pieuse describe. |
| 41. | Has the system ever had a financial audit? YES NO I If YES, what is the date of the most recent audit? |
| 42. | Has the system ever had its electricity or phone turned off due to non-payment? Please describe. |
| | |
| | |
| | |
| | |
| In | nterviewer Comments on Financial Assessment: |
| | |
| | |
| | |
| | |
| | |
| | |

| 43. | What do you think the system capabilities are now and what are the issues you feel your system will be |
|-----|---|
| | facing in the future? In addition, are there any specific needs, such as types of training that you would |
| | like to see addressed by NMED or its contractors? |

1 APPENDIX B 2 COST BASIS

This section presents the basis for unit costs used to develop the conceptual cost estimates for the compliance alternatives. Cost estimates are conceptual in nature (+50%/-30%), and are intended to make comparisons between compliance options and to provide a preliminary indication of possible rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation. Capital cost includes an allowance for engineering and construction management. It is assumed that adequate electrical power is available near the site. The cost estimates specifically do not include costs for the following:

- Obtaining land or easements.
- Surveying.

- Mobilization/demobilization for construction.
- Insurance and bonds

In general, unit costs are based on recent construction bids for similar work in the area; when possible, consultations with vendors or other suppliers; published construction and O&M cost data; and USEPA cost guidance. Unit costs used for the cost estimates are summarized in Table B.1.

Unit costs for pipeline components are based on 2009 RS Means Site Work & Landscape Cost Data. The number of borings and encasements and open cuts and encasements is estimated by counting the road, highway, railroad, stream, and river crossings for a conceptual routing of the pipeline. The number of air release valves is estimated by examining the land surface profile along the conceptual pipeline route. It is assumed that gate valves and flush valves would be installed, on average, every 5,000 feet along the pipeline. Pipeline cost estimates are based on the use of C-900 PVC pipe. Other pipe materials could be considered for more detailed development of attractive alternatives.

Pump station unit costs are based on experience with similar installations. The cost estimate for the pump stations include two pumps, station piping and valves, station electrical and instrumentation, minor site improvement, installation of a concrete pad, fence and building, and tools. The number of pump stations is based on calculations of pressure losses in the proposed pipeline for each alternative. Back-flow prevention is required in cases where pressure losses are negligible, and pump stations are not needed. Construction cost of a storage tank is based on consultations with vendors and 2007 RS Means Site Work & Landscape Cost Data.

Labor costs are estimated based on 2009 RS Means Site Work & Landscape Cost Data specific to the Lubbock County region.

Electrical power cost is estimated to be \$0.072 per kWH, as supplied by the General Manager and Operator of the Mirando City Water Supply Corporation. The annual cost for power to a pump station is calculated based on the pumping head and volume, and includes 11,800 kWH for pump building heating, cooling, and lighting, as recommended in USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992).

In addition to the cost of electricity, pump stations have other maintenance costs. These costs cover: materials for minor repairs to keep the pumps operating; purchase of a maintenance vehicle, fuel costs, and vehicle maintenance costs; utilities; office supplies, small tools and equipment; and miscellaneous materials such as safety, clothing, chemicals, and paint. The non-power O&M costs are estimated based on the USEPA publication, *Standardized Costs for Water Supply Distribution Systems* (1992), which provides cost curves for O&M components. Costs from the 1992 report are adjusted to 2010 dollars based on the ENR construction cost index.

Pipeline maintenance costs include routine cleaning and flushing, as well as minor repairs to lines. The unit rate for pipeline maintenance is calculated based on the USEPA technical report, *Innovative and Alternate Technology Assessment Manual MCD 53* (1978). Costs from the 1978 report are adjusted to 2010 dollars based on the ENR construction cost index.

Storage tank maintenance costs include cleaning and renewal of interior lining and exterior coating. Unit costs for storage tank O&M are based on USEPA publication *Standardized Costs for Water Supply Distribution Systems* (1992). Costs from the 1992 report are adjusted to 2010 dollars based on the ENR construction cost index.

The purchase price for point-of-use (POU) water treatment units is based on vendor price lists for treatment units, plus installation. O&M costs for POU treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

The purchase price for point-of-entry (POE) water treatment units is based on vendor price lists for treatment units, plus an allowance for installation, including a concrete pad and shed, piping modifications, and electrical connection. O&M costs for POE treatment units are also based on vendor price lists. It is assumed that a yearly water sample would be analyzed for the contaminant of concern.

Central treatment plant costs, for both adsorption and coagulation/filtration, include pricing for buildings, utilities, and site work. Costs are based on pricing given in the various R.S. Means Construction Cost Data References, as well as prices obtained from similar work on other projects. Pricing for treatment equipment was obtained from vendors.

Well installation costs are based on quotations from drillers for installation of similar depth wells in the area. Well installation costs include drilling, a well pump, electrical and instrumentation installation, well finishing, piping, and water quality testing. O&M costs for water wells include power, materials, and labor. It is assumed that new wells located more than

1 mile from the intake point of an existing system would require a storage tank and pump station.

Purchase price for the treatment unit dispenser is based on vendor price lists, plus an allowance for installation at a centralized public location. The O&M costs are also based on vendor price lists. It is assumed that weekly water samples would be analyzed for the contaminant of concern.

Costs for bottled water delivery alternatives are based on consultation with vendors that deliver residential bottled water. The cost estimate includes an initial allowance for set-up of the program, and a yearly allowance for program administration.

The cost estimate for a public dispenser for trucked water includes the purchase price for a water truck and construction of a storage tank. Annual costs include labor for purchasing the water, picking up and delivering the water, truck maintenance, and water sampling and testing. It is assumed the water truck would be required to make one trip each week, and that chlorine residual would be determined for each truck load.

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Table B.1 **Summary of General Data** Mirando City WSC 2400025

General PWS Information

Service Population 500
Total PWS Daily Water Usage 0.08 (mgd)

Number of Connections 250

| Total PWS Daily Water Usage | 0.08 (mgd) |) | | | | | |
|---|--------------------|----------|-----------------|---|--------------|----------|-------------------|
| | | | Unit Co | ost Data | | | |
| General Items | Unit | u | Init Cost | Central Treatment Unit Costs | Unit | U | nit Cost |
| Treated water purchase cost | | alterna | ative | General | | | |
| Water purchase cost (trucked) | \$/1,000 gal | s \$ | 2.97 | Site preparation | acre | \$ | 4,000 |
| | | | , | Slab | CY | \$ | 1,000 |
| Contingency | 20% | | n/a | Building | SF | \$ | 60 |
| Engineering & Constr. Management Procurement/admin (POU/POE) | 25% 20% | | n/a n/a | Building electrical | SF SF | \$ \$ | 8.00 8.00 |
| Procurement/admin (POO/POE) | 20% | | n/a | Building plumbing Heating and ventilation | SF SF | э \$ | 7.00 |
| Pipeline Unit Costs | Unit | u | Init Cost | Fence | LF | \$ | 15 |
| PVC water line, Class 200, 06" | LF | \$ | 21 | Paving | SF | \$ | 2.00 |
| Bore and encasement, 10" | LF | \$ | 235 | Chlorination point | EA | \$ | 4,000 |
| Open cut and encasement, 10" | LF | \$ | 127 | | | | |
| Gate valve and box, 06" | EA | \$ | 1,125 | Building power | kwh/yr | \$ | 0.072 |
| Air valve | EA | \$ | 2,079 | Equipment power | kwh/yr | \$ | 0.072 |
| Flush valve | EA LF | \$ \$ | 1,700 | Labor, O&M | hr | \$ \$ | 40 200 |
| Metal detectable tape | LF | Ф | 0.05 | Analyses | test | Ф | 200 |
| Bore and encasement, length | Feet | | 200 | Adsorption | | | |
| Open cut and encasement, length | Feet | | 50 | Electrical | JOB | \$ | 80,000 |
| , , | | | | Piping | JOB | \$ | 50,000 |
| Pump Station Unit Costs | Unit | U | Init Cost | Adsorption package plant | UNIT | \$ | 127,162 |
| Pump | EA | \$ | 8,230 | Backwash tank | GAL | \$ | 2.00 |
| Pump Station Piping, 06" | EA | \$ | 817 | Sewer connection fee | EA | \$ | 15,000 |
| Gate valve, 06" | EA | \$ | 1,125 | | | _ | |
| Check valve, 06" | EA | \$ | 1,223 | Spent media disposal | CY | \$ | 20 |
| Electrical/Instrumentation | EΑ | \$ | 10,550 | Adsorption materials replacement | cft | \$ | 80.00 |
| Site work | EA EA | \$ \$ | 2,635 5,275 | Backwash discharge to sewer | MG/year | \$ | 5,000 |
| Building pad Pump Building | EA | \$ | 10,550 | Prefilter for RO | | | |
| Fence | EA | \$ | 6,330 | Electrical | JOB | \$ | 5,000 |
| Tools | EA | \$ | 1,055 | Piping | JOB | \$ | 5,000 |
| 5,000 gal feed tank | EA | \$ | 12,487 | Backwash tank | GAL | \$ | 2.00 |
| Backflow preventer, 6" | EA | \$ | 4,059 | Coagulation/Filtration Materials | year | \$ | 8,000 |
| Backflow Testing/Certification | EA | \$ | 110 | Chemicals, Coagulation | year | \$ | 2,000 |
| | | | | Backwash discharge to sewer | MG/year | \$ | 5,000 |
| Well Installation Unit Costs | Unit | | Init Cost | D | | | |
| Well installation | | alterna | | Reverse Osmosis | IOD | • | 400.000 |
| Water quality testing 5HP Well Pump | EA EA | \$ \$ | 1,320 4,132 | Electrical Piping | JOB JOB | \$ \$ | 100,000 50,000 |
| Well electrical/instrumentation | EA | \$ | 5,800 | RO package plant | UNIT | \$ | 313,000 |
| Well cover and base | EA | \$ | 3,165 | Transfer pumps (5 hp) | EA | \$ | 5,000 |
| Piping | EA | \$ | 3,165 | Permeate tank | gal | \$ | 3 |
| 50,000 gal ground storage tank | EA | \$ | 101,655 | RO materials and chemicals | kgal | \$ | 0.43 |
| | | | | RO chemicals | year | \$ | 2,000 |
| Electrical Power | \$/kWH | \$ | 0.07181 | Backwash disposal mileage cost | miles | \$ | 1.50 |
| Building Power | kWH | | 11,800 | Backwash disposal fee | 1,000 gal/yr | \$ | 5.00 |
| Labor | \$/hr | \$ | 60 | Acatago | | • | 50 |
| Materials Transmission main O&M | EA \$/mile | \$ \$ | 1,585 | Analyses | test | \$ | 50 |
| Tank O&M | φ/mile EA | Ф \$ | 285 1,055 | Reject Pond | | | |
| TAIK OXIVI | EA | Φ | 1,055 | Reject pond, excavation | CYD | \$ | 3 |
| POU/POE Unit Costs | | | | Reject pond, compacted fill | CYD | \$ | 4 |
| POU treatment unit purchase | EA | \$ | 300 | Reject pond, lining | SF | \$ | 0.50 |
| POU treatment unit installation | EA | \$ | 160 | Reject pond, vegetation | SY | \$ | 1.50 |
| POE treatment unit purchase | EA | \$ | 5,275 | Reject pond, access road | LF | \$ | 30 |
| POE - pad and shed, per unit | EA | \$ | 2,110 | Reject water haulage truck | EA | \$ | 100,000 |
| POE - piping connection, per unit | EA | \$ | 1,055 | | | | |
| POE - electrical hook-up, per unit | EA | \$ | 1,055 | | | | |
| DOLL Treatment ORM per unit | ¢/,,,,,,, | ď | 102 | | | | |
| POU Treatment O&M, per unit POE Treatment O&M, per unit | \$/year \$/year | \$ \$ | 103 1,585 | | | | |
| Treatment analysis | \$/year | φ \$ | 210 | | | | |
| POU/POE labor support | \$/hr | \$ | 42 | | | | |
| . Con CI labor capport | Ψ/ | Ψ | | | | | |
| Dispenser/Bottled Water Unit Cost | s | | | | | | |
| POE-Treatment unit purchase | EA | \$ | 7,385 | | | | |
| POE-Treatment unit installation | EA | \$ | 5,275 | | | | |
| Treatment unit O&M | EA | \$ | 2,100 | | | | |
| Administrative labor | hr | \$ | 46 | | | | |
| Bottled water cost (inc. delivery) | gallon | \$ | 1.55 | | | | |
| Water use, per capita per day | gpcd | ď | 1.0 | | | | |
| Bottled water program materials 5,000 gal ground storage tank | EA EA | \$ \$ | 5,275 12.487 | | | | |
| Site improvements | EA | \$ | 12,487 3,165 | | | | |
| Potable water truck | EA | | 15,000.00 | | | | |
| Water analysis, per sample | EA | \$ | 210 | | | | |
| Potable water truck O&M costs | \$/mile | \$ | 1.50 | | | | |
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APPENDIX C COMPLIANCE ALTERNATIVE CONCEPTUAL COST ESTIMATES

This appendix presents the conceptual cost estimates developed for the compliance alternatives. The conceptual cost estimates are given in Tables C.1 through C.13. The cost estimates are conceptual in nature (+50%/-30%), and are intended for making comparisons between compliance options and to provide a preliminary indication of possible water rate impacts. Consequently, these costs are pre-planning level and should not be viewed as final estimated costs for alternative implementation.

C-1

PWS Name Mirando City WSC

Alternative Name Purchase Water from Oilton Rural

Alternative Number MC-1

 Distance from Alternative to PWS (along pipe)
 4.8 miles

 Total PWS annual water usage
 29.200 MG

 Treated water purchase cost
 \$ 1.01 per 1,000 gals

 Pump Stations needed w/ 1 feed tank each
 1

Pump Stations needed w/ 1 feed tank each 1
On site storage tanks / pump sets needed 0

Capital Costs

| Cost Item Pipeline Construction | Quantity | Unit | Un | it Cost | Т | otal Cost | Cost Item Pipeline O&M | Quantity | Unit | Unit | Cost | То | tal Cost |
|---------------------------------|-----------|---------|-----|---------|-----|-----------|-------------------------|----------|-----------|------|-------|----|----------|
| Number of Crossings, bore | 2 | n/a | n/a | | n/a | | Pipeline O&M | 4.8 | mile | \$ | 285 | \$ | 1,373 |
| Number of Crossings, open cut | | n/a | n/a | | n/a | | Subtotal | | | • | | \$ | 1,373 |
| PVC water line, Class 200, 06" | 25,434 | | \$ | 21 | \$ | 525,910 | | | | | | • | 1,010 |
| Bore and encasement, 10" | 400 | LF | \$ | 235 | \$ | 93,888 | Water Purchase Cost | | | | | | |
| Open cut and encasement, 10" | 700 | LF | \$ | 127 | \$ | 88,998 | From PWS | 29.200 | 1,000 gal | \$ | 1.01 | \$ | 29,530 |
| Gate valve and box, 06" | 5 | EA | \$ | 1,125 | \$ | 5,721 | Subtotal | -, | , 3 | • | | \$ | 29,530 |
| Air valve | 5 | EA | \$ | 2,079 | \$ | 10,395 | | | | | | • | , |
| Flush valve | 5 | EA | \$ | 1,700 | \$ | 8,648 | | | | | | | |
| Metal detectable tape | 25,434 | LF | \$ | 0 | \$ | 1,272 | | | | | | | |
| Subtotal | | | · | | \$ | 734,831 | | | | | | | |
| Pump Station(s) Installation | | | | | | | Pump Station(s) O&M | | | | | | |
| Pump | 2 | EA | \$ | 8,230 | \$ | 16,460 | Building Power | 11,800 | kWH | \$ | 0.072 | \$ | 847 |
| Pump Station Piping, 06" | 1 | EA | \$ | 817 | \$ | 817 | Pump Power | 16,174 | kWH | \$ | 0.072 | \$ | 1,161 |
| Gate valve, 06" | 4 | EΑ | \$ | 1,125 | \$ | 4,499 | Materials | 1 | EA | \$ | 1,585 | \$ | 1,585 |
| Check valve, 06" | 2 | EA | \$ | 1,223 | \$ | 2,445 | Labor | 365 | Hrs | \$ | 60.00 | \$ | 21,900 |
| Electrical/Instrumentation | 1 | EA | \$ | 10,550 | \$ | 10,550 | Tank O&M | - | EA | \$ | 1,055 | \$ | - |
| Site work | 1 | EA | \$ | 2,635 | \$ | 2,635 | Backflow Test/Cert | - | EA | \$ | 110 | \$ | - |
| Building pad | 1 | | \$ | 5,275 | \$ | 5,275 | Subtotal | | | | | \$ | 25,494 |
| Pump Building | 1 | EA | \$ | 10,550 | \$ | 10,550 | | | | | | | |
| Fence | 1 | | \$ | 6,330 | \$ | 6,330 | | | | | | | |
| Tools | 1 | EA | \$ | 1,055 | \$ | 1,055 | | | | | | | |
| 5,000 gal feed tank | 1 | EA | \$ | 12,487 | \$ | 12,487 | | | | | | | |
| 50,000 gal ground storage tank | - | EA | \$ | 101,655 | \$ | - | | | | | | | |
| Backflow Preventor | - | EA | \$ | 4,059 | | - | | | | | | | |
| Subtotal | | | | | \$ | 73,102 | | | | | | | |
| | | | | | | | O&M Credit for Existing | | | | | | |
| | | | | | | | Pump power | 126,680 | | \$ | 0.072 | | (9,097) |
| | | | | | | | Well O&M matl | | EA | \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | 360 | Hrs | \$ | 60.00 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of | Compone | nt Cost | s | | \$ | 807,934 | | | | | | | |
| Contingency | 20% |) | | | \$ | 161,587 | | | | | | | |
| Design & Constr Management | 25% |) | | | \$ | 201,983 | | | | | | | |
| TOTAL | . CAPITAL | COST | s | | \$ | 1,171,504 | TOTAL A | NNUAL O | M COSTS | 6 | | \$ | 22,530 |

PWS Name Mirando City WSC

Alternative Name Purchase Water from Webb County

Alternative Number MC-2

Distance from Alternative to PWS (along pipe) 27.0 miles
Total PWS annual water usage 29.200 MG
Treated water purchase cost \$ 2.90 per 1,000 gals
Pump Stations needed w/ 1 feed tank each 2

Pump Stations needed w/ 1 feed tank each
On site storage tanks / pump sets needed
0

Capital Costs

| Cost Item Pipeline Construction | Quantity | Unit | Un | it Cost | T | otal Cost | Cost Item Pipeline O&M | Quantity | Unit | Unit | Cost | То | tal Cost |
|---------------------------------------|-----------|---------|------------|-----------|------------|-----------|-------------------------|----------|-----------|------|-------|---------|-----------------------|
| Number of Crossings, bore | 0 | n/a | n/a | | n/a | | Pipeline O&M | 27.0 | mile | \$ | 285 | \$ | 7,701 |
| Number of Crossings, open cut | | n/a | n/a | | n/a | | Subtotal | | IIIIIE | Φ | 200 | φ \$ | 7,701 7,701 |
| PVC water line, Class 200, 06" | 142,674 | | 11/a \$ | | 11/a \$ | 2,950,132 | Subtotai | | | | | Ф | 7,701 |
| Bore and encasement, 10" | 1,800 | | Ф \$ | 21 235 | \$ | | Water Purchase Cost | | | | | | |
| · · · · · · · · · · · · · · · · · · · | , | | | | | 422,496 | | 00.000 | 4 000 | • | 0.00 | • | 04.000 |
| Open cut and encasement, 10" | 1,350 | | \$ | 127 | \$ | 171,639 | From PWS | | 1,000 gal | Ф | 2.90 | \$ | 84,680 |
| Gate valve and box, 06" | | EA | \$ | 1,125 | \$ | 32,093 | Subtotal | | | | | \$ | 84,680 |
| Air valve | | EA | \$ | 2,079 | \$ | 56,133 | | | | | | | |
| Flush valve | | EA | \$ | 1,700 | \$ | 48,509 | | | | | | | |
| Metal detectable tape | 142,674 | LF | \$ | 0 | \$ | 7,134 | | | | | | | |
| Subtotal | | | | | \$ | 3,688,136 | | | | | | | |
| Pump Station(s) Installation | | | | | | | Pump Station(s) O&M | | | | | | |
| Pump | 4 | EA | \$ | 8,230 | \$ | 32,920 | Building Power | 23,600 | kWH | \$ | 0.072 | \$ | 1,695 |
| Pump Station Piping, 06" | 2 | EA | \$ | 817 | \$ | 1,633 | Pump Power | 142,066 | kWH | \$ | 0.072 | \$ | 10,202 |
| Gate valve, 06" | 8 | EA | \$ | 1,125 | \$ | 8,998 | Materials | 2 | EA | \$ | 1,585 | \$ | 3,170 |
| Check valve, 06" | 4 | EA | \$ | 1,223 | \$ | 4,890 | Labor | 730 | Hrs | \$ | 60.00 | \$ | 43,800 |
| Electrical/Instrumentation | 2 | EA | \$ | 10,550 | \$ | 21,100 | Tank O&M | 2 | EA | \$ | 1,055 | \$ | 2,110 |
| Site work | 2 | EA | \$ | 2,635 | \$ | 5,270 | Backflow Test/Cert | t 0 | EA | \$ | 110 | \$ | ´- |
| Building pad | 2 | EA | \$ | 5,275 | \$ | 10,550 | Subtotal | | | | | \$ | 60,976 |
| Pump Building | 2 | EA | \$ | 10,550 | \$ | 21,100 | | | | | | • | , |
| Fence | 2 | EA | \$ | 6,330 | \$ | 12,660 | | | | | | | |
| Tools | 2 | EA | \$ | 1.055 | \$ | 2.110 | | | | | | | |
| 5,000 gal feed tank | 2 | EA | \$ | 12,487 | \$ | 24,974 | | | | | | | |
| 50,000 gal ground storage tank | | EA | - | 101,655 | \$ | , | | | | | | | |
| Backflow Preventor | _ | EA | \$ | 4,059 | \$ | _ | | | | | | | |
| Subtotal | | | Ψ | 1,000 | \$ | 146,205 | | | | | | | |
| Gustotal | | | | | ٠ | 140,200 | | | | | | | |
| | | | | | | | O&M Credit for Existing | | | | | | |
| | | | | | | | Pump power | 126,680 | | \$ | 0.072 | \$ | (9,097) |
| | | | | | | | Well O&M matl | | EA | \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | | Hrs | \$ | 60 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of (| Compone | nt Cost | s | | \$ | 3,834,341 | | | | | | | |
| Contingency | 20% | , | | | \$ | 766,868 | | | | | | | |
| Design & Constr Management | 25% | • | | | \$ | 958,585 | | | | | | | |
| TOTAL | . CAPITAL | COST | s | | \$ | 5,559,795 | TOTAL | ANNUAL O | M COSTS | i | | \$ | 119,491 |

PWS Name Mirando City WSC
Alternative Name New Well at Mirando

Alternative Number MC-3

 Distance from PWS to new well location
 0.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 3

 Well installation cost (location specific)
 \$153 per foot

 Pump Stations needed w/1 feed tank each
 0

 On site storage tanks / pump sets needed
 0

Capital Costs

| Cost Item | Quantity | Unit | Un | it Cost | T | otal Cost | Cost Item | Quantity | Unit | Unit | Cost | To | otal Cost |
|---------------------------------|----------|--------|------|---------|-----|-----------|-------------------------|------------|-------|---------|-------|----|---------------|
| Pipeline Construction | | -/- | - /- | | / | | Pipeline O&M | 0.0 | | • | 205 | Φ. | - |
| Number of Crossings, bore | | n/a | n/a | | n/a | | Pipeline O&M | 0.0 | mile | \$ | 285 | \$ | 5 5 |
| Number of Crossings, open cut | - | n/a | n/a | | n/a | 0.000 | Subtotal | | | | | \$ | 5 |
| PVC water line, Class 200, 06" | 100 | | \$ | 21 | \$ | 2,068 | | | | | | | |
| Bore and encasement, 10" | - | LF | \$ | 235 | \$ | - | | | | | | | |
| Open cut and encasement, 10" | | LF | \$ | 127 | \$ | | | | | | | | |
| Gate valve and box, 06" | 0 | | \$ | 1,125 | \$ | 22 | | | | | | | |
| Air valve | - | EA | \$ | 2,079 | \$ | - | | | | | | | |
| Flush valve | | | \$ | 1,700 | \$ | 34 | | | | | | | |
| Metal detectable tape | 100 | LF | \$ | 0 | \$ | 5 | | | | | | | |
| Subtotal | | | | | \$ | 2,129 | | | | | | | |
| Pump Station(s) Installation | | | | | | | Pump Station(s) O&M | | | | | | |
| Pump | - | EA | \$ | 8,230 | \$ | - | Building Power | - | kWH | \$ | 0.072 | \$ | - |
| Pump Station Piping, 06" | - | EA | \$ | 817 | \$ | - | Pump Power | - | kWH | \$ | 0.072 | \$ | - |
| Gate valve, 06" | - | EA | \$ | 1,125 | \$ | - | Materials | - | EA | \$ | 1,585 | \$ | - |
| Check valve, 06" | - | EA | \$ | 1,223 | \$ | - | Labor | - | Hrs | \$ | 60.00 | \$ | - |
| Electrical/Instrumentation | - | EA | \$ | 10,550 | \$ | - | Tank O&M | - | EA | \$ | 1,055 | \$ | - |
| Site work | - | EA | \$ | 2,635 | \$ | - | Backflow Cert/Test | 1 | EA | \$ | 110 | \$ | 110 |
| Building pad | - | EA | \$ | 5,275 | \$ | - | Subtotal | | | | | \$ | 110 |
| Pump Building | - | EA | \$ | 10,550 | \$ | - | | | | | | | |
| Fence | - | EA | \$ | 6,330 | \$ | - | | | | | | | |
| Tools | - | EA | \$ | 1.055 | \$ | _ | | | | | | | |
| 5,000 gal feed tank | | EA | \$ | 12,487 | \$ | _ | | | | | | | |
| 50,000 gal ground storage tank | | EA | | 101,655 | \$ | _ | | | | | | | |
| Backflow Preventor | 3 | B EA | \$ | 4,059 | \$ | 12,176 | | | | | | | |
| Subtotal | | , _, | Ψ | 4,000 | \$ | 12,176 | | | | | | | |
| Well Installation | | | | | | | Well O&M | | | | | | |
| Well installation | 900 | 1 F | \$ | 153 | \$ | 137,700 | Pump power | 70.378 | kWH | \$ | 0.072 | \$ | 5.054 |
| Water quality testing | | EΑ | \$ | 1,320 | \$ | 7,920 | Well O&M matl | | EA | \$ | 1.585 | \$ | 4,755 |
| Well pump | - | EA | \$ | 4.132 | \$ | 12.396 | Well O&M labor | - | Hrs | \$ | 60 | \$ | 32,400 |
| Well electrical/instrumentation | - | EA | \$ | 5.800 | \$ | 17,400 | Subtotal | 340 | 1113 | Ψ | 00 | \$ | 42,209 |
| Well cover and base | | EA | \$ | 3,165 | \$ | 9,495 | Subtotai | | | | | φ | 42,209 |
| | | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | |
| Piping Subtotal | | EA | Ф | 3,103 | \$ | 194,406 | | | | | | | |
| | | | | | | | O&M Credit for Existing | Wall Class | ıro | | | | |
| | | | | | | | Pump power | 126.680 | | \$ | 0.072 | \$ | (9,097) |
| | | | | | | | Well O&M matl | | EA | э \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | | Hrs | э \$ | | | , |
| | | | | | | | | 360 | HIS | Ф | 60 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of C | componen | t Cost | S | | \$ | 208,711 | | | | | | | |
| Contingency | 20% | | | | \$ | 41,742 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 52,178 | | | | | | | |
| TOTAL | CAPITAL | COSTS | 3 | | \$ | 302,632 | TOTAL AN | INUAL O8 | M COS | TS | | \$ | 8,457 |

PWS Name Mirando City WSC
Alternative Name New Well at 10 Miles

Alternative Number MC-4

 Distance from PWS to new well location
 10.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 3

 Well installation cost (location specific)
 \$153 per foot

 Pump Stations needed w/ 1 feed tank each
 1

 On site storage tanks / pump sets needed
 0

Capital Costs

| Cost Item Pipeline Construction | Quantity | Unit | Un | it Cost | 7 | otal Cost | Cost Item Pipeline O&M | Quantity | Unit | Unit | Cost | To | otal Cost |
|---------------------------------|-----------|-------|-----|---------|-----|-----------|------------------------|------------|--------|------|-------|----|-----------|
| Number of Crossings, bore | 3 | n/a | n/a | 1 | n/a | ı | Pipeline O&M | 10.0 | mile | \$ | 285 | \$ | 2.850 |
| Number of Crossings, open cut | 13 | n/a | n/a | ı | n/a | ı | Subtotal | | | • | | \$ | 2,850 |
| PVC water line, Class 200, 06" | 52,800 | LF | \$ | 21 | \$ | 1,091,769 | | | | | | • | , |
| Bore and encasement, 10" | 600 | | \$ | 235 | \$ | 140,832 | | | | | | | |
| Open cut and encasement, 10" | 650 | | \$ | 127 | \$ | 82,641 | | | | | | | |
| Gate valve and box, 06" | 11 | EA | \$ | 1.125 | \$ | 11.877 | | | | | | | |
| Air valve | 10 | EA | \$ | 2,079 | \$ | 20,790 | | | | | | | |
| Flush valve | 11 | EA | \$ | 1,700 | \$ | 17,952 | | | | | | | |
| Metal detectable tape | 52,800 | LF | \$ | 0 | \$ | 2,640 | | | | | | | |
| Subtotal | , | | • | | \$ | 1,368,500 | | | | | | | |
| Pump Station(s) Installation | | | | | | | Pump Station(s) O&N | 1 | | | | | |
| Pump | 2 | EA | \$ | 8,230 | \$ | 16,460 | Building Power | 11,800 | kWH | \$ | 0.072 | \$ | 847 |
| Pump Station Piping, 06" | 1 | EA | \$ | 817 | \$ | 817 | Pump Power | 49,671 | kWH | \$ | 0.072 | \$ | 3,567 |
| Gate valve, 06" | 4 | EA | \$ | 1,125 | \$ | 4,499 | Materials | 1 | EA | \$ | 1,585 | \$ | 1,585 |
| Check valve, 06" | 2 | EA | \$ | 1,223 | \$ | 2,445 | Labor | 365 | Hrs | \$ | 60.00 | \$ | 21,900 |
| Electrical/Instrumentation | 1 | EA | \$ | 10,550 | \$ | 10,550 | Tank O&M | - | EA | \$ | 1,055 | \$ | - |
| Site work | 1 | EA | \$ | 2,635 | \$ | 2,635 | Subtotal | | | | | \$ | 27,899 |
| Building pad | 1 | EA | \$ | 5,275 | \$ | 5,275 | | | | | | | |
| Pump Building | 1 | EA | \$ | 10,550 | \$ | 10,550 | | | | | | | |
| Fence | 1 | EA | \$ | 6,330 | \$ | 6,330 | | | | | | | |
| Tools | 1 | | \$ | 1,055 | \$ | 1,055 | | | | | | | |
| 5,000 gal feed tank | 1 | EA | \$ | 12,487 | \$ | 12,487 | | | | | | | |
| 50,000 gal ground storage tank | - | EA | \$ | 101,655 | \$ | - | | | | | | | |
| Subtotal | | | | | \$ | 73,102 | | | | | | | |
| Well Installation | | | | | | | Well O&M | | | | | | |
| Well installation | 900 | 1 F | \$ | 153 | \$ | 137,700 | Pump power | 70.378 | kWH | \$ | 0.072 | \$ | 5,054 |
| Water quality testing | | EΑ | \$ | 1,320 | \$ | 7,920 | Well O&M matl | - , | EA | \$ | 1,585 | \$ | 4,755 |
| Well pump | 3 | | \$ | 4,132 | | 12,396 | Well O&M labor | | Hrs | \$ | 60 | \$ | 32,400 |
| Well electrical/instrumentation | 3 | | \$ | 5.800 | \$ | 17,400 | Subtotal | | | • | | \$ | 42,209 |
| Well cover and base | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | • | , |
| Piping | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | |
| Subtotal | | | · | | \$ | 194,406 | | | | | | | |
| | | | | | | | O&M Credit for Existi | ng Well Cl | osure | | | | |
| | | | | | | | Pump power | 126,680 | kWH | \$ | 0.072 | \$ | (9,097) |
| | | | | | | | Well O&M matl | 2 | EA | \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | 360 | Hrs | \$ | 60 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of C | omponent | ; | | | | | | | | | | | |
| Contingency | 20% | , | | | \$ | 327,202 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 409,002 | | | | | | | |
| · · | | | | | _ | | | | | | | | |
| TOTAL (| CAPITAL (| COSTS | ; | | \$ | 2,372,213 | TOTAL AN | NUAL 0& | M COST | s | | \$ | 39,091 |

PWS Name Mirando City WSC
Alternative Name New Well at 5 Miles

Alternative Number MC-5

 Distance from PWS to new well location
 5.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 3

 Well installation cost (location specific)
 \$153 per foot

 Pump Stations needed w/1 feed tank each
 1

 On site storage tanks / pump sets needed
 0

Capital Costs

| Cost Item Pipeline Construction | Quantity | Unit | Un | it Cost | To | otal Cost | Cost Item Pipeline O&M | Quantity | Unit | Unit | Cost | То | tal Cost |
|---------------------------------|-----------|-------|-----|---------|-----|-----------|-------------------------|-------------|----------|------|-------|----|----------|
| Number of Crossings, bore | 2 | n/a | n/a | 1 | n/a | | Pipeline O&M | 5.0 |) mile | \$ | 285 | \$ | 1.425 |
| Number of Crossings, open cut | _ | n/a | n/a | | n/a | | Subtotal | 0.0 | , ,,,,,, | • | 200 | \$ | 1,425 |
| PVC water line, Class 200, 06" | 26,400 | | \$ | 21 | \$ | 545.884 | - Cubiciai | | | | | • | .,0 |
| Bore and encasement, 10" | 400 | | \$ | 235 | \$ | 93.888 | | | | | | | |
| Open cut and encasement, 10" | 300 | | \$ | 127 | \$ | 38,142 | | | | | | | |
| Gate valve and box, 06" | | EΑ | \$ | 1,125 | \$ | 5,938 | | | | | | | |
| Air valve | - | EA | \$ | 2.079 | \$ | 10,395 | | | | | | | |
| Flush valve | - | EA | \$ | 1.700 | \$ | 8,976 | | | | | | | |
| Metal detectable tape | 26,400 | | \$ | 0,700 | \$ | 1.320 | | | | | | | |
| Subtotal | -, | | Ψ | • | \$ | 704,544 | | | | | | | |
| | | | | | ٠ | , | | | | | | | |
| Pump Station(s) Installation | | | • | 0.000 | • | 40.400 | Pump Station(s) O&N | | | • | 0.070 | • | 0.47 |
| Pump | _ | EA | \$ | 8,230 | \$ | 16,460 | Building Power | 11,800 | | \$ | 0.072 | \$ | 847 |
| Pump Station Piping, 06" | | EA | \$ | 817 | \$ | 817 | Pump Power | 24,836 | | \$ | 0.072 | \$ | 1,783 |
| Gate valve, 06" | | EA | \$ | 1,125 | \$ | 4,499 | Materials | | EA | \$ | 1,585 | \$ | 1,585 |
| Check valve, 06" | | EA | \$ | 1,223 | \$ | 2,445 | Labor | | Hrs | \$ | 60.00 | \$ | 21,900 |
| Electrical/Instrumentation | | EA | \$ | 10,550 | \$ | 10,550 | Tank O&M | 1 | EA | \$ | 1,055 | \$ | 1,055 |
| Site work | 1 | | \$ | 2,635 | \$ | 2,635 | Subtotal | | | | | \$ | 27,171 |
| Building pad | | EΑ | \$ | 5,275 | \$ | 5,275 | | | | | | | |
| Pump Building | | EΑ | \$ | 10,550 | | 10,550 | | | | | | | |
| Fence | | EA | \$ | 6,330 | | 6,330 | | | | | | | |
| Tools | 1 | | \$ | 1,055 | \$ | 1,055 | | | | | | | |
| 5,000 gal feed tank | 1 | EΑ | \$ | 12,487 | \$ | 12,487 | | | | | | | |
| 50,000 gal ground storage tank | - | EΑ | \$ | 101,655 | \$ | - | | | | | | | |
| Subtotal | | | | | \$ | 73,102 | | | | | | | |
| Well Installation | | | | | | | Well O&M | | | | | | |
| Well installation | 900 | LF | \$ | 153 | \$ | 137,700 | Pump power | 70,378 | kWH | \$ | 0.072 | \$ | 5,054 |
| Water quality testing | 6 | EA | \$ | 1,320 | \$ | 7,920 | Well O&M matl | 3 | EA | \$ | 1,585 | \$ | 4,755 |
| Well pump | 3 | EA | \$ | 4,132 | \$ | 12,396 | Well O&M labor | 540 | Hrs | \$ | 60 | \$ | 32,400 |
| Well electrical/instrumentation | 3 | EA | \$ | 5,800 | \$ | 17,400 | Subtotal | | | | | \$ | 42,209 |
| Well cover and base | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | |
| Piping | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | |
| Subtotal | | | | | \$ | 194,406 | | | | | | | |
| | | | | | | | O&M Credit for Existing | ng Well Clo | sure | | | | |
| | | | | | | | Pump power | 126,680 | kWH | \$ | 0.072 | \$ | (9,097) |
| | | | | | | | Well O&M matl | 2 | EA | \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | 360 | Hrs | \$ | 60 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of C | omponent | Costs | 3 | | \$ | 972,052 | | | | | | | |
| Contingency | 20% | | | | \$ | 194,410 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 243,013 | | | | | | | |
| Design & Constitutionagement | 23% | , | | | φ | 243,013 | | | | | | | |
| TOTAL | CAPITAL (| COSTS | 6 | | \$ | 1,409,476 | TOTAL AN | NUAL O& | M COS | TS | | \$ | 36,938 |

PWS Name Mirando City WSC
Alternative Name New Well at 1 Mile

Alternative Number MC-6

 Distance from PWS to new well location
 1.0 miles

 Estimated well depth
 300 feet

 Number of wells required
 3

 Well installation cost (location specific)
 \$153 per foot

 Pump Stations needed w/ 1 feed tank each
 0

 On site storage tanks / pump sets needed
 0

Capital Costs

| Cost Item Pipeline Construction | Quantity | Unit | Un | it Cost | To | otal Cost | Cost Item Pipeline O&M | Quantity | Unit | Unit | Cost | To | otal Cost |
|---------------------------------|-----------|-------|-----|---------|-----|-----------|------------------------|------------|--------|------|-------|----|-----------|
| Number of Crossings, bore | _ | n/a | n/a | 1 | n/a | | Pipeline O&M | 1.0 | mile | \$ | 285 | \$ | 285 |
| Number of Crossings, open cut | 1 | | n/a | | n/a | | Subtotal | | | Ψ. | 200 | \$ | 285 |
| PVC water line, Class 200, 06" | 5,280 | | \$ | 21 | \$ | 109,177 | | | | | | • | |
| Bore and encasement, 10" | -, | LF | \$ | 235 | \$ | - | | | | | | | |
| Open cut and encasement, 10" | 50 | LF | \$ | 127 | \$ | 6,357 | | | | | | | |
| Gate valve and box, 06" | 1 | EA | \$ | 1,125 | \$ | 1,188 | | | | | | | |
| Air valve | 1 | EA | \$ | 2,079 | \$ | 2,079 | | | | | | | |
| Flush valve | 1 | EA | \$ | 1,700 | \$ | 1,795 | | | | | | | |
| Metal detectable tape | 5.280 | LF | \$ | 0 | \$ | 264 | | | | | | | |
| Subtotal | ., | | | | \$ | 120,860 | | | | | | | |
| Pump Station(s) Installation | | | | | | | Pump Station(s) O&N | 1 | | | | | |
| Pump | - | EA | \$ | 8,230 | \$ | - | Building Power | - | kWH | \$ | 0.072 | \$ | - |
| Pump Station Piping, 06" | - | EA | \$ | 817 | \$ | - | Pump Power | - | kWH | \$ | 0.072 | \$ | - |
| Gate valve, 06" | - | EA | \$ | 1,125 | \$ | - | Materials | - | EA | \$ | 1,585 | \$ | - |
| Check valve, 06" | - | EA | \$ | 1,223 | \$ | - | Labor | - | Hrs | \$ | 60.00 | \$ | - |
| Electrical/Instrumentation | - | EA | \$ | 10,550 | \$ | - | Tank O&M | - | EA | \$ | 1,055 | \$ | - |
| Site work | - | EA | \$ | 2,635 | \$ | - | Subtotal | | | | | \$ | - |
| Building pad | - | EA | \$ | 5,275 | \$ | - | | | | | | | |
| Pump Building | - | EA | \$ | 10,550 | \$ | - | | | | | | | |
| Fence | - | EA | \$ | 6,330 | \$ | - | | | | | | | |
| Tools | - | EA | \$ | 1,055 | \$ | - | | | | | | | |
| 5,000 gal feed tank | - | EA | \$ | 12,487 | \$ | - | | | | | | | |
| 50,000 gal ground storage tank | - | EA | \$ | 101,655 | \$ | - | | | | | | | |
| Subtotal | | | | | \$ | - | | | | | | | |
| Well Installation | | | | | | | Well O&M | | | | | | |
| Well installation | 900 | LF | \$ | 153 | \$ | 137,700 | Pump power | 70.378 | kWH | \$ | 0.072 | \$ | 5.054 |
| Water quality testing | 6 | | \$ | 1.320 | \$ | 7.920 | Well O&M matl | - , | EA | \$ | 1.585 | \$ | 4.755 |
| Well pump | 3 | | \$ | 4,132 | \$ | 12,396 | Well O&M labor | | Hrs | \$ | 60 | \$ | 32,400 |
| Well electrical/instrumentation | 3 | EA | \$ | 5,800 | \$ | 17,400 | Subtotal | | | | | \$ | 42,209 |
| Well cover and base | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | , |
| Piping | 3 | EA | \$ | 3,165 | \$ | 9,495 | | | | | | | |
| Subtotal | | | | | \$ | 194,406 | | | | | | | |
| | | | | | | | O&M Credit for Existi | ng Well Cl | osure | | | | |
| | | | | | | | Pump power | 126,680 | kWH | \$ | 0.072 | \$ | (9,097) |
| | | | | | | | Well O&M matl | 2 | EA | \$ | 1,585 | \$ | (3,170) |
| | | | | | | | Well O&M labor | 360 | Hrs | \$ | 60 | \$ | (21,600) |
| | | | | | | | Subtotal | | | | | \$ | (33,867) |
| Subtotal of Co | omponent | Costs | • | | \$ | 315,266 | | | | | | | |
| Contingency | 20% | | | | \$ | 63,053 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 78,816 | | | | | | | |
| 200igii a consti management | 2070 | • | | | Ψ | 70,010 | | | | | | | |
| TOTAL (| CAPITAL C | COSTS | • | | \$ | 457,136 | TOTAL AN | NUAL O& | M COST | s | | \$ | 8,627 |

PWS Name Mirando City WSC
Alternative Name Central Treatment - RO

Alternative Number MC-7

Capital Costs

Annual Operations and Maintenance Costs

| Cost Item | Quantity | Unit | Un | it Cost | To | otal Cost | Cost Item | Quantity | Unit | Un | it Cost | T | otal Cost |
|---|------------|----------|----|---------|----|-----------|----------------------------|----------|---------------|----|---------|----|-----------|
| Reverse Osmosis Unit Purchase/Ir | | | Φ | 4 000 | Φ | 4 440 | Reverse Osmosis Unit O&M | 40.074 | مر بام باد در | Φ | 0.070 | Φ | 700 |
| Site preparation | | acre | \$ | 4,000 | | 1,440 | Building Power | | kwh/yr | | 0.072 | | 723 |
| Slab | | CY | \$ | 1,000 | \$ | 36,000 | Equipment power | 108,329 | , | \$ | 0.072 | \$ | 7,779 |
| Building | 960 | | \$ | 60 | \$ | 57,600 | Labor | , | hrs/yr | \$ | 40 | \$ | 40,000 |
| Building electrical | 960 | | \$ | 8 | \$ | 7,680 | RO materials and Chemicals | 28,366 | • | \$ | 0.43 | \$ | 12,197 |
| Building plumbing | 960 | | \$ | 8 | \$ | 7,680 | Analyses | | test | \$ | 200 | \$ | 2,400 |
| Heating and ventilation | 960 | - | \$ | 7 | \$ | 6,720 | Subtotal | | | | | \$ | 63,100 |
| Fence | 448 | | \$ | 15 | \$ | 6,720 | | | | | | | |
| Paving | 4,760 | _ | \$ | 2 | \$ | 9,520 | Backwash Disposal | | | | | | |
| Electrical | 1 | | \$ | 100,000 | \$ | 100,000 | Disposal truck mileage | 14,672 | miles | \$ | 1.50 | | 22,008 |
| Piping | 1 | JOB | \$ | 50,000 | \$ | 50,000 | Backwash disposal fee | 7,336 | kgal/yr | \$ | 5.00 | \$ | 36,680 |
| | | | | | | | Subtotal | | | | | \$ | 58,688 |
| Reverse osmosis package inclu High pressure pumps - 10hp Cartridge filters and vessels RO membranes and vessels Control system Chemical feed systems Freight cost | ding: | | | | | | | | | | | | |
| Vendor start-up services | 1 | UNIT | \$ | 313,000 | \$ | 313,000 | | | | | | | |
| Feed pumps | 3 | EA | \$ | 5.000 | \$ | 15,000 | | | | | | | |
| Permeate tank | 26,000 | gal | \$ | 3 | \$ | 78,000 | | | | | | | |
| Reject pond: | | | | | | | | | | | | | |
| Excavation | 2,400 | CYD | \$ | 3.00 | \$ | 7,200 | | | | | | | |
| Compacted fill | 1,600 | CYD | \$ | 4.00 | \$ | 6,400 | | | | | | | |
| Lining | 22,700 | SF | \$ | 0.50 | \$ | 11,350 | | | | | | | |
| Vegetation | 1,996 | SY | \$ | 1.50 | \$ | 2,994 | | | | | | | |
| Fence around pond | 806 | LF | \$ | 15.00 | \$ | 12,090 | | | | | | | |
| Access road | 50 | LF | \$ | 30.00 | \$ | 1,500 | | | | | | | |
| Subtotal of Design/Co | nstruction | Costs | • | | \$ | 730,894 | | | | | | | |
| Contingency | 20% |) | | | \$ | 146,179 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 182,724 | | | | | | | |
| Reject water haulage truck | 1 | EA | \$ | 200 | \$ | 100,000 | | | | | | | |

TOTAL CAPITAL COSTS

\$ 1,159,796

TOTAL ANNUAL O&M COSTS

121,788

Table C.8

PWS Name Mirando City WSC

Alternative Name Central Treatment - Iron-Based Absorption

Alternative Number MC-8

Capital Costs

| Cost Item | Quantity | Unit | Uni | t Cost | Total Cost | Cost Item | Quantity | Unit | Un | it Cost | To | otal Cost |
|---------------------------------------|-----------|---------|-----|---------|---------------|-----------------------------|----------|---------|----|---------|----|-----------|
| Adsorption Unit Purchase/Installation | | | | | | Adsorption Unit O&M | | | | | | |
| Site preparation | | acre | \$ | 4,000 | 1,440 | Building Power | | kwh/yr | | 0.072 | | 725 |
| Slab | 36 | - | \$ | 1,000 | \$ 36,000 | Equipment power | • | kwh/yr | | 0.072 | \$ | 1,870 |
| Building | 960 | | \$ | 60 | \$ 57,600 | Labor | | hrs/yr | \$ | 40 | \$ | 32,000 |
| Building electrical | 960 | SF | \$ | 8 | \$ 7,680 | Media replacement | 117 | cf | \$ | 80 | \$ | 9,360 |
| Building plumbing | 960 | SF | \$ | 8 | \$ 7,680 | Analyses | 12 | test | \$ | 200 | \$ | 2,400 |
| Heating and ventilation | 960 | SF | \$ | 7 | \$ 6,720 | Backwash discharge to sewer | - | MG/yr | \$ | 5,000 | | |
| Fence | 448 | | \$ | 15 | \$ 6,720 | Spent Media Disposal | 117 | CY | \$ | 20 | \$ | 2,340 |
| Paving | 4,760 | SF | \$ | 2 | \$ 9,520 | Subtota | al | | | | \$ | 48,696 |
| Electrical | 1 | JOB | \$ | 80,000 | \$ 80,000 | | | | | | | |
| Piping | 1 | JOB | \$ | 50,000 | \$ 50,000 | Backwash Disposal | | | | | | |
| | | | | | | Disposal truck mileage | (|) miles | \$ | 1.50 | | \$0 |
| Adsorption package including: | | | | | | Backwash disposal fee | (| kgal/yr | \$ | 5.00 | | \$0 |
| 3 Adsorption vessels | | | | | | Subtota | al | | | | | \$0 |
| E33 Iron oxide media | | | | | | | | | | | | |
| Controls & instruments | 1 | UNIT | \$ | 127,162 | \$ 127,162 | | | | | | | |
| Backwash Tank | 3,602 | GAL | \$ | 2 | \$ 7,204 | | | | | | | |
| Chlorination Point | 1 | EA | \$ | 4,000 | \$ 4,000 | | | | | | | |
| Backwash evap pond | | | | | | | | | | | | |
| Excavation | 40 | CYD | \$ | 3.00 | \$ 120 | | | | | | | |
| Compacted fill | 35 | CYD | \$ | 4.00 | \$ 140 | | | | | | | |
| Lining | 949 | SF | \$ | 0.50 | \$ 475 | | | | | | | |
| Vegetation | 274 | SY | \$ | 1.50 | \$ 411 | | | | | | | |
| Fence around pond | 234 | LF | \$ | 15.00 | \$ 3,510 | | | | | | | |
| Access road | 50 | LF | \$ | 30.00 | \$ 1,500 | | | | | | | |
| Subtotal of Co | omponent | t Costs | • | | \$ 407,882 | | | | | | | |
| Contingency | 20% |) | | | \$ 81,576 | | | | | | | |
| Design & Constr Management | 25% |) | | | \$ 101,970 | | | | | | | |
| TOTAL (| CAPITAL (| COSTS | ; | | \$ 591,429 | TOTAL AN | NUAL O&M | COSTS | | | \$ | 48,696 |

PWS Name Mirando City WSC
Alternative Name Point-of-Use Treatment

Alternative Number MC-9

Number of Connections for POU Unit Installation 250 connections

Capital Costs

| Cost Item POU-Treatment - Purchase/Install | Quantity ation | Unit | Uni | t Cost | To | otal Cost | Cost Item O&M | Quantity | Unit | Unit | Cost | To | otal Cost |
|--|-------------------|---------|-----|--------|----|-----------|------------------------------------|----------|-------|------|------|----|-----------|
| POU treatment unit purchase | 250 | EΑ | \$ | 300 | \$ | 75,000 | POU materials, per unit | 250 | EΑ | \$ | 103 | \$ | 25,750 |
| POU treatment unit installation | 250 | EΑ | \$ | 160 | \$ | 40,000 | Contaminant analysis, 1/yr per uni | 250 | EΑ | \$ | 210 | \$ | 52,500 |
| Subtota | al | | | | \$ | 115,000 | Program labor, 10 hrs/unit | 2,500 | hrs | \$ | 42 | \$ | 105,000 |
| | | | | | | | Subtotal | | | | | \$ | 183,250 |
| Subtotal of 0 | Componen | t Costs | 8 | | \$ | 115,000 | | | | | | | |
| Contingency | 20% | , D | | | \$ | 23,000 | | | | | | | |
| Design & Constr Management | 25% | , D | | | \$ | 28,750 | | | | | | | |
| Procurement & Administration | 20% | Ď | | | \$ | 23,000 | | | | | | | |
| TOTAL | CAPITAL | COSTS | 6 | | \$ | 189,750 | TOTAL ANNU | AL O&M C | соѕтѕ | 6 | | \$ | 183,250 |

PWS Name Mirando City WSC

Alternative Name Point-of-Entry Treatment

Alternative Number *MC-10*

Number of Connections for POE Unit Installation 250 connections

Capital Costs

| Cost Item POE-Treatment - Purchase/Installa | Quantity | Unit | Un | it Cost | 7 | Total Cost | Cost Item O&M | Quantity | / Unit | Uni | t Cost | To | otal Cost |
|---|-----------|-------|----|---------|----|------------|-------------------------------------|----------|--------|-----|--------|----|-----------|
| POE treatment unit purchase | 250 | EΑ | \$ | 5,275 | \$ | 1,318,750 | POE materials, per unit | 250 | EΑ | \$ | 1,585 | \$ | 396,250 |
| Pad and shed, per unit | 250 | EΑ | \$ | 2,110 | \$ | 527,500 | Contaminant analysis, 1/yr per unit | 250 | EΑ | \$ | 210 | \$ | 52,500 |
| Piping connection, per unit | 250 | EΑ | \$ | 1,055 | \$ | 263,750 | Program labor, 10 hrs/unit | 2,500 | hrs | \$ | 42 | \$ | 105,000 |
| Electrical hook-up, per unit | 250 | EΑ | \$ | 1,055 | \$ | 263,750 | Subtotal | | | | | \$ | 553,750 |
| Subtotal | | | | | \$ | 2,373,750 | | | | | | | |
| Subtotal of C | omponent | Costs | 5 | | \$ | 2,373,750 | | | | | | | |
| Contingency | 20% | | | | \$ | 474,750 | | | | | | | |
| Design & Constr Management | 25% |) | | | \$ | 593,438 | | | | | | | |
| Procurement & Administration | 20% | • | | | \$ | 474,750 | | | | | | | |
| TOTAL | CAPITAL (| COSTS | 3 | | \$ | 3,916,688 | TOTAL ANNUA | L O&M C | COSTS | 3 | | \$ | 553,750 |

PWS Name Mirando City WSC

Alternative Name Public Dispenser for Treated Drinking Water

1

Alternative Number MC-11

Number of Treatment Units Recommended

Capital Costs

| Cost Item Public Dispenser Unit Installation | Quantity | Unit | Un | it Cost | T | otal Cost | Cost Item Program Operation | Quantity | Unit | Uni | t Cost | То | tal Cost |
|--|----------|----------|----|---------|----|-----------|----------------------------------|----------|-------|-----|--------|----|----------|
| POE-Treatment unit(s) | 1 | EΑ | \$ | 7,385 | \$ | 7,385 | Treatment unit O&M, 1 per unit | 1 | EΑ | \$ | 2,100 | \$ | 2,100 |
| Unit installation costs | 1 | EA | \$ | 5,275 | \$ | 5,275 | Contaminant analysis, 1/wk per u | 52 | EΑ | \$ | 210 | \$ | 10,920 |
| Subtota | İ | | | | \$ | 12,660 | Sampling/reporting, 1 hr/day | 365 | HRS | \$ | 60 | \$ | 21,900 |
| | | | | | | | Subtotal | | | | | \$ | 34,920 |
| Subtotal of C | componen | t Costs | 5 | | \$ | 12,660 | | | | | | | |
| Contingency | 20% | , | | | \$ | 2,532 | | | | | | | |
| Design & Constr Management | 25% | 6 | | | \$ | 3,165 | | | | | | | |
| TOTAL | CAPITAL | COSTS | 3 | | | 18,357 | TOTAL ANNU | AL O&M C | COSTS | 6 | | \$ | 34,920 |

PWS Name Mirando City WSC

Alternative Name Supply Bottled Water to 100% of Population

Alternative Number *MC-12*

Service Population500Percentage of population requiring supply100%Water consumption per person1.00 gpcdCalculated annual potable water needs182,500 gallons

Capital Costs

| Cost Item Program Implementation | Quantity | Unit | Unit (| Cost | To | otal Cost | Cost Item Program Operation | Quantity | Unit | Unit | Cost | To | otal Cost |
|----------------------------------|------------------------|---------|--------|------|-----------------|-------------------------|--|----------------|------|----------|------------|----------|-------------------------|
| Initial program set-up | 500 Subtotal | hours | \$ | 46 | \$ \$ | 23,000 23,000 | Water purchase costs Program admin, 9 hrs/wk | 182,500 468 | - | \$ \$ | 1.55 46 | \$ \$ | 282,875 21,528 |
| · | oubtotui | | | | Ф | 20,000 | Program materials Subtota | 1 | EA | \$ | 5,275 | * | 5,275 309,678 |
| Subte | otal of Component | t Costs | i | | \$ | 23,000 | | | | | | | |
| Contingency | 20% | b | | | \$ | 4,600 | | | | | | | |
| | TOTAL CAPITAL | COSTS | } | Ī | \$ | 27,600 | TOTAL AN | NUAL O&M C | OSTS | | ĺ | \$ | 309,678 |

PWS Name Mirando City WSC

Alternative Name Central Trucked Drinking Water - Oilton

Alternative Number MC-13

Service Population500Percentage of population requiring supply100%Water consumption per person1.00 gpcdCalculated annual potable water needs182,500 gallonsTravel distance to compliant water source4 miles

Capital Costs

| Cost Item Storage Tank Installation | Quantity | Unit | Uni | it Cost | To | otal Cost | Cost Item Program Operation | Quantity | Unit | Uni | t Cost | To | otal Cost |
|--|-----------|---------|-----|---------|----|-----------|----------------------------------|----------|------------|-----|--------|----|-----------|
| 5,000 gal ground storage tank | 1 | EΑ | \$ | 12,487 | \$ | 12.487 | Water delivery labor, 4 hrs/wk | 208 | hrs | \$ | 60 | \$ | 12.480 |
| Site improvements | 1 | EΑ | \$ | 3.165 | \$ | 3,165 | Truck operation, 1 round trip/wk | 364 | miles | \$ | 1.50 | \$ | 546 |
| Potable water truck | 1 | EΑ | \$ | 115,000 | \$ | 115,000 | Water purchase | 183 | 1,000 gals | \$ | 2.97 | \$ | 541 |
| Subtotal | | | | • | \$ | 130,652 | Water testing, 1 test/wk | 52 | EA | \$ | 210 | \$ | 10,920 |
| | | | | | · | • | Sampling/reporting, 2 hrs/wk | 104 | hrs | \$ | 60 | \$ | 6,240 |
| | | | | | | | Subtotal | I | | | | \$ | 30,727 |
| Subtotal of C | omponent | t Costs | 5 | | \$ | 130,652 | | | | | | | |
| Contingency | 20% | D | | | \$ | 26.130 | | | | | | | |
| Design & Constr Management | 25% | | | | \$ | 32,663 | | | | | | | |
| TOTAL | CAPITAL (| COSTS | 6 | | \$ | 189,445 | TOTAL A | ANNUAL C | &M COSTS | | | \$ | 30,727 |

1 APPENDIX D 2 EXAMPLE FINANCIAL MODEL

Mirando City WSC

| Mirando City WSC | | |
|--|-------------------|-----------------------------|
| Number of Alternatives | 13 | Selected from Results Sheet |
| Input Fields are Indicated | by: | |
| General Inputs | | 1 |
| | 2011 | |
| Implementation Year | 2011 | |
| Months of Working Capital | 0 | |
| Depreciation | - | |
| Percent of Depreciation for Replacement Fund | 0% | |
| Allow Negative Cash Balance (yes or no) | No 25 500 | |
| Median Household Income | \$ 26,500 | Mirando City WSC |
| Median HH Income Texas | \$ 39,927 | |
| Grant Funded Percentage | 0% | Selected from Results |
| Capital Funded from Revenues | - | |
| L . | Base Year | 200 |
| | Growth/Escalation | 200 |
| Accounts & Consumption | | |
| Metered Residential Accounts | | |
| Number of Accounts | 0.0% | 25 |
| Number of Bills Per Year | | 1 |
| Annual Billed Consumption | | 29,200,000 |
| Consumption per Account Per Pay Period | 0.0% | |
| Consumption Allowance in Rates | | 2,000 |
| Total Allowance | | 6,000,000 |
| Net Consumption Billed | | 23,200,000 |
| Percentage Collected | | 100.09 |
| | | |
| Unmetered Residential Accounts | | |
| Number of Accounts | 0.0% | |
| Number of Bills Per Year | | |
| Percentage Collected | | 100.09 |
| Metered Non-Residential Accounts | | |
| Number of Accounts | 0.0% | |
| Number of Bills Per Year | | 1 |
| Non-Residential Consumption | | _ |
| Consumption per Account | 0.0% | - |
| Consumption Allowance in Rates | | _ |
| Total Allowance | | - |
| Net Consumption Billed | | - |
| Percentage Collected | | 0.0 |
| Unmetered Non D1-4-1 A | | |
| Unmetered Non-Residential Accounts | 0.000 | |
| Number of Accounts | 0.0% | |
| Number of Bills Per Year | | 100.00 |
| Percentage Collected | | 100.09 |
| Water Purchase & Production | | |
| Water Purchased (gallons) | 0.0% | _ |
| Average Cost Per Unit Purchased | 0.0% | |
| Bulk Water Purchases | 0.0% | |
| Water Production | 0.0% | |
| Unaccounted for Water | | - |
| Percentage Unaccounted for Water | | 0.09 |
| | | 0.0 |

Mirando City WSC

Number of Alternatives 13 Selected from Results Sheet

Input Fields are Indicated by:

| Residential Rate Structure | Allowance within Tier | |
|---|-----------------------|--------------|
| | - | |
| Estimated Average Water Rate (\$/1000gallons) | | \$ 6.51 |
| | | |
| | | |
| | | |
| | | |
| Non-Residential Rate Structure | | |
| | - | |
| Estimated Average Water Rate (\$/1000gallons) | | \$ |
| | | |
| | | |
| | | |
| INITIAL YEAR EXPENDITURES | Inflation | Initial Year |
| Operating Expenditures: | | |
| Salaries & Benefits | 0.0% | - |
| Contract Labor | 0.0% | - |
| Water Purchases | 0.0% | - |
| Chemicals, Treatment | 0.0% | - |
| Utilities | 0.0% | |
| Repairs, Maintenance, Supplies | 0.0% | - |
| Repairs | 0.0% | |
| Maintenance | 0.0% | - |
| Supplies | 0.0% | |
| Administrative Expenses | 0.0% | |
| Accounting and Legal Fees | 0.0% | - |
| Insurance | 0.0% | |
| Automotive and Travel | 0.0% | |
| Professional and Directors Fees Bad Debts | 0.0% | |
| Garbage Pick-up | 0.0% | - |
| Miscellaneous | 0.0% | |
| Other 3 | 0.0% | 187,500 |
| Other 4 | 0.0% | - |
| Incremental O&M for Alternative | 0.0% | |
| Total Operating Expenses | 3,070 | 187,500 |
| | | |
| Non-Operating Income/Expenditures | | |
| Interest Income | 0.0% | - |
| Other Income | 0.0% | - |
| Other Expense | 0.0% | - |
| Transfers In (Out) | 0.0% | |
| Net Non-Operating | | - |
| Esisting Debt Service | | |
| Bonds Payable, Less Current Maturities | | \$ - |
| Bonds Payable, Current | | \$ - |
| Interest Expense | | \$ - |

Debt Service for Mirando City WSC Alternative Number = 13 Funding Source = Loan/Bond

| Fullding Source – | Loan/bo | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--------------------------------|---------|------|------|--------------|---------|----------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|----------|--------|--------|--------|--------|--------|----------|----------|--------|------|------|------|------|
| | | 2009 | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 | | 2020 | 2021 | 2022 | | 2024 | 2025 | | | 2028 | 2029 | 2030 | 2031 | 2032 | | 2034 | 2035 | 2036 | 2037 | 2038 | 2039 |
| | | 0 | 1 | 2 | 3 | 4 | 1 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Existing Debt Service | \$ - | | _ | - | - | - | - | - | - | - | - | _ | - | _ | - | - | - | - | - | - | - | - | - | - | - | - | _ | - | - 1 | - | - | _ |
| Principal Payments | | _ | _ | - | _ | _ | _ | _ | _ | - | _ | _ | _ | - | - | _ | _ | _ | _ | - | _ | _ | _ | - | _ | _ | _ | _ | _ | - | _ | _ |
| Interest Payment | 0.00% | _ | _ | - | _ | _ | _ | _ | _ | - | _ | _ | _ | - | - | _ | _ | _ | _ | _ | _ | _ | _ | - | _ | _ | _ | _ | _ | - | _ | _ |
| Total Debt Service | 0.007 | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| New Balance | | _ | _ | _ | _ | _ | - | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | |
| riew Buildine | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Т | 25 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Term | 23 | | | 400 445 | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Revenue Bonds | | - | - | 189,445 | - | - | - | - | - | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| Forgiveness | 0.00% | - | - | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 |
| Balance | | - | - | 189,445 | 185,992 | 182,332 | | | 169,981 | 165,360 | 160,462 | 155,270 | 149,766 | 143,933 | 137,749 | 131,194 | 124,246 | 116,881 | 109,074 | 100,799 | 92,027 | 82,729 | 72,873 | 62,426 | 51,352 | 39,613 | 27,170 | 13,981 | 0 | 0 | 0 | 0 |
| Principal | | - | - | 3,453 | 3,660 | 3,880 | 4,113 | 4,359 | 4,621 | 4,898 | 5,192 | 5,504 | 5,834 | 6,184 | 6,555 | 6,948 | 7,365 | 7,807 | 8,275 | | | 9,856 | 10,447 | 11,074 | 11,739 | 12,443 | 13,189 | 13,981 | - | - | - | - |
| Interest | 6.00% | - | - | 11,367 | 11,160 | 10,940 | 10,707 | 10,460 | 10,199 | 9,922 | 9,628 | 9,316 | 8,986 | 8,636 | 8,265 | 7,872 | 7,455 | 7,013 | 6,544 | 6,048 | 5,522 | 4,964 | 4,372 | 3,746 | 3,081 | 2,377 | 1,630 | 0 | 0 | 0 | 0 | 0 |
| Total Debt Service | | - | | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 14,820 | 13,981 | 0 | 0 | 0 | 0 |
| New Balance | | - | - | 185,992 | 182,332 | 178,453 | 174,340 | 169,981 | 165,360 | 160,462 | 155,270 | 149,766 | 143,933 | 137,749 | 131,194 | 124,246 | 116,881 | 109,074 | 100,799 | 92,027 | 82,729 | 72,873 | 62,426 | 51,352 | 39,613 | 27,170 | 13,981 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Term | 20 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| State Revolving Fund | | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ | _ |
| Forgiveness | 0.00% | | | 1 | 2 | 3 | 1 | 5 | 6 | 7 | Q | 0 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 |
| Balance | 0.0070 | _ | _ | 1 | 2 | 3 | - | 3 | 0 | , | 0 | , | 10 | 11 | - | 13 | 14 | 13 | 10 | 17 | 10 | 1) | - | - | 22 | 23 | - | 23 | 20 | 21 | 20 | 2) |
| Principal | | - | - | - | _ | - | _ | _ | - | - | - | - | _ | - | | - | - | - | _ | _ | - | - | | | - | - | | - | - | - | - | - |
| II - I | 0.00% | - | - | _ | - | _ | - | _ | - | - | - | - | - | - | - | - | - | - | _ | _ | - | - | - | - | - | - | - | - | - | - | - | - ! |
| Interest Total Debt Service | 0.00% | - | | - | - | - | - | - | _ | - | | - | _ | | - | - | - | | | - | - | - | | - | - | _ | - | - | - | - | - | |
| | = | | | 1 | | | 1 | + | l l | - | | | | | | | | | 1 | 1 | | | | | | - | | | | | | |
| New Balance | | - | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Term | 10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Bank/Interfund Loan | | | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| Forgiveness | 0.00% | - | - | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 |
| Balance | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| Principal | | - | - | - | - | _ | - | - | - | - | _ | - | - | - | - | - | - | - | - | - | _ | - | - | - | - | _ | - | - | - | - | - | - |
| Interest | 8.00% | - | - | - | - | - | _ | - | - | - | - | - | _ | - | - | - | - | - | - | - | - | - | - | - | - | _ | _ | - | - | - | - | - |
| Total Debt Service | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| New Balance | | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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